;	the 2 The	ding Code of New York State whi 2018 International Building Code. structural components have been			ure Prevention and Build	aing Code and is based upon	portion 2 The fc psf as	nent, not a cor ns of the repo oundations hav s recommende
	A.	Uniform live load: Corridors above first floor Lobbies and first-floor corrid Offices Classrooms	lors			80 psf 100 psf 50 psf + 15 psf partitions 40 psf + 15 psf partitions	concre 3 The sl The m	g elevations nete in order to lab-on-grade s naterial shall h 00 sieve.
	Б	Stairs and exits All other Live load has been reduced or Roof loads:	n girders, columns	s and footings in accorda		100 psf de	4 The b 5 All soi 6 Step f	ottom of exter il surrounding footings where
	В.	Snow: Ground snow load, p <sub>g</sub> Flat roof snow load, p <sub>f</sub>				25 psf 30 psf*	compr 8 Found	dation walls sh ressive streng dation walls sh
		Exposure factor, $C_e$ Importance factor, $I_s$ Thermal factor, $C_t$				1.0 1.1 1.0	height into m	ium anchor bo t basements a nasonry, and 7 shall have a m
		Rain loads: in accordance with Roof live load:	h Section 1611			20 psf min	10 Keep 11 Use c	foundation exp rushed stone l ng utilities: loc
		Rainfall intensity (15-min. dura *Note: the flat roof snow load	shall be no less th	nan 30 psf.		6.00 in./hr.	during 13 Where	g earthwork op e footings are wise shown on
	C.	The link building roof has beer Wind design data:	-	additional 30psf dead lo	ad to account for green r	roof.	surrou	fabrics/geotex unding soil. Fa ittals to the en
		Wind loads have been determi Section 1609.1.1 in accordanc Risk category		6, Chapters 26, 27, 29 ar	nd 30, Directional Procec	dure	Concrete No	
		Basic wind speed (3-second g Ultimate design v			125 mph 97 mph		"Build	ncrete work sł ing Code Req rete shall be tł
		Exposure Internal pressure coefficient "a" dimension for use with com			B 0.18 12.8 ft			Location
		Design wind pressure (Nomina use Zone 5 within "a" of buildir	ng wall corners):		and cladding on building	g walls (use Zone 4 generally;		Footings Walls and pie Interior
		Area Negative Zone 4	<u>10 sq.ft</u> -32.8	Surface pressure (psf) <u>20 sq. ft.</u> -31.5 27 Z	50 sq. ft. -29.7	100 sq. ft. -28.4		Exterior Exterior e Slabs-on-gra
		Negative Zone 5 Positive Zones 4 & 5 Design wind pressure (Nomina		-37.7 29.0 ssures) for components	-34.2 27.2 and cladding on building	-31.5 25.9 g roofs (for locations of zones		Slabs-on-me Slabs-on-me
		1, 2 and 3, refer to building cod <u>Area</u> Negative Zone 1		Surface pressure (psf) 20 sq. ft. -49.3	<u>50 sq. ft.</u> -44.7	<u>100 sq.ft.</u> -41.2		tailing fabricat
		Negative Zone 1' Negative Zone 2 Negative Zone 3	-30.3 -69.6 -94.9	-30.3 -65.1 -85.9	-30.3 -59.2 -74.1	-30.3 -54.7 -65.1	4 Concr the mi	ual of Standard rete design miz inimum streng und cement sha
		Positive all zones Overhang Zones 1 & 1' Overhang Zone 2	16.0 -47.7 -64.6	16.0 -46.9 -58.6	16.0 -45.8 -50.7	16.0 -44.9 -44.7	6 Other concre	cement sn cementitious ete mix. Flyas to 35%; Class
	D.	Overhang Zone 3 Earthquake design data:	-89.8	-79.4	-65.6	-55.1	total w 7 For no	veight of ceme ormal weight of al sand from th
		Risk category Seismic importance factor, I <sub>e</sub> : Mapped short period spectral I Mapped 1 second period spec			III 1.25 0.296g 0.061g		8 No ad such a compo	lmixtures are   as that used ir ounds shall co
		Site class: Design short period spectral re Design 1 second period spectral re	esponse accelerat	ions, S <sub>DS</sub> :	D 0.308g 0.098g		concre chloric	water-soluble, ete that will be des, 0.15 for r
		Seismic design category: Seismic force resisting system Classroom Building: Structura	n varies:		В		10 Lightw Lightw	essed (post-te veight concret veight aggrega
		Community Building: Interme Design base shear: Classroom Building:	diate reinforced m	nasonry walls	181 KIP		11 Reinfo 12 Welde	eight aggrega orcing steel sh ed wire fabric s ogether.
		Community Building: Seismic response coefficient, ( Classroom Building:	Cs:		34KIPS 0.059		13 Vapor condit	r retarder shal tioning (ASTM STM E 1643 a
		Community Building: Response modification factor, Classroom Building:	R:		0.11 3 3.5		14 The fo	bllowing concr
		Community Building: Deflection amplification factor, Classroom Building: Community Building:	Cd:		3.5 3 2.25			Concrete cas Permanently
	E.	Analysis procedure: Equivalen Other loads:	it Lateral Force					Concrete exp
		Concentrated loads: All floors except as note Stair treads and catwall	ks (on 4 inches sq	juare)	3	1000 lbs 300 lb		Concrete not Slabs
		Roof surfaces subject to Impact loads Loads increased as follo	ows:	orkers		300 lbs		Beam Prima
	F.	Elevator machine Hangers for floor Vehicle and whe Special loads:	rs or balconies		3	00% 33% 30%	304R,	onveyance, pl , "Guide for M
	•••	Retaining walls Lateral equivalent fluid   Seismic load (h = heigh				35 pcf 5.5 h^2	honey segre	y cast concret /combing, pitti gation.
	G.	Vertical live load surcha Guardrails (load applied in any Top rail concentrated				00 psf 200 lbs	17 All lap 18 Concr	elding of reinfo splices shall rete piers: Pla de dowels with
		or Top rail uniform load Intermediate rail concentrated	load			50 plf 50 lbs	splice 19 The c	
	stab	structure has been designed to b ility of the structure prior to comp acts of the construction activity inc	pletion is solely the	e responsibility of the cor	ntractor. This responsibili	ity extends to all related	expos 20 Finish additio	ed to view sha elevated floo onal concrete
	shor desig	ing, use of equipment, and simila gn aspects only, not to review the onstruction procedures is not to b	ar construction pro e contractor's cons	ocedures. Review of the struction procedures. La	construction by the engir ck of comment on the pa	neer is for conformance with	deflec 21 The in	nstallation of s
;	This brac	structure utilizes moment frames ing and moment frames have be ring note: The contractor is respo	s to provide lateral en erected.	stability. Therefore, ten	porary bracing, guys, et		(scrate) 22 Expar	slab surfaces ched) surface nsion and isola strips: ASTM I
	insta for ir elem	abilities of existing structure during nstallation of new framing and fou nents.	ng construction and undations. Shoring	d due to the removal of e g shall be fully installed a	existing supporting walls and stable prior to remove	and existing framing members al of existing structural	Seala 23 Water	nt at top of joi stops: Flexib enter bulb.
•	is foi engi	site safety and construction proce r conformance with design aspec neer is not to be interpreted as a i digital files of all creation and de	cts only, not to revi pproval of those a	iew the contractor's prov spects of work.	isions for job site safety.	. Lack of comment by the	24 The cu Concr ASTM	uring and prot rete slabs sha 1 C 309 or cor
5	struc layoi	digital files of all erection and de ctural steel, steel joists, cold-form ut, and all accessories, must be s re submission to the architect for	ned metal framing submitted to and b	(wall framing), and steel be checked by the contra	deck, indicating the fabractor and subcontractor a	ricator, manufacturer, finish, and bear the checker's initials	finish 25 Cold v the av	flooring until s weather concr /erage outdoo
)	rece Defe	iving approved shop drawings sh prred submittals: Deferred submit ubmitted to the building official w	hall be at the fabric ttals are those port	cator's own risk. tions of the design that a	are not submitted at the t	time of application and are to	26 Hot we ACI 30	old Weather Co eather concre 05R, "Guide to
	appr After	oval. A. Cold-formed met r approval by the engineer, the de	tal framing eferred submittal c	documents shall be subr	nitted to the building offic	cial for approval. Deferred		ately position, orts to maintai Bolsters, cha reinforcemer
	subr Test steel	nittal items shall not be installed ( ing and inspection of concrete, si I, steel joists, steel deck, cold-for	until the deferred s teel reinforcing ba med metal framing	submittal documents ha ars (concrete and concre g, and other work are de	ve been approved by the te masonry construction) scribed in the project "Si	e building official. ), concrete masonry, structural tatement of Special		precast conc and locations eneral contrac
0	spec If fau	ections". The contractor shall rev cial inspector. Uninspected work t ulty construction procedures, or n sures, professional fees may be	that required inspe naterial, result in d	ections may be rejected defective work that requi	solely on that basis. res additional engineerin	ig time to devise corrective	29 Subm	ittals to the er
0	mea	sures, protessional tees may be held from the general contractor's ds, openings and structure in any . However, these plans do not sh	s payment. / way related to red	quirements of other (nor	n-structural) disciplines a	re shown for bidding purposes	1 All cor	asonry Notes: ncrete mason
	Load	architectural and mechanical dra	wings. Do not sca the final approved ther shown or not s	le openings. The contra d size and location of all	ctor shall obtain from the openings, equipment and	e heating and ventilating, d work to be provided for their	sectio 2 The co	5/ TMS 402)' on of these ger ompressive m
1	Load only see elect trade	trical, plumbing and other trades e for roofs, floors and walls, whet		of supporting elements	that are indicated on the sight shown on drawings	drawings. Contractor shall	3 Concr compr	oth method. rete block sha ressive streng and cement us
1 2 3	Load only see elect trade or ed For a notify	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation	nts used in design n of equipment if a	and a set The set	irai drawings and existing		5 Morta	
1 2	Load only see elect trade or ec For a notifi The with The	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall <u>field verify existir</u>	nts used in design n of equipment if a ions, elevations ar ng conditions befo	-	work. The contractor sha			
1 2 3 4 5 6	Load only. see elect trade or ec For a notif The with The note The thord	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall field verify existin d "±" that are indicated on the dra contractor and subcontractors shoughly with these plans before co	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the lates ommencing any wo	ore proceeding with any v st copies of approved pl ork.	ans and surveys and the		6 Coars cemer	ials (i.e. Portla se grout used nt, 2.25 to 3 p
1 2 3 4 5	Load only. see a elect trade or ec For a notif The with The note The thord The thord The Worl	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall <u>field verify existin</u> d "±" that are indicated on the dra contractor and subcontractors sh bughly with these plans before co se drawings are supplemented by inded to summarize basic requirer k shown as "Typical Details" appl	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the late ommencing any we y a detailed techni ments.	ore proceeding with any v st copies of approved pl ork. ical specification. The n	ans and surveys and the otes shown under certair	n categories of work are	6 Coars cemer slump Altern cemer	ials (i.e. Portla e grout used i nt, 2.25 to 3 p o. atively, fine gr nt, 2.25 to 3 p
1 2 3 4 5 6 7	Load only see elect trade or ec For a notify The with The thore Thes inter Worl the s Som the f	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall <u>field verify existin</u> d "±" that are indicated on the dra contractor and subcontractors sh bughly with these plans before co se drawings are supplemented by aded to summarize basic requirer	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the lates ommencing any wo y a detailed techni ments. ly throughout the p e building. on the architectura	ore proceeding with any v st copies of approved pl ork. ical specification. The n project as required. Wor	ans and surveys and the otes shown under certair rk shown as "Sections" s	n categories of work are shall be considered to apply for	6 Coars cemer slump Altern cemer 7 Steel Reinfo 8 Joint (	ials (i.e. Portla e grout used i nt, 2.25 to 3 p atively, fine gr nt, 2.25 to 3 p reinforcing ba prcement to be (horizontal) rei
1 2 3 4 5 6 7 8 9 20	Load only. see a elect trade or ed For a notify The with The with The thore The thore Som the f Do n	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall field verify existin d "±" that are indicated on the dra contractor and subcontractors sh oughly with these plans before co se drawings are supplemented by nded to summarize basic requirer k shown as "Typical Details" appl same and similar conditions in the base details of the work are shown of full scope of the work can be com	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the lates ommencing any wo y a detailed techni ments. ly throughout the p e building. on the architectura	ore proceeding with any v st copies of approved pl ork. ical specification. The n project as required. Wor	ans and surveys and the otes shown under certair rk shown as "Sections" s	n categories of work are shall be considered to apply for	6 Coars cemer slump Altern 7 Steel Reinfo 8 Joint ( coatin Joint r 9 Place	ials (i.e. Portla e grout used i nt, 2.25 to 3 p. atively, fine gr nt, 2.25 to 3 p. reinforcing ba preement to be (horizontal) reing, Ladder type reinforcement units while me
1 2 3 4 5 6 7 8 9 20	Load only. see trade or ed For a notif The with The thore The thore thore Som the f Do n	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall field verify existin d "±" that are indicated on the dra contractor and subcontractors sh bughly with these plans before co se drawings are supplemented by nded to summarize basic requirer k shown as "Typical Details" appl same and similar conditions in the net details of the work are shown of full scope of the work can be com not scale drawings. <u>Standards References</u> Concrete: Concrete work shall conform to ACI 301-10, "Specifications for	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the lates ommencing any we y a detailed techni ments. ly throughout the p e building. on the architectura nprehended.	ore proceeding with any v st copies of approved pl ork. ical specification. The n project as required. Wor al drawings. A careful rev s of: ete in Buildings" and	ans and surveys and the otes shown under certair rk shown as "Sections" s	n categories of work are shall be considered to apply for	6 Coars cemer slump Altern cemer 7 Steel Reinfo 8 Joint ( coatin Joint r 9 Place broker 10 Fully b	ials (i.e. Portla se grout used i nt, 2.25 to 3 pa atively, fine gr nt, 2.25 to 3 pa reinforcing ba preement to be (horizontal) rei g, Ladder type reinforcement units while mo n after initial p bed units in all lls with reinforce
1 2 3 4 5 6 7 8 9 20	Load only. see a elect trade or ed For a notif The with The with The thore The thore Som the f Do n	trical, plumbing and other trades e for roofs, floors and walls, whet quipment are not to be borne by t any mechanical equipment weigh y the architect prior to installation contractor shall verify all dimensi any work. contractor shall <u>field verify existin</u> d "±" that are indicated on the dra contractor and subcontractors sho oughly with these plans before co se drawings are supplemented by nded to summarize basic requirer k shown as "Typical Details" appl same and similar conditions in the ne details of the work are shown of full scope of the work can be com not scale drawings. <u>Standards References</u> Concrete: Concrete work shall conform to	nts used in design of equipment if a ions, elevations ar <u>ng conditions</u> befo awings. nall obtain the lates ommencing any we y a detailed techni ments. ly throughout the p e building. on the architectura prehended.	ore proceeding with any v st copies of approved pl ork. ical specification. The n project as required. Wor al drawings. A careful rev s of: ete in Buildings" and tructural Concrete". quirements of TMS 402-1	ans and surveys and the otes shown under certair rk shown as "Sections" s view and study of these o	n categories of work are shall be considered to apply for details are necessary before	6 Coars cemer slump Altern cemer 7 Steel Reinfo 8 Joint ( coatin Joint r 9 Place broker 10 Fully b 11 All cel 12 Vertice bedde 13 Conso	ials (i.e. Portla e grout used i nt, 2.25 to 3 p atively, fine gr nt, 2.25 to 3 p reinforcing ba preement to be (horizontal) re ig, Ladder typ reinforcement units while m n after initial p bed units in al

- preparation and earthwork within the perimeter of the proposed new structure, preparation of soil cification and placement of structural backfill for support of foundations and slabs-on-grade, and technical engineering report prepared for this project by Skyland Engineering, LLC and dated February ions are project requirements. Other portions of the geotechnical report are included as a reference ment, and the design team is not responsible or liable for the accuracy of the information for those
- signed to rest on inorganic, undisturbed soil or compacted granular fill having a bearing value of 4000 otechnical engineering report referenced above. Such bearing strata are anticipated at the bottom of foundation plan. All bearing strata shall be reviewed by the geotechnical engineer prior to placing bearing value hall be a crusher run stone free from soft disintegrated pieces, mud, dirt, or other injurious material. one greater than 2 inches in any one dimension and with less than 10 percent by weight passing a
- s not on solid rock shall be at least 3' 6" below finished grade. footings shall be protected from freezing and frost action during the course of construction. change at a maximum slope of one vertical on two horizontal and place lower footings first. packfilled before they are temporarily braced or before the concrete has attained its specified
- framed floor system at the top of the wall is in place. kfilled by placing fill on both sides simultaneously and to the same level. nents for attachment of superstructure to foundation shall be 3/4" diameter at 4'-0" o.c. spacing for full meter at 6'-0" o.c. spacing for crawl spaces and slabs on grade. Embed anchor bolts a minimum of 15"
- concrete. Anchor bolts are to be placed within 1'-0" of all corners on all exterior walls. All pieces of sill two anchor bolts. ree of water at all times controlled compacted fill or lean concrete (f'c=1500 psi) for over-excavation of footings.
- underground utilities in areas of excavation work. Provide adequate means of support and protection primity to sub-surface piping bottom of footings shall be at least 8" below elevation of piping unless where indicated on the drawings, provide a filtration type geotextile between crushed stone and the
- be TenCate Mirafi 140N or Propex Geotex 401. required for structural fill and slab sub-base.

m to all the requirements of ACI 301. "Specifications for Structural Concrete in Buildings" and ACI 318 for Structural Concrete", as specified in the code reference section of these general notes. weight and develop a minimum compressive strength in 28 days as follows:

p a minimum compressiv	ve strengtri in zo days as	Maximum
Weight	Minimum <u>Strength</u>	W/C Ratio_(or slump where indicated)
Normal	3,000 psi	0.55
Normal Normal Normal Normal	3,000 psi 4,000 psi 5,000 psi 4,000 psi	0.55 0.45 0.40 0.45
Normal Light	3,500 psi 3,500 psi	Slump: Slump: 4" +/- <sup>-</sup> Slump: Slump: 3" +/- <sup>-</sup>

ection of reinforcing bars, unless otherwise noted, must follow the latest ACI code and the latest ACI for Detailing Reinforced Concrete Structures". ubmitted to the engineer for review, together with laboratory reports attesting that the mixes can attain I in accordance with ACI 301 indicated above.

- e I or Type II and conform to ASTM C 150. uch as flyash or ground granulated blast- furnace slag may be blended with cement for use in the nform to ASTM C 618 and may replace cement if the following ranges for the 2 classes of flyash; Class 25%. Ground granulated blast- furnace slag shall conform to ASTM C 989 and may not exceed 50% of
- oarse aggregate shall be 3/4" and conform to ASTM C 33. Fine aggregate shall be manufactured or ource for the entire project and shall conform to ASTM C 33. vithout the engineer's written permission other than entrained air. Concrete exposed to the weather, n walls, shall contain 5% +/- 1 1/2% entrained air. Concrete exposed to the weather and to de-icing +/- 1 1/2% entrained air. Do not use air entrainment admixture for interior normal weight concrete slabs. on content in hardened concrete to the following percent by weight of cement: 1.00 for reinforced rotected from moisture, 0.30 for reinforced concrete that will exposed to moisture but not exposed to
- oncrete exposed to moisture and chlorides from deicing chemicals and salt/seawater, and 0.06 for Il have a calculated equilibrium weight of 115 pcf plus or minus 3 pcf as determined by ASTM C 567. ave maximum size of 3/4 inch and shall conform to ASTM C 330. Aggregate shall be presoaked as per cturer's instruction to ASTM A 615, Grade 60.
- orm to ASTM A 1064 with a minimum yield strength of 65 ksi. Lap one mesh size at sides and ends, and Wrap (15 mil) vapor retarder by Stego Industries LLC and shall have a water vapor permeance after Paragraphs 7.1.2 – 7.1.5) that is less than 0.01 perms and meets the requirements of Class A. Place as cturer's written instructions. shall be provided for reinforcement:

shall be provided for reinforcement.	
	Cover (inches)
and	· · · · · · · · · · · · · · · · · · ·
o earth	3
arth or weather:	
h #18 bars	2
d smaller	1 1/2
o weather or in contact with ground: ts:	
18 bars	1 1/2
nd smaller	3/4
ement, ties, stirrups, spirals	1 1/2

and protection of the concrete shall conform to the requirements of ACI 318, indicated above, and ACI *Iixing*, Transporting and Placing Concrete". Mechanical vibrators are to be used to consolidate the the reinforcing and against form surfaces and to prevent the formation of air or stone pockets, nes of weakness. However, care must be used to avoid over vibration that can lead to aggregate

#### be permitted. , in accordance with ACI 318 indicated above. te piers and walls together. Set pier reinforcing and set wall reinforcing through pier vertical bars.

hook from footing at all piers. Size and quantity of dowels to match vertical pier reinforcing (Class "B" sible for limiting pours to minimize shrinkage cracking. In general, walls shall not be poured in

30 feet without providing construction joints or control joints. The location and configuration of joints linated with the architect. and level within a tolerance of +/- 1 /4 inches to the elevation indicated on the drawings. Provide due to formwork and floor framing deflection to achieve this finished top of slab elevation. The anticipated girders is 1 1/4". Design of floor framing members includes the additional concrete for this anticipated conform to the requirements of ACI 302.1R, "Guide to Concrete Floor and Slab Construction". Interior ve a steel trowel finish. Surfaces of slabs forming the substrate for mud jobs are to have a clean textured slab surfaces are to have a broom finish unless specified on the architectural drawings.

phalt-saturated cellulosic fiber or ASTM D 1752, cork or self expanding cork Sikaflex 2c SL poly urethane elastomeric sealant. Provide cap to separate sealant from filler.

chloride (PVC) waterstop conforming to Corp of Engineers Specification CRD-C 572, with flat dumbbell concrete shall conform to the requirements of ACI 318 and ACI 308R, "Guide to Curing Concrete". cted from loss of surface moisture for not less than 7 days using a curing compound conforming to etted burlap. Curing compounds shall be compatible with any intended flooring overlay. Do not install lequately dried per the flooring manufacturer's specifications. nent: If cold weather concreting conditions exist as defined by a period of more than three days when ure, (high + low)/2, is less than 40 deg. F. the procedures outlined in ACI 306.1, "Standard Specification and ACI 306R, "Guide to Cold Weather Concreting" shall be utilized. ent: Maintain concrete temperature below 90 deg. F. at time of placement and comply with ACI 301 and

her Concreting". and secure reinforcement and anchors against displacement. Locate and support reinforcement with bar concrete cover. Do not tack weld crossing reinforcing bars. Provide bar supports as follows: s, and other devices for spacing, supporting, and fastening reinforcing bars and welded wire Do not "wet stick" reinforcement or anchors. Manufacture bar supports from steel wire, plastic, or ling to CRSI's "Manual of Standard Practice," of greater compressive strength than concrete. red embedded items, such as anchor bolts, piping sleeves, etc., for all trades shall be coordinated by her trades.

required for concrete mix designs, cement, reinforcing bars, admixtures, and aggregates. as specified

all conform to the requirements of the "Building Code Requirements for Masonry Structures (ACI 530/ Specifications for Masonry Structures (ACI 530.1/ASCE 6/ TMS 602, as specified in the code reference ngth, f'm, shall be 2,000 psi minimum. System components have been selected based on the unit

eight hollow load bearing masonry units conforming to ASTM C 90, with a minimum ultimate psi on the net area of the units. Units shall be protected from moisture absorption. nortar and grout shall conform to ASTM C 150. Masonry cement or mortar cement shall not be ning to the volumetric proportions set forth in ASTM C 270. Use 1 part Portland cement; 0.25 to 0.5

tty; and aggregate proportioned to 2.25 to 3 times the sum of the separate volumes of cementitious plus lime). Provide aggregate in loose, damp condition. Add water to produce a workable mix. and walls shall conform to the volumetric proportions set forth in ASTM C 476. Use one part Portlanc , loose sand, 1 to 2 parts 3/8" pea gravel. Add water to produce a flowable mix with an 8 to 11 inch

used that conforms to the volumetric proportions set forth in ASTM C 476 using one part Portland loose sand and adding water to produce a flowable mix with an 8 to 11 inch slump. nform to ASTM A 615, Grade 60. Reinforcing bars to be lapped 48 bar diameters at splices. gainst displacement at spacing not exceeding 200 bar diameters. t shall be hot-dipped galvanized W1.7 (9 gage) steel wire. ASTM A 951 with ASTM A 153 Class B-2 nann & Barnard, Inc., or an approved equal. Place joint reinforcing in every second course (16"o.c.). oped 6 inches at splices. and plastic. Remove and relay in fresh mortar any unit disturbed to the extent that initial bond is

### s including cross webs. bolts shall be grouted solid.

blid shall have a minimum clear opening of 3"x2-1/2". The entire perimeter of the cell shall be fully eding 12 inches in height by mechanical vibration and reconsolidate by mechanical vibration after initial ent has occurred. Grout pours exceeding 5.33 feet are high lift pours and shall require cleanouts. High unless high lift grout procedures are submitted to the engineer for review and approved by the engineer. ow the full extent of masonry lintels that may be required for doors, windows, ducts, louvers, etc. For asonry lintels and are not shown on the structural plans, see architectural drawings for location and size nings. For masonry lintel size for corresponding masonry opening size, see "Typical Masonry Lintels in ". Note, if architectural drawings do not indicate a masonry lintel for a particular opening, then provide a nless otherwise directed by the engineer.

- 15 Masonry opening lintels have been designed on the basis of arching action of the completed wall. Lintels require temporary support until the mortar has achieved the specified strength. Cover the tops of all masonry construction to protect against precipitation
- Masonry shall not be constructed in temperatures below 40 F. Provide a heat source and protection as required to maintain temperature above 40 F in accordance with ACI 530.1. Hot weather construction techniques shall be in accordance with ACI 530.1 and shall be implemented when the ambient air temperature exceeds 100 F, or 90 F if the wind speed exceeds 8 mph. Unless otherwise shown on architectural or structural drawings provide vertical control joints through concrete masonry unit walls for full wall height as follows:
- A Distance between joints should not exceed the lesser of length to height ratio of 1.5, or 25 feet At changes in wall height At changes in wall thickness – including pipe and duct chases and pilasters At and above expansion joints in foundations and floors
- At and below expansion joints in roofs and floors that bear on the wall For openings, do not locate control joints within 32 inches of openings
- Adjacent to corners of walls or at wall intersections within a distance equal to half the control joint spacing. 20 Submittals to the engineer are required for certificates of compliance for block grade and strength, grout, mortar, and reinforcing bars prior to delivery to the site.

Connections to Existing Masonry or Hardened Concrete:

- All proprietary anchoring systems (expansion, adhesive anchoring systems, etc.) to be installed into hardened concrete and masonry elements are to be installed in strict accordance with the manufacturer's instructions for drilling and preparation of holes, for spacing and edge distance requirements, and for the utilization of supplemental components for the anchoring systems such as screen tubes, doweling adhesives, etc.
- For connections to hardened concrete and masonry, contractor must locate the position of existing reinforcing bars with an R-meter or pilot holes prior to installation of anchors. Notify engineer of field conflicts prior to installation. Connections to hardened concrete shall be made with anchors conforming to ACI 318, as specified in the code reference section of these general notes, for cracked concrete, and Chapter 19 of the state building code indicated at the beginning of these general
- A. Mechanical anchors shall be either Hilti "Kwik Bolt TZ" expansion anchor. Hilti "Kwik HUS-EZ" screw anchor (use only in permanently dry, interior non-corrosive environments) Simpson "Strong Bolt 2" expansion anchor
  - Simpson "Titen HD" screw ancho, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments Dewalt "Power-Stud + SD2" expansion anchor.
  - Dewalt "Power-Stud + SD4/SD6". Type 304/316 SS expansion anchor Dewalt "Screw-Bolt+" screw anchor, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments)
- Size, embedment, spacing and edge distance of anchors shall be as indicated on the drawings. Adhesive anchor rods or reinforcing bars shall be installed in rotary hammered drilled holes with carbide drill bits using one of the following adhesive anchoring systems: Hilti "HIT-HY 200 safe set system" with hollow drill bit or Hilti "HIT-RE-500 V3" adhesive anchoring system with ISO
  - 898 Class 5.8 anchors rods (minimum yield strength = 58 ksi and minimum ultimate strength = 72.5 ksi) or ASTM A 193 Grade B7 high strength anchor rods.
  - Simpson "AT-XP" adhesive anchoring system for base material temperatures between 14 degrees and 80 degrees or Simpson "Set-3G" adhesive anchoring system for temperatures above 40 degrees, with Simpson "RFB" ASTM F 1554 Grade 36 anchor rods.
  - Dewalt DUST X+ system with "Pure110+" epoxy adhesive, standard cure or Dewalt "AC200+" two part adhesive, cold temperature cure with ASTM A 193 B7 high strength anchor rods.
  - Reinforcing bars shall conform to the requirements of the Concrete General Notes. Adhesive for reinforcing bars and anchors shall have been tested in accordance with ACI 355.4 "Qualification of
  - Post-Installed Adhesive Anchors in Concrete" and ICC-ES (ICC Evaluation Service) "Acceptance Criteria for Post-Installed Adhesive Anchors in Concrete Elements" (AC308) for cracked concrete and seismic applications.
  - Adhesive bond design strength is based upon concrete that has cured at least 21 days with a minimum compressive strength of 2,500 psi and an in-service temperature in accordance with ACI 355.4 Temperature Category B. Installation method shall be in accordance with the Manufacturer's Printed Installation Instructions (MPII
  - Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by the ACI/CRSI Adhesive Anchor Installer Certification program or equal. Unless otherwise noted on the drawings, embed anchor rods and reinforcing bars into drilled holes a minimum of 9
- anchor diameters, with a minimum edge distance of 4 inches, measured from the edge of the concrete to the centerline of the anchor/reinforcing bar. Increased embedment depths or edge distances may be required at certain locations, see plans and details. Connections to grout filled concrete masonry shall be made with either 1) Hilti standard "HAS-E" ISO 898 Class 5.8 anchor rods (minimum yield strength = 58 ksi and minimum ultimate strength = 72.5 ksi) using Hilti "HIT HY270" masonry adhesive anchoring
- systems or 2) Simpson "RFB" ASTM F 1554 Grade 36 anchor rods using Simpson "Set-XP" masonry adhesive anchoring system or 3) ASTM F1554 Grade 36 or ASTM A 193 Grade B7 high strength anchor rods using Dewalt AC100+ Gold masonry adhesive anchoring system. A. Unless otherwise noted on the drawings, embed anchor rods into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.
- Connections to hollow concrete or clay brick masonry shall be made with either 1) Hilti standard "HAS-E" ISO 898 Class 5.8 anchor rods (minimum yield strength = 58 ksi and minimum ultimate strength = 72.5 ksi) using Hilti "HIT HY270" masonry adhesive anchoring system with "HIT-SC" composite screen tubes or 2) Simpson "RFB" ASTM F 1554 Grade 36 anchor rods using Simpson "SET-XP" masonry adhesive anchoring system with Simpson "Opti-mesh" plastic screen tubes or 3) ASTM F1554 Grade 36 or ASTM A 193 Grade B7 high strength anchor rods using Dewalt AC100+ Gold masonry adhesive anchoring system with composite screen tubes. A. For anchors in hollow concrete masonry, embed anchor rods into drilled holes a minimum of 2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor.
- Increased embedment depths or edge distances may be required at certain locations, see plans and details. For anchors in hollow clay brick masonry, embed anchor rods into drilled holes a minimum of 3 1/2 inches, with a minimum edge distance of 4 inches, unless otherwise noted, measured from the edge of the masonry to the centerline of the anchor. Increased embedment depths or edge distances may be required at certain locations, see plans and details.

## Structural Steel Notes:

- Design fabrication and erection of structural steel shall conform to the American Institute of Steel Construction's "Specification for Structural Steel for Buildings", as specified in the code reference section of these general notes. Materials: Wide flange shapes: ASTM A 992 Grade 50
- American standard shapes, angles, ASTM A 36 Plates and bars: Structural steel tubing, ASTM A 500, Grade C (Fy=50 ksi) Rectangular and square Structural steel tubing, Round: ASTM A 500, Grade C (Fy=46 ksi) ASTM F3125, Grade A 325 Bolts ASTM F 1554, Grade 36 Anchor rods ASTM E 70xx, low hydrogen Welding electrode
- 3/4" diam. ASTM A 108 Shear connectors All welding shall conform to American Welding Society's AWS D1.1 "Structural Welding Code-Steel" code for arc and gas welding and be performed by a certified welder in accordance with A.W.S. standards. High strength bolts: install high-strength bolts according to Research Council on Structural Connections' (RCSC's) "Specification for Structural Joints Using High-Strength Bolts" for type of bolt and type of joint specified.

Joint type: Snug tightened unless other	wise noted.
Beam reactions shall be designed for the	e values listed in the table below unless otherwise note
Nominal beam depth	ASD reaction
W8, W10, W12	12 kips
W14, W16, W18	40 kips
W21, W24	50 kips
W27 and greater	80 kips
5	

	W27 and greater	66 Kp5	
6	calculations, signed and sealed by a qua preparation, for review by the structural "Allowable Stress Design" (ASD). Conne	alified professional engineer registered in engineer of record through the architect. ections may be designed for these values	n these plans and submitting these design the state of jurisdiction who is responsible for The reactions shown are "service" loads for using the conventional "Allowable Stress Do reference section of these general notes.

- For moment connections, notch-tough welding electrodes, complying with AWS requirements, shall be used for full penetration welds. Also for full penetration welds, provide welding tabs at beam flange edges to allow welding of full beam width. For moment connections, backing bars and weld tabs for welds need not be removed, unless testing agency requires removal to
- facilitate testing and inspection or weld tabs interfere with architectural finishes
- All moment connections utilizing full or partial penetration groove welds, shall be ultrasonic tested, as indicated in the Statement of Special Inspections, and shall be detailed to allow for such ultrasonic testing. Stud shear connectors shall be field-welded in accordance with AWS D1.1.
- Where slotted hole connections are shown, nuts shall be fastened snug tight, then untightened by one-half turn. Peen threads to prevent further loosening of the nut.
- Grout shall be nonmetallic, shrinkage-resistant grout conforming to ASTM C 1107, Grade B or C, factory-packaged, nonmetallic 12 aggregate grout, noncorrosive, nonstaining, mixed with water to consistency suitable for application and a 30-minute working time. 13 Structural steel shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 3 for Power Tool Cleaning (except for steel exposed to weather) and all steel that is not that is not receiving Spray-applied Fire Resistive Material shall painted to
- a minimum dry film thickness of 2 mils with a shop coat of Tnemec #10-99 alkyd rust inhibitive primer as manufactured by Tnemec Company, Inc. of Kansas City, MO, or an approved equal. and painted with the specified primer. All steel members and bolting exposed to weather shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 6 for Commercial Blast Cleaned and hot-dipped galvanized in accordance with ASTM A 123 and ASTM A 153. Provide bitumastic protection coating for all structural steel below grade.
- Minimum acceptable zinc coating weight shall be 2 oz./sq. Ft. See architectural specifications for finished paint if required. Clean areas where galvanizing is damaged or missing and repair galvanizing to comply with ASTM A 780. Continuous members, where indicated on the drawings, shall require either 1) the member to be furnished as one piece, or 2) if continuous member
- individual pieces are to be provided, then they shall be connected by either welding or bolting to develop the full strength of the Split cantilevers for steel beams shall be designed for the full moment capacity of the beam unless otherwise noted. Unless otherwise noted, at cantilever beam connection to top of columns, provide welded 3/4" cap plate with (4)-3/4" diameter A 325 bolts on beam gage. Provide 1/2" minimum fitted stiffeners welded at both sides of beam web. Locate stiffeners over column flange on cantilever side. 19 Bent beams are defined as beams having its full section properties throughout its length, including at the bent. Provide full
- penetration welds all around at the bent
- Provide 1/4" closure plates with seal weld at ends of all HSS members. The design of composite floor beams is based on unshored construction conditions. Fabricator shall hold a current AISC certification for "Certified Building Fabricator (BU)", (formerly known as "Standard for Steel Building Structures (STD)".)
- For miscellaneous steel, see architectural drawings. Existing steel surfaces to receive field welds shall be thoroughly cleaned and free from paint, rust, grease, etc.
- prior to the fabrication of any steel
- 25 Submittals to the engineer are required for certificates of compliance for structural steel, bolts, nuts, washers, and weld filler material All structural steel beams and columns adjacent to masonry shall have 1-1/2" x 1/8" strap anchors at 2'-8" o/c. 27 At the completion of fabrication, the fabricator shall submit a certificate of compliance stating that the work was performed in accordance with the approved contract documents, as required by Section 1704.2 of the building code indicated at the beginning of these General Notes.

# Steel Deck Notes:

- Steel deck shall be designed, fabricated, and erected in accordance with the current specification of the Steel Deck Institute. Composite formed steel floor deck to be 2" deep, 20 gage (uncoated steel thickness = 0.0358"), galvanized (G60 coating) (for exterior
- exposed applications useG90 coating), United Steel Deck Type Lok-Floor, composite floor deck as manufactured by Canam Steel Corporation, or an approved equal. Formed steel roof deck to be 1-1/2" deep, 20 gage (uncoated steel thickness = 0.0358"), galvanized (G60 coating) (for exterior exposed applications use G90 coating), wide rib, United Steel Deck "B" Deck profile ("B Lok" for deck under RTU concrete pad), as manufactured by Canam Steel Deck, Incorporated or an approved equal.
- GYM Formed steel roof deck to be 3" deep, 20 gage (uncoated steel thickness = 0.0358"), galvanized (G60 coating), deep rib, Vulcraft Steel Deck "NPA" Deck profile, as manufactured by Canam Steel Deck, Incorporated or an approved equal.
- Zinc Tile veneer back up deck shall be 9/16" deep, 20 gage (uncoated steel thickness = 0.0358"), galvanized (G60 coating), United Steel Deck "UFX" Deck profile, as manufactured by Canam Steel Deck, Incorporated or an approved equal.
- The steel deck shall be supplied in minimum lengths as required to provide a "3-span" condition. End closures, roof sumps, closures at penetrations, and all other accessories necessary for a complete installation are required.
- Composite formed steel floor deck shall be welded to supporting steel with 5/8" diameter puddle welds at 12" on center. Intermediate side connections shall bemade with #10 self tapping screws at a maximum spacing of 3'-0" on center.

ed on plan

lans and submitting these design of jurisdiction who is responsible for their tions shown are "service" loads for e conventional "Allowable Stress Design"

In lieu of puddle welds, powder actuated fasteners having the same capacity as the specified puddle welds, may be used. Fasteners shall be manufactured by Hilti, Inc. or an approved equal. Shop drawings shall be submitted for approval to the engineer indicating fastener data including size vs. steel substrate material, spacings, capacities, including diaphragm shear capacities, method of installation and program for guality assurance of installation. Formed steel roof deck shall be welded to supporting steel with 5/8" diameter puddle welds at all edge ribs plus a sufficient number of

interior ribs to limit the spacing between adjacent points of attachment to 12" on center. (8"o.c. for 3" deck) (For connection of metal roof deck to cold formed framing, use No. 10 self tapping screws at 12" on center). Intermediate side connections shall be made with No.10 selftapping screws at mid-span or 3'-0" on center, whichever is smaller. End laps of sheets shall be a minimum of 2" and shall occur over supports.

In lieu of puddle welds, powder actuated fasteners having the same capacity as the specified puddle welds, may be used. Fasteners shall be manufactured by Hilti, Inc. or an approved equal. Shop drawings shall be submitted for approval to the engineer indicating fastener data including size vs. steel substrate material, spacings, capacities, including diaphragm shear capacities, method of installation and program for quality assurance of installation. Steel deck must be protected before and after erection and all debris cleaned from its surface where concrete will be poured or roofing is to be placed

Lintel Notes: Unless otherwise noted, for lintels over doors, windows, ducts, and miscellaneous openings in non-bearing 4", 8" and 12" CMU walls

and brick walls use for each 4" of masonry: <u>Angle size</u> <u>Bearing each end</u> 3 1/2" x 3/8" flat plate 3 1/2" x 4" x 5/16" (LLV) 3'-6" 5'-0" 3 1/2" x 5" x 5/16" (LLV) 3 1/2" x 6" x 5/16" (LLV) 6'-0" 3 1/2" x 6" x 3/8" (LLV) 8'-0" Openings in non-bearing walls masonry where no specific lintels or lintel sizes are indicated shall have 8" bearing at each end and shall have the following sizes: <u>Max. M.O.</u> Wall thickness Lintel size W8x24 9"-13" 9'-0" W8x31 + 5/16" plate 12'-0" W8x28 W8x35 + 5/16" plate 12'-0" 9"-13" Plates indicated in above lintels shall have a width 1" less than the wall thickness and a length 1" less than the masonry opening Provide 7 1/2" x 5/8" x 0'-7 1/2" bearing plates on 3/4" grout bed with (2)-5/8" diameter x 6" long welded anchor studs at 3" o.c. unless otherwise noted. Field weld bottom flange of beam to bearing plate with 1/4" weld at each side of flange. For 6" masonry partitions use WT 7x13 for spans up to 7'-0" with 6" bearing each end. For 10" masonry partitions use WT 7x21.5 for spans up to 7'-0" with 8" bearing each end. Fill the first two courses directly under bearing with grout for 16" length. See architectural and mechanical drawings for size and location of openings. When openings occur in bearing walls or the height of masonry above the lintel is less than the opening width or when a control joint is located directly above or within 16" of the jamb opening and drawings do not otherwise indicate a specific lintel design, consult with the architect to confirm lintel requirements. Lintels over adjacent openings with piers between less than 2'-8" wide shall be continuous over piers. Masonry units of such piers shall be filled with grout for full story height.

Connect lintel to structural steel columns when there is less than 16" of masonry between the masonry opening and the outermost face of column. Lintels supporting exterior masonry shall be hot-dipped galvanized. See structural steel notes. 11 Lintels for openings in walls: These plans do not show the full scope of steel lintels required for new wall openings for doors, windows, ducts, louvers, etc.. For masonry opening size and location of all wall openings, see architectural and mechanical drawings.

12 See architectural drawings for any precast concrete lintels that are required. Shop drawing submittals for precast concrete lintels shall be designed and certified by a professional engineer licensed in the jurisdiction of the project. The lintels shall be designed to carry the code required gravity and lateral loads.

Cold-Formed Metal Framing Notes:

1. Design, fabrication and erection of cold-formed metal framing shall conform to the American Iron and Steel Institute's "Specification for the Design of Cold-formed Steel Structural Members", current edition as referenced by the applicable building code, and the specifications of the manufacturer of the cold-formed metal framing.

bracing and bridging shall be manufactured per ASTM C 955. All studs, joists and accessories shall be formed from steel that conforms to the requirements of ASTM A 1003 with a yield as follows: 16 ga.(0.0598") or heavier 50 ksi 33 ksi

All studs, joists and accessories shall be galvanized with a minimum G-60 coating. Touchup paint: Immediately after fabrication and erection, clean welds, fasteners, and damaged galvanized surfaces. Touchup and repair surfaces with galvanized repair paint in accordance with ASTM A 780, applied by brush or spray to provide minimum dry film thickness of 2.0 mils.

Cold framed metal sizes and attachments shall not be less than those indicated on the contract documents. Lyvnere traming systems are not shown on the drawings, provide delegated design of framing systems for the design loads indicated in the contract documents. Provide for movement of framing members without damage or overstressing, sheathing failure, connection failure, undue strain on fasteners and anchors, or other detrimental effects when subject to a maximum ambient temperature of 120 deg F. Design framing system to maintain clearances at openings, to allow for construction tolerances, and to accommodate live load

deflection of primary building structure for an upward and downward movement of L/360 for floors. Connections shall be accomplished with self-driving screws or welding so that the connection meets or exceeds the design loads required at that connection. All connections shall be made using a minimum of four (4) No. 10-16 screws, unless otherwise shown on drawings. 10. Screw spacing and edge distance shall not be less than 1"

Minimum connection angle thickness shall be 16 gage, but no thinner than the material of the members that are being connected, 12. Welding shall conform to Structural Welding Code D1.1 and Specification for Welding Sheet in Structures E1.3 of the American Welding Society and be performed by a certified welder in accordance with AWS standards.

14. Joists shall be located directly over bearing studs or a load distribution member shall be provided to transfer loads.

16. Avoid holes at ends of members. However, should holes occur, provide additional reinforcing at the ends of the member where holes occur, unless otherwise noted.

17. Provide lateral blocking, bridging, and web stiffeners for vertical and horizontal framing members, and other framing members as required and in accordance with manufacturer's specifications or recommendations, unless indicated otherwise on the drawings. 18. Track: 16 gage minimum. securely anchored to adjacent structure or member.

19. All framing components: cut squarely or at an angle to fit squarely against abutting members. All members: held firmly in position until properly fastened, erect member level, plumb, and true to line and to dimensions and elevations indicated. 20. Studs: Seated squarely in the track with the stud web and flanges abutting the track web, and securely attached to the flanges or web of both tracks.

23. Shape designations and section properties are based on the universal system for light gage steel framing members. For example, "600S162-54" denotes 6" deep, 1-5/8" wide flange, stud, 54 mils(=16 gage) thickness. 24. Submittals to the engineer are required for certificates of compliance for framing members (studs, joists, tracks, etc.), screws, and accessories (connection clips, stiffeners, etc.) prior to delivery to the site. 25. Shop drawings: Show layout, spacing, sizes, thickness, material specification, and types of cold-formed metal framing. Show

strapping, bracing, bridging, splices, accessories, connection details, and attachment to adjoining work. Regarding delegated design of cold formed framing, the cold-formed metal engineer and fabricator shall design only members and connections that are not shown on the drawings and shall submit calculations of all designs, signed and sealed by a qualified professional engineer registered in the state of jurisdiction who is responsible for their preparation. They shall not design any

13. Temporary bracing shall be provided and left in place until work is permanently stabilized. 15. All built-up framing members shall be of welded construction, unless otherwise noted.

21. Splices in studs and other framing components: Not permitted.

22. <for the shape designations that you are using on the drawings, choose one of the following.>

All studs and/or joists and accessories shall be of the type, size, steel thickness and spacing shown on the drawings. Studs, tracks,

# 18 ga (0.0474") or lighter

fastening and anchorage details, including mechanical fasteners. Show reinforcing channels, opening framing, supplemental framing,

changes to the cold-formed metal framing, including sizes, gage, spacing and connections, that are indicated on the drawings.

Do not scale openings. For steel lintel size for corresponding masonry opening size, see notes, above, unless otherwise noted on

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