

BULLETIN # 0 4 R



DATE: March 11, 2025

TO: Piazza Inc.
Attn: Phil Pignatelli

CC: Jordan Medina, Hill Intl.,

FROM: Brian Dunn, AIA
KG+D Architects, PC

PROJECT: Harrison Recreation & Community Center – PHASE 2
Harrison, NY

RE: Additional Work from Phase 1 to Phase 2 Contractor

Message:

The following represents additional Work that is required of the Phase 2 Contractor for items that require correction from Phase 1.

Upper Level Alterations:

The Drawing sheets referenced in this section only are contained in the Phase 1 Documents.

Phase 2 Contractor to provide proof rolling of structural base material per the Skylands report attached to confirm compaction and perform corrective measures, if necessary, based on third party observation of proof rolling. Contractor to provide 6" of gravel sub base at the slab on grade not shown on the Phase 2 Bid Documents. Refer to Bulletin 2 Skylands Engineering Letter.

Phase 2 Contractor to re-excavate the area to the north of the 6 wall and stockpile the removed Item 4 material installed by the Phase 1 Contractor for re-use. Phase 2 Contractor to re-excavate to the point where the Geotech engineer determines that the material meets compaction requirements, extending down to the level of the footings if necessary. Re-excavation to extend the length of the 6 wall footings up to and including where they step up to the SOG level at column lines A and G as shown on S-101. Extents of excavation going north of the 6 wall to be determined by the Geotech engineer. The intent is to limit the amount of excavation while maintaining the correct compaction. Phase 2 Contractor to remove the incorrectly installed storm drain pipe including the vertical pipe located adjacent to the A wall. Contractor to leave corrugated pipe stub penetrating the A wall once the 90 degree elbow is removed.

KG+D Architects, PC

285 Main Street, Mount Kisco, New York 10549
914.666.5900 kgdarchitects.com

Phase 2 Contractor to install the specified cast iron storm pipes as shown on the Phase 1 Documents with the appropriately pitched drain pipe behind the 6 wall as originally designed. Connect the cast iron piping to the in place corrugated pipe on the inside of the A wall with a Fernco coupling. Phase 2 Contractor to provide the drain piping as shown on M-001 with vertical drops located at the 6 wall penetrations. Phase 2 Contractor to remove the grouted in place pipes at the wall penetrations and replace with the sleeves shown on 3/M-701. If the in place backfill compaction allows for a shallower excavation the vertical drops can be sloped to drain to the main drain pipe to reduce the necessary excavation.

Lower Level Occupied Space Alterations:

Lower Level slab elevations raised from -12'-0" and -12'-6" below First Floor to 11'-6" below First Floor. Phase 2 Contractor to provide additional 6" of gravel below slab at all areas. Phase 2 Contractor to provide vapor barrier at all areas except Parking Area B01. Contractor to provide Domestic and Fire Service valves not installed in Phase 1. Ref Dwg Phase 1 M-101.

Parking Garage Alterations:

Provide 3 under slab drain pipes running east to west directly under the slab per the Skylands report dated February 5, 2025. Contractor to core the side of the trench drain to accept the added under slab drain pipes. Provide striping to replace the eliminated curb at the perimeter of the Parking area. Provide additional gravel below slab, 6" at occupied space and walkways and approximately 12" at the Parking Area. Contractor to delete the vapor barrier below the Parking Area only. Please note to the south of Stair B entrance ramp to pitch down to trench drain. Ref Dwg A 201.

Stair Alterations:

Stairs A, B, and D have been modified due to the change in slab elevation at the Lower Level. Contractor to increase the gravel base by 6" as noted above. The stairs contain one less riser and tread and the railing length has been reduced. Refer to Drawings A201 and A701.

General Alterations:

Correct anchor bolts where not within construction tolerances in 19 locations. Phase 2 Contractor to replace out of tolerance bolts per Structural recommendations.

Cc: File

Encl:

Town / Village of Harrison

HARRISON RECREATION & COMMUNITY CENTER

270 Harrison Avenue
Harrison, NY 10528

JULY 12, 2023

KG+D Project No.
2020-1005

DESIGN TEAM

PHASE 1 - ISSUE FOR BID



ARCHITECT
KG+D Architects, PC
285 Main Street
Mount Kisco, NY 10549
phone: 914.666.5900

SYSTEMS ENGINEER
OLA Consulting Engineers, PC
50 Broadway - Suite 2
Hawthorne, NY 10532
phone: 914.747.2800

STRUCTURAL ENGINEER
The Di Salvo Engineering Group
93 Lake Ave - Suite 201
Danbury, CT 06810
phone: 203.490.4140

CIVIL ENGINEER
**Woodard & Curran Engineering
& Geological Services, PA, PC**
800 Westchester Ave - Suite N507
Rye Brook, NY 10573
phone: 914.246.2931

DRAWING LIST

G-000	COVER SHEET	STRUCTURAL		MECHANICAL	
GENERAL		S-001	GENERAL STRUCTURAL NOTES	M-001	SYMBOLS, ABBREVIATIONS & NOTES
G-100	EXISTING CONDITIONS & REMOVALS PLAN	S-100	LOWER LEVEL FOUNDATION PLAN	M-101	PARKING FLOOR PLAN PHASE 1
		S-101	FIRST FLOOR FOUNDATION PLAN	M-102	FIRST FLOOR PLAN PHASE 1
SURVEY		S-201	FOUNDATION SCHEDULES AND TYPICAL DETAILS	M-201	MECHANICAL SITE PLAN
V-001	TOPOGRAPHIC SURVEY	S-202	FOUNDATION SCHEDULES AND TYPICAL DETAILS	M-202	ELECTRICAL SITE PLAN
CIVIL		S-210	FOUNDATION SECTIONS	M-701	DETAILS
C-001	CONSTRUCTION NOTES & LEGENDS	S-211	FOUNDATION SECTIONS		Grand total: 32
C-100	GRADING PLAN	ARCHITECTURAL			
C-101	LAYOUT PLAN	A-200	LOWER LEVEL PLAN & FOUNDATION DETAIL		
C-200	UTILITIES PLAN	A-201	1st FLOOR PLAN - For Reference Only		
C-300	EROSION & SEDIMENT CONTROL PLAN	A-301	EXTERIOR ELEVATIONS - For Reference Only		
C-400	CONSTRUCTION DETAILS	A-302	EXTERIOR ELEVATIONS - For Reference Only		
C-401	CONSTRUCTION DETAILS				
C-402	CONSTRUCTION DETAILS				
C-403	CONSTRUCTION DETAILS				
C-404	CONSTRUCTION DETAILS				
C-405	CONSTRUCTION DETAILS				
C-501	SITE DETAILS				

COVER SHEET

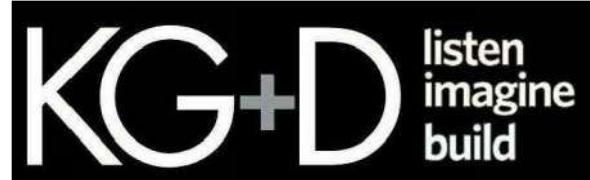
G-000

HARRISON
RECREATION &
COMMUNITY
CENTER

New Construction - Phase 1

Town / Village of Harrison

270 Harrison Avenue
Harrison, NY 10528



KG+D ARCHITECTS, PC
285 MAIN STREET • MOUNT KISCO, NEW YORK 10549
P:914.666.5900 KGDARCHITECTS.COM

CONSTRUCTION DOCUMENTS

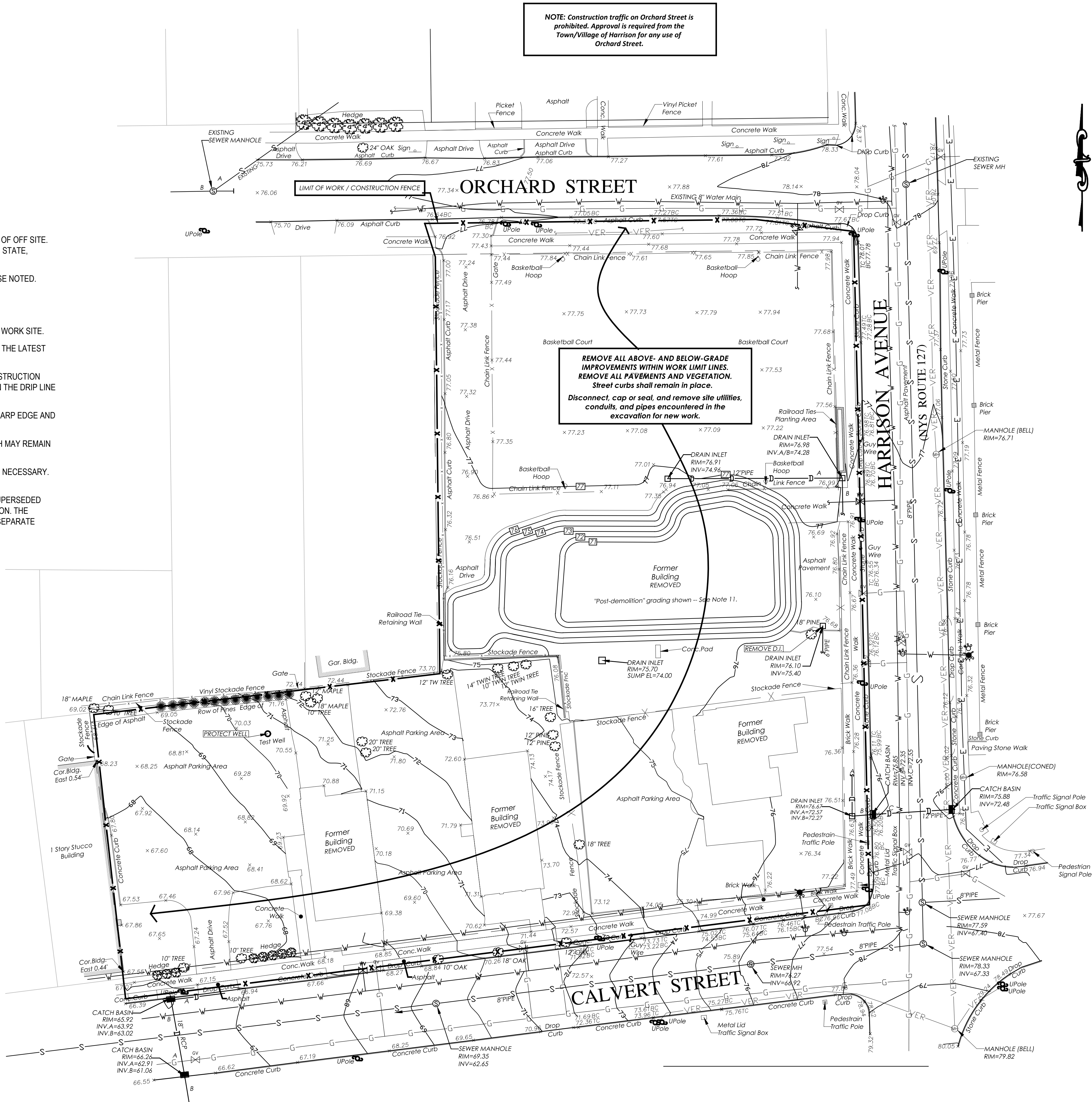
GENERAL NOTES:

- THE CONTRACTOR IS CAUTIONED TO VERIFY ALL DIMENSIONS, GRADES AND EXISTING CONDITIONS AT THE SITE PRIOR TO ORDERING MATERIALS OR COMMENCING WORK. ALL DIMENSIONS SHOWN ARE APPROXIMATE ONLY.
- LOCATIONS OF UNDERGROUND UTILITIES AND SUBSURFACE DRAINAGE SHOWN ARE APPROXIMATE ONLY. ALL EXISTING UTILITY LINES MAY NOT BE SHOWN ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IDENTIFY AND DETERMINE THE EXACT LOCATION OF ANY UTILITY OR DRAINAGE LINE THAT MAY BE ENCOUNTERED DURING CONSTRUCTION AND TO PROTECT ALL LINES INTENDED TO REMAIN.
- THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER UPON DISCOVERY OF ANY UNDERGROUND UTILITY OR DRAINAGE LINE NOT SHOWN ON THE DRAWINGS.
- LIMIT OF WORK LINE (LOW) IS COINCIDENT WITH PROPERTY LINE UNLESS OTHERWISE SHOWN. EXTEND CONTRACT LIMIT LINE TO INCLUDE UTILITY WORK.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORING ALL CONSTRUCTION DAMAGE INSIDE AND OUTSIDE THE CONTRACT LIMITS TO THEIR ORIGINAL NEW CONDITION.
- SOURCE OF BASE TOPOGRAPHIC DATA: EXISTING CONDITIONS TOPOGRAPHIC SURVEY BY TC MERRITTS LAND SURVEYORS LAST DATED FEBRUARY 18, 2019. NOTE THAT THE BUILDINGS SHOWN ON THE 2019 SURVEY ARE NOW REMOVED IN THEIR ENTIRETY AND KNOWN UTILITIES CAPPED.

REMOVALS NOTES:

- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE OWNER'S REPRESENTATIVE ON ALL MATERIALS TO BE REMOVED AND RECYCLED/DISPOSED OF OFF SITE. ALL HANDLING OF MATERIALS TO BE REMOVED, RECYCLED, OR DISPOSED OF MUST BE DONE SO IN A SAFE, LEGAL MANNER, IN ACCORDANCE WITH ALL LOCAL, COUNTY, STATE, FEDERAL AND ANY OTHER APPLICABLE REGULATIONS.
- ALL ITEMS REQUIRING REMOVAL SHALL BE REMOVED TO FULL DEPTH TO INCLUDE BASE MATERIAL AND FOOTINGS OR FOUNDATIONS AS APPLICABLE UNLESS OTHERWISE NOTED.
- EXISTING ITEMS INDICATED TO BE SALVAGED OR STOCKPILED SHALL BE CAREFULLY REMOVED AND STORED AS DIRECTED BY THE OWNER'S REPRESENTATIVE.
- EVERY CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE INSIDE AND OUTSIDE THE LIMIT OF WORK DUE TO ITS CONTRACT OPERATIONS.
- CONTRACTOR SHALL PROTECT AND SUSTAIN IN NORMAL SERVICE ALL EXISTING UTILITIES, STRUCTURES, EQUIPMENT, ROADWAYS AND DRIVEWAYS SURROUNDING THE WORK SITE.
- CONTRACTOR SHALL INSTALL AND MAINTAIN NECESSARY TRAFFIC CONTROL DEVICES, DRUMS, DELINEATORS, SIGNS, FENCES, AND BARRICADES IN ACCORDANCE WITH THE LATEST EDITION OF THE NATIONAL MUTCD WITH NYS SUPPLEMENT TO PROPERLY PROTECT WORK, EQUIPMENT, PERSONS, AND PROPERTY FROM DAMAGE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING TREES AND PLANTS OUTSIDE THE LIMIT OF WORK FOR THE DURATION OF THE CONSTRUCTION PERIOD AND SHALL, AT A MINIMUM, INSTALL TREE PROTECTION FENCE AS SHOWN AND SPECIFIED. VEHICLES, EQUIPMENT OR MATERIALS SHALL NOT BE STORED WITHIN THE DRIP LINE OR WHERE DAMAGE MAY RESULT TO TREES TO BE SAVED.
- AT ALL LOCATIONS WHERE EXISTING PAVEMENT ABUTS NEW CONSTRUCTION, THE EDGE OF THE EXISTING PAVEMENT SHALL BE SAW CUT TO A CLEAN, SMOOTH AND SHARP EDGE AND PROTECTED UNTIL ABUTTING MATERIALS ARE INSTALLED.
- EXISTING CURB TO BE REPLACED SHALL BE COMPLETELY REMOVED BETWEEN EXISTING EXPANSION JOINTS, EXCEPT FOR EXPANSION JOINT REINFORCING RODS WHICH MAY REMAIN IN PLACE.
- LIMIT OF WORK LINE IS COINCIDENT WITH THE PERIMETER PROPERTY LINE. LIMIT OF WORK SHALL EXTEND TO INCLUDE OFF-SITE UTILITIES OR OTHER CONNECTIONS AS NECESSARY. WORK LIMIT / CONSTRUCTION FENCE SHALL BE RESTORED TO THE PERIMETER SHOWN AT THE END OF EVERY WORK DAY.
- THE POST-DEMOLITION GRADING AND OTHER EXISTING CONDITIONS INFORMATION SHOWN ON THIS PLAN IS PROVIDED FOR BIDDING PURPOSES ONLY AND SHALL BE SUPERSEDED AND REPLACED BY AN AS-BUILT SURVEY NOT AVAILABLE AT THE TIME OF BIDDING BUT TO BE PROVIDED BY THE OWNER PRIOR TO BEGINNING OF PHASE 1 CONSTRUCTION. THE POST-DEMOLITION GRADING REPRESENTS THE GRADES AND STABILIZED CONDITIONS TO BE PRESENT AT THE CONCLUSION OF BUILDING DEMOLITION WORK UNDER A SEPARATE CONTRACT.

NOTE: Construction traffic on Orchard Street is prohibited. Approval is required from the Town/Village of Harrison for any use of Orchard Street.



NOTE: ALL LOCAL, REGIONAL, ARRANGEMENTS AND PLANS INDICATED OR REPRESENTED BY THE DRAWING ARE OWNED BY AND ARE THE PROPERTY OF KG+D ARCHITECTS, PC. NO PART OF THIS DRAWING OR ANY INFORMATION CONTAINED HEREIN IS TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE WRITTEN PERMISSION OF KG+D.

WRITTEN DIMENSIONS ON THIS DRAWING SHALL HAVE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTOR SHALL VERIFY ALL ACTUAL DIMENSIONS AND CONDITIONS ON THE JOB AND THE ARCHITECT MUST BE NOTIFIED OF ANY VARIATIONS FROM DIMENSIONS AND CONDITIONS SHOWN. SHOP DETAILS MUST BE SUBMITTED TO THIS OFFICE FOR APPROVAL BEFORE PROCEEDING WITH FABRICATION.

ALTERATIONS BY ANY PERSON IN ANY WAY OF ANY ITEM CONTAINED ON THIS DOCUMENT UNLESS ACTING UNDER THE DIRECTION OF THE LICENSED ARCHITECT WHOSE PROFESSIONAL SEAL IS AFFIXED HERETO, IS A VIOLATION OF TITLE 16, SECTION 16-1.1 OF THE NEW YORK STATE LAW.

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Professional Seal

1 07/12/2023 ISSUE FOR BID - PHASE 1

No. Date Issue

Sheet Title

EXISTING
CONDITIONS &
REMOVALS PLAN

Job No. 2020-1005 Date 07/12/2023

Scale 1" = 20' Drawn / Checked

Sheet Number

G-100



Only copies from the original of this topography map marked with an original of the Land Surveyors embossed seal shall be considered to be true, valid copies.

Unauthorized alteration or addition to a map bearing a licensed Land Surveyors seal is a violation of Section 7209, Subdivision 2 of the New York State Education Law.

Possession only where indicated.

Adjacent property lines and easements not surveyed or certified. Access to adjacent rights of way, easements and public or private lands not guaranteed or certified.

Underground utilities shown hereon are approximate and should be verified before excavating. Additional underground utilities are not shown or certified. Encroachments and structures below grade, if any, not shown or certified.

Subject to covenants, easements, restrictions, conditions and agreements of record.

This map is prepared to show topography only and is not to be used for title transfer purposes. Map may not be certified to title companies and/or banks.

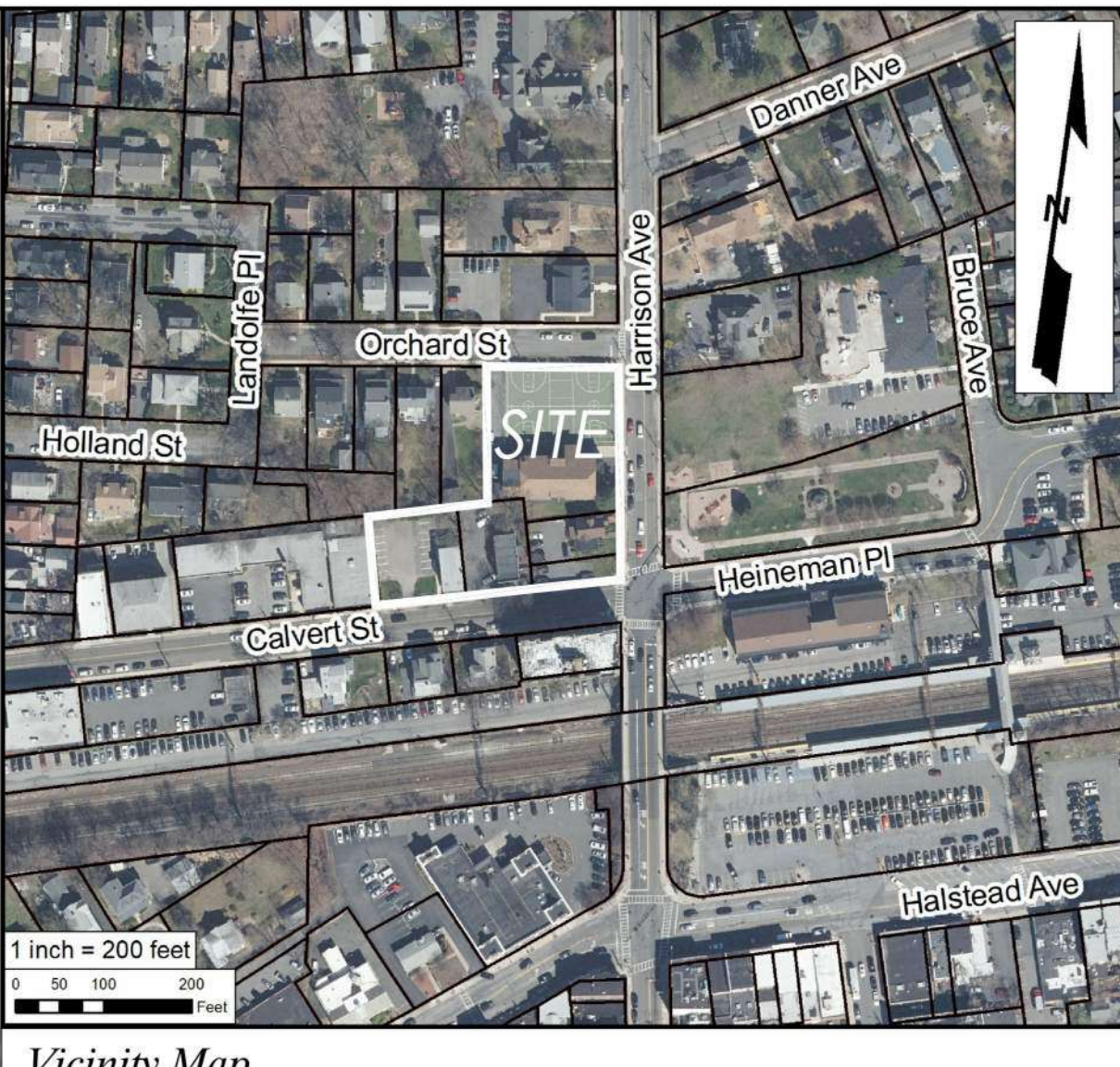
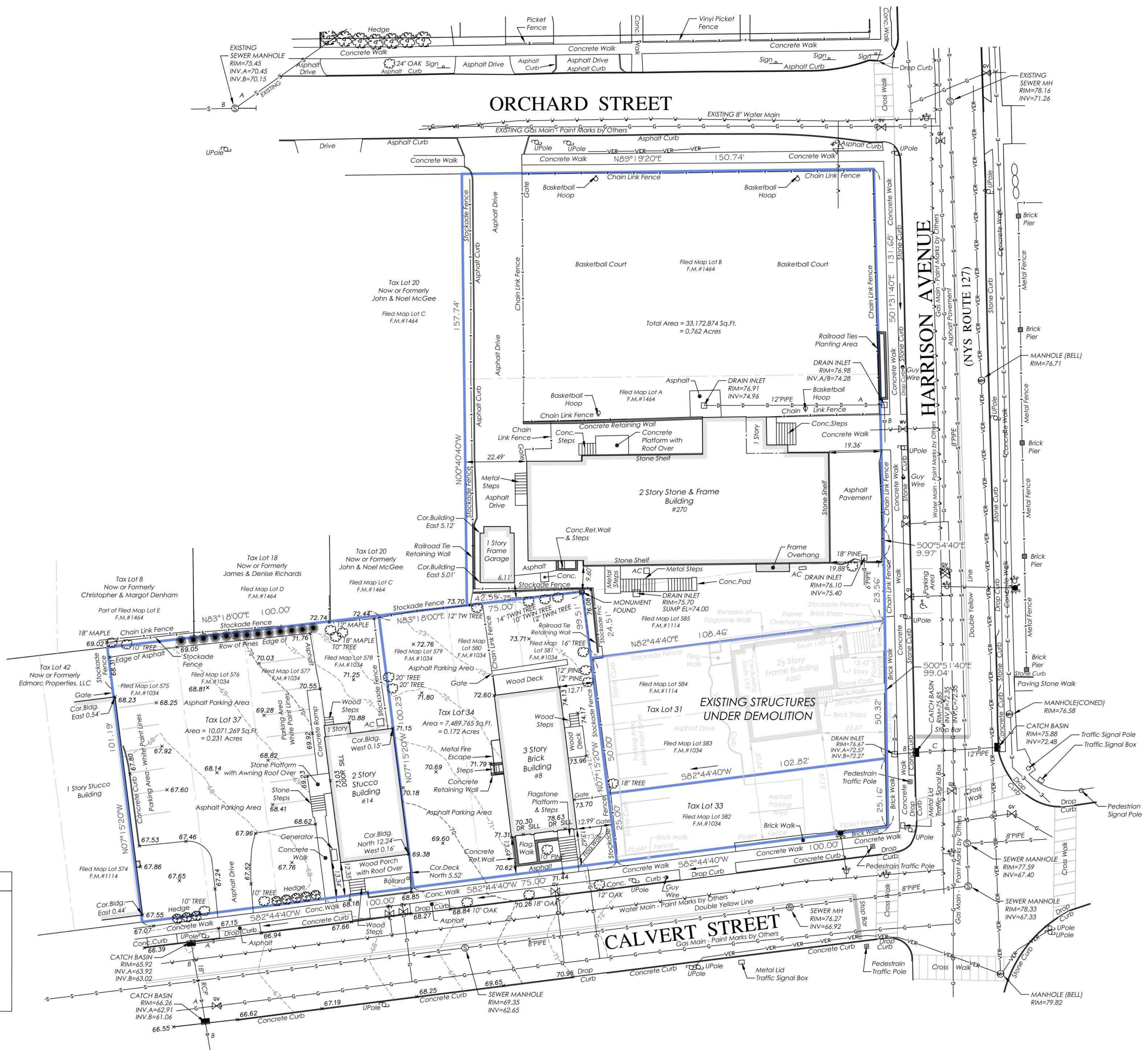
Tree species shown hereon to be verified by a licensed arborist and are not certified by surveyor.

Elevations shown hereon generally in accordance with North American Vertical Datum 88.

LEGEND	
MANHOLES	
DRAIN INLETS/CATCH BASINS	
GAS VALVE/GAS BOX	
WATER VALVE	
HYDRANT	
WATER SHUT OFF VALVE	
UTILITY POLE	
LIGHT POST	
TOP OF CURB	
BOTTOM OF CURB	

LEGEND	
W	WATER MAIN
D	DRAINAGE PIPE
G	GAS MAIN
S	SANITARY SEWER MAIN
E	ELECTRICAL LINE
VER	VERIZON LINE

Below ground utilities shown hereon as per delineation by others. Paint marks set in street by others & drawings supplied by utility companies. Additional utilities may exist, not shown.



Vicinity Map

TAX LOTS 31 AND 33
Premises hereon being Lots 582 and 583 as shown on a certain map entitled, "2nd Map of Brentwood Plaza."
Said map filed in the Westchester County Clerk's Office, Division of Land Records July 27, 1892 as map number 1034.
Premises hereon being Lot 584 as shown on a certain map entitled, "4th Map of Brentwood Plaza."
Said map filed in the Westchester County Clerk's Office, Division of Land Records May 31, 1894 as map number 1114.

Surveyed in accordance with Deed Control Number 572483257.

Premises shown hereon designated on the Town of Harrison Tax Maps as Block 253, Lots 31 and 33.

Property Address: 260 Harrison Avenue
Harrison, NY 10528

TAX LOT 28
Premises hereon being Lot 585 as shown on a certain map entitled, "4th Map of Brentwood Plaza."
Said map filed in the Westchester County Clerk's Office, Division of Land Records May 31, 1894 as map number 1114.
Premises hereon being Lots A and B as shown on a certain map entitled, "Map of Lots Belonging to Mary B. Pundy."
Said map filed in the Westchester County Clerk's Office, Division of Land Records February 23, 1905 as map number 1464.

Premises shown hereon designated on the Town of Harrison Tax Maps as Block 253, Lot 28.

Property Address: 270 Harrison Avenue
Harrison, NY 10528

TAX LOT 34
Premises hereon being Lots 579, 580 and 581 as shown on a certain map entitled, "2nd Map of Brentwood Plaza."
Said map filed in the Westchester County Clerk's Office, Division of Land Records July 27, 1892 as map number 1034.

Surveyed in accordance with Deed Control Number 532243364.

Premises shown hereon designated on the Town of Harrison Tax Maps as Block 253, Lot 34.

Property Address: Calvert Street
Harrison, NY 10528

TAX LOT 37
Premises hereon being Lots 575, 576, 577 and 578 as shown on a certain map entitled, "2nd Map of Brentwood Plaza."
Said map filed in the Westchester County Clerk's Office, Division of Land Records July 27, 1892 as map number 1034.

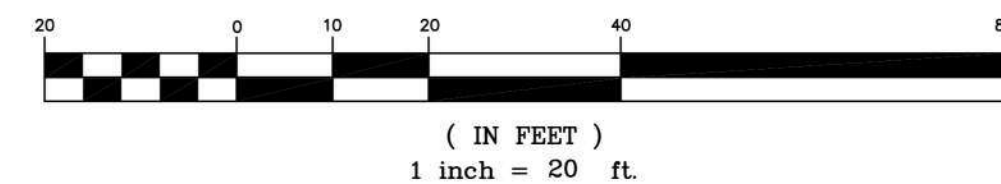
Premises shown hereon designated on the Town of Harrison Tax Maps as Block 253, Lot 37.

Property Address: 14 Calvert Street
Harrison, NY 10528

TOPOGRAPHY OF PROPERTY
PREPARED FOR
TOWN OF HARRISON
SITUATE IN THE
TOWN OF HARRISON
WESTCHESTER COUNTY, NEW YORK

SCALE: 1" = 20'

GRAPHIC SCALE



Project: Ref. 10-216 17-336	Field Survey By: BC/CR
Drawn By: DA	Checked By: DM

HARRISON RECREATION & COMMUNITY CENTER

New Construction - Phase 1

Town / Village of Harrison

270 Harrison Avenue
Harrison, NY 10528



CONSTRUCTION DOCUMENTS

NOTE: ALL DESIGN, ARRANGEMENTS AND PLANS INDICATED ON THIS DRAWING ARE THE PROPERTY OF KG+D ARCHITECTS, PC. NO PART OF THIS DRAWING SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE WRITTEN PERMISSION OF KG+D ARCHITECTS, PC.

WRITTEN DIMENSIONS ON THIS DRAWING SHALL HAVE PRECEDENCE OVER UNWRITTEN DIMENSIONS. CONTRACTOR SHALL VERIFY ALL ACTUAL DIMENSIONS AND CONDITIONS ON THE JOB AND THE ARCHITECT MUST BE NOTIFIED OF ANY VARIATIONS FROM DIMENSIONS AND CONDITIONS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DIMENSIONS SHALL BE IN FEET AND INCHES, UNLESS OTHERWISE SPECIFIED.

No.	Date	Issue
3	07/12/2023	ISSUE FOR BID - PHASE 1

TOPOGRAPHIC SURVEY

Job No. 2020-1005	Date 07/12/2023
Scale AS NOTED	Drawn / Checked ..
Sheet Number	

V-001

GENERAL NOTES:

- EXISTING CONDITIONS SHOWN HEREON HAVE BEEN TAKEN FROM A FIELD SURVEY PREPARED BY TC MERRITTS LAND SURVEYORS, FOR THE TOWN OF HARRISON, WESTCHESTER COUNTY, NEW YORK, LAST REVISED FEBRUARY 18, 2019.
- TC MERRITTS LAND SURVEYORS IS LOCATED AT THE FOLLOWING ADDRESS:
594 BEDFORD ROAD
PLEASANTVILLE, NEW YORK 10570
(914) 769-8003
- EXISTING UTILITIES SHOWN ON THESE PLANS WERE COMPILED FROM A FIELD SURVEY PREPARED BY TC MERRITTS LAND SURVEYORS. THESE PLANS DO NOT NECESSARILY DEPICT THE EXACT LOCATIONS OF ALL UTILITIES, WHICH MAY EXIST AT THIS TIME WITHIN THE SURVEY LIMITS. THERE MAY BE EXISTING LINES OTHER THAN THOSE INDICATED. THE CONTRACTOR SHALL FIELD LOCATE ALL EXISTING UTILITIES USING GROUND PENETRATION RADAR (GPR) PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ENGINEER IN WRITING. THE CONTRACTOR SHALL CONTACT THE RESPECTIVE UTILITY COMPANIES RELATIVE TO THE LOCATIONS AND ELEVATIONS OF THESE LINES. NO SEPARATE OR ADDITIONAL COMPENSATION WILL BE ALLOWED TO THE CONTRACTOR DUE TO ANY VARIANCE BETWEEN THE DATA SHOWN ON THE PLANS AND ACTUAL FIELD CONDITIONS ENCOUNTERED. THE CONTRACTOR IS RESPONSIBLE FOR HIS/HER OWN DETERMINATION AS TO THE TYPE AND LOCATION OF THE EXISTING UTILITIES AS MAY BE NECESSARY TO AVOID THEIR DAMAGE AND TO FACILITATE THE PROPOSED CONNECTION(S).
- LAND OUTSIDE THE PROPOSED LIMIT OF WORK SHALL NOT BE DISTURBED BY THE CONTRACTOR.
- PRIOR TO CONSTRUCTION THE ENGINEER AND CONTRACTOR WILL REVIEW THE PERMITTED LIMITS OF WORK.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING ALL CONTROL POINTS AND BENCH MARKS NECESSARY FOR THE WORK.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RELOCATIONS THAT CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE SITE DRAWINGS, INCLUDING BUT NOT LIMITED TO ALL UTILITIES, STORM INFRASTRUCTURE, SIGNS, UTILITY POLES, FENCES, ETC. AS REQUIRED. ALL WORK SHALL BE IN ACCORDANCE WITH THE GOVERNING AUTHORITY'S SPECIFICATIONS AND SHALL BE APPROVED BY SUCH. ALL COSTS SHALL BE INCURRED BY THE CONTRACTOR.
- THE CONTRACTOR SHALL CALL "DIG SAFELY, NEW YORK" AT 811 AT LEAST 72 HOURS PRIOR TO EXCAVATION IN ACCORDANCE WITH LOCAL, STATE AND FEDERAL SAFETY CODES.
- THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL SAFETY CODES, REGULATIONS, LEGAL REQUIREMENTS, PERMIT CONDITIONS, ETC. PERMIT CONDITIONS RELATE TO THOSE OUTLINED IN THE NYSDEC (NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION) SPDES GENERAL PERMIT 0-20-001, AND ALL PERTINENT TOWN/VILLAGE OF HARRISON PERMITS.
- THE CONTRACTOR SHALL TAKE ADEQUATE PRECAUTIONS TO PROTECT ALL WALKS, STREETS, PAVEMENTS, CURBING, TREES, PLANTINGS, LAWN, FENCING TO REMAIN ON OR OFF THE PREMISES, AND SHALL REPAIR AND REPLACE AT HIS/HER OWN EXPENSE AS DIRECTED BY THE ENGINEER ANY ITEMS DAMAGED AS A RESULT OF THE CONTRACTOR'S WORK.
- THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING AND SHORING OF ALL EXCAVATIONS IN ACCORDANCE WITH THE REQUIREMENTS OF ALL GOVERNING CODES AND REGULATIONS.
- ALL SURFACES TO REMAIN THAT ARE DISTURBED BY THIS WORK SHALL BE RESTORED TO THEIR ORIGINAL CONDITION, AS DETAILED, OR AS SPECIFIED BY THE ENGINEER.
- SAWCUTS IN EXISTING PAVEMENT SHALL BE SMOOTH AND STRAIGHT.
- WORK WITHIN PUBLIC RIGHT-OF-WAY SHALL COMPLY WITH APPLICABLE MUNICIPAL AND STATE REQUIREMENTS.
- IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR AND/OR REPLACE THE EXISTING STRUCTURE AS NECESSARY.
- IF IMPORTED FILL MATERIAL IS REQUIRED, IT SHALL BE CERTIFIED IN WRITING BY A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER AS NON-CONTAMINATED, CLEAN FILL SUITABLE FOR THE INTENDED USE. THE MATERIAL SHALL BE TESTED WITH THE ENVIRONMENTAL PARAMETERS AS OUTLINED IN THE PROJECT SPECIFICATIONS.
- THIS PROJECT INCLUDES THE COMPLETION OF DEEP EXCAVATIONS TO COMPLETE THE WORK. THE CONTRACTOR IS RESPONSIBLE FOR COMPLETING ALL EXCAVATION WORK IN ACCORDANCE WITH ALL LOCAL, STATE AND FEDERAL REQUIREMENTS TO ENSURE WORKER AND PUBLIC SAFETY. SPECIAL NOTICE SHOULD BE TAKEN THAT THE CONTRACTOR IS RESPONSIBLE FOR THE COSTS OF DESIGN AND CONSTRUCTION OF ALL SYSTEMS REQUIRED TO COMPLETE EXCAVATIONS SUCH AS SHEETING AND SHORING SYSTEMS. THESE COSTS MUST BE INCLUDED IN THE BID.
- CONTRACTOR SHALL PROVIDE ALL MAINTENANCE AND PROTECTION OF TRAFFIC LABOR AND EQUIPMENT AS REQUIRED AND SHALL OBTAIN ALL NECESSARY PERMITS AND APPROVALS IN ORDER TO COMPLETE THE WORK IN A SATISFACTORY AND SAFE MANNER.
- WHenever an item of material, equipment, construction methods or procedures is specified or described in the contract/bidding documents by using the name of a proprietary item, make or catalogue number, or the name of a particular supplier, the specification or description is intended to establish a standard of the required type, function, appearance, and quality required to be met by any proposed substitute or or-equal item. Other items of material or equipment of other suppliers may be submitted to and considered by the engineer for review as an equal or substitute.

MATERIALS NOTES:

- ALL FULL DEPTH PAVEMENT REPLACEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE NEW YORK STATE DEPARTMENT OF TRANSPORTATION SPECIFICATIONS, LATEST REVISION.
- ALL EXISTING PAVEMENT DEPICTED TO BE SAWCUT, SHALL BE MILLED 2 FEET FROM THE SAWCUT EDGE. SAWCUTS IN EXISTING PAVEMENT SHALL BE SMOOTH AND STRAIGHT.
- EXISTING PAVEMENT TO BE REMOVED AROUND STRUCTURES SHALL BE SAWCUT 3 FEET FROM THE OUTSIDE WALL OF STRUCTURE AND RESURFACED 2 FEET BEYOND THE EDGE OF THE SAWCUT.
- TOPSOIL FROM THE EXISTING SITE SHALL BE REUSED ON-SITE IN ALL AREAS REQUIRING TOPSOIL. THE CONTRACTOR SHALL NOT REMOVE ANY TOPSOIL FROM THE SITE PRIOR TO COMPLETION OF WORK, UNLESS PRIOR APPROVAL FROM THE OWNER/ENGINEER IS OBTAINED.
- ALL FILL GENERATED FROM REQUIRED EXCAVATION ACTIVITY SHALL BE REUSED ON-SITE WHERE IT MEETS THE SPECIFICATIONS, TO THE MAXIMUM EXTENT POSSIBLE. REMOVE AND DISPOSE OF ALL EXCESS FILL MATERIAL GENERATED THAT IS NOT REQUIRED AND/OR SUITABLE TO COMPLETE THE WORK.

GRADING AND DRAINAGE NOTES:

- CASTINGS FOR EXISTING STRUCTURES TO REMAIN AND PROPOSED STRUCTURES SHALL BE SET TO THE PROPOSED ELEVATIONS SHOWN ON THE DRAWINGS.
- THE FILLING OF SOIL OVER THE ROOTS OF TREES TO BE PRESERVED IS PROHIBITED.
- SUITABLE SOIL MATERIALS ARE DEFINED AS THOSE COMPLYING WITH ASTM D2487 SOIL CLASSIFICATION GROUPS GW, SM, SW AND SP AS STATED IN THE PROJECT SPECIFICATIONS.
- UNSUITABLE FILL AND BACKFILL MATERIALS ARE MATERIALS CONTAINING EXCESSIVE AMOUNT OF WATER, PLASTIC CLAY, VEGETATION, ORGANIC MATTER, DEBRIS, PAVEMENT, STONES OR BOULDERS OVER 3 INCHES IN GREATEST DIMENSION, FROZEN MATERIAL, AND MATERIAL WHICH, IN THE OPINION OF THE ENGINEER, WILL NOT PROVIDE A SUITABLE FOUNDATION OR SUBGRADE.
- SUITABLE SOIL SHALL BE PLACED IN LAYERS TO THE REQUIRED ELEVATIONS AS SHOWN ON THE DRAWINGS. FILL, BACKFILL AND COMPACT TO PRODUCE MINIMUM SUBSEQUENT SETTLEMENT OF THE MATERIAL AND PROVIDE ADEQUATE SUPPORT FOR THE SURFACE TREATMENT OR STRUCTURE TO BE PLACED ON THE MATERIAL. PLACE MATERIAL IN APPROXIMATELY HORIZONTAL LAYERS OF BEGINNING AT LOWEST AREA TO BE FILLED. DO NOT IMPAIR DRAINAGE. DO NOT USE ON-SITE TOPSOIL AS FILL MATERIAL.
- CONTRACTOR SHALL PLACE BACKFILL AND FILL MATERIALS IN LAYERS NOT MORE THAN (6) INCHES IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HEAVY COMPACTION EQUIPMENT, AND NOT MORE THAN (4) INCHES IN LOOSE DEPTH FOR MATERIAL COMPACTED BY HAND-OPERATED TAMPERS. DO NOT PLACE BACKFILL OR FILL MATERIAL ON SURFACES THAT ARE MUDDY, FROZEN, OR CONTAIN FROST OR ICE.
- FOR GROUND SURFACE PREPARATION, CONTRACTOR SHALL REMOVE VEGETATION, DEBRIS, UNSUITABLE SOIL MATERIALS, OBSTRUCTIONS, AND DELETERIOUS MATERIALS FROM GROUND SURFACE PRIOR TO PLACEMENT OF FILL. REMOVE MATERIAL TO THE FULL EXTENT OF ROOT PENETRATION. PROOF-ROLL EXISTING GROUND SURFACE PRIOR TO PLACEMENT OF FILL TO PROVIDE A DENSE, STABLE BASE FOR THE FIRST LIFTS OF THE STRUCTURAL FILL.
- PRIOR TO CONSTRUCTION, COMPLETE CAMERA INSPECTION OF EXISTING STORM DRAINAGE PIPES ADJACENT TO THE PROJECT AS SHOWN ON THE DRAWINGS.
- PRIOR TO THE START OF CONSTRUCTION, CONTRACTOR SHALL HAND DIG TEST PITS TO VERIFY THE LOCATION OF ALL EXISTING UNDERGROUND UTILITIES. CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF ALL EXISTING UTILITIES AT PROPOSED UTILITY CROSSINGS, AND ADVISE THE ENGINEER IN WRITING OF ANY DISCREPANCIES.
- ALL PROPOSED UTILITIES AND THEIR CONNECTIONS, DISCONNECTION AND RELOCATION OF EXISTING UTILITIES SHALL BE COMPLETED IN ACCORDANCE WITH THE REQUIREMENTS OF THE UTILITY COMPANY HAVING JURISDICTION. ANY COORDINATION WITH THE MUNICIPALITY AND/OR UTILITY COMPANY, PERMITS OR APPROVALS REQUIRED SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- AT NO TIME SHALL THE CONTRACTOR DISTURB MORE THAN 5 ACRES AT ANY ONE TIME WITHOUT WRITTEN APPROVAL FROM AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL SEQUENCE THE WORK ACCORDINGLY.

SCHEDULE OF TEMPORARY EROSION CONTROL MEASURES:

MEASURE	DATES FOR USE	TIMING, ACTIVITY, AND LOCATION
SILT FENCE/ CONSTRUCTION FENCE	ALL	CONTRACTOR TO INSTALL CONSTRUCTION AND SILT FENCE PRIOR TO THE START OF ANY EXCAVATION AND/OR FILLING ON THE SITE. SILT FENCE SHALL BE INSTALLED AS SHOWN ON THE EROSION AND SEDIMENT CONTROL PLAN AND DETAIL SHEETS, AND SHALL BE MAINTAINED IN EFFECTIVE OPERATING CONDITION THROUGHOUT THE CONSTRUCTION PROCESS. ALL STOCKPILES OF DIRT SHALL BE RIMMED WITH SILT FENCE IN ADDITION TO BEING TEMPORARILY SEEDED. DAMAGED SECTIONS OF SILT FENCE SHALL BE REPLACED IMMEDIATELY. FENCING SHALL BE REMOVED ONLY AFTER ALL CONSTRUCTION HAS BEEN COMPLETED AND ALL AREAS HAVE BEEN PERMANENTLY STABILIZED.
INLET PROTECTION	ALL	SHORT TERM PROTECTION OF CATCH BASIN INLETS. ALL EXISTING AND PROPOSED CATCH BASINS SHALL BE PROTECTED WITH INLET PROTECTION INSTALLED BY THE CONTRACTOR AS DETAILED, AND SHALL BE REPLACED AS NECESSARY. INLET PROTECTION SHALL REMAIN IN ALL STRUCTURES UNTIL THE SITE HAS BEEN PERMANENTLY STABILIZED.
DUST CONTROL	ALL	DURING DRY WEATHER, FOR AREAS OF EXPOSED SOIL WHERE IT IS NOT FEASIBLE TO ESTABLISH TEMPORARY GROUND COVER DUE TO CONSTRUCTION OPERATIONS, THE CONTRACTOR SHALL WET AREAS WITH WATER AT LEAST TWICE A DAY IN ORDER TO CONTROL DUST. THE MOISTENING OF SUCH AREAS MAY BE INCREASED TO FOUR TIMES A DAY DURING PERIODS OF LITTLE RAIN AS DETERMINED BY THE ENGINEER AND/OR THE CONTRACTOR.
TEMPORARY SEEDING	ALL	CONTRACTOR SHALL TEMPORARILY SEED ALL EXPOSED AREAS OF SOIL THAT WILL NOT RECEIVE PERMANENT SURFACE TREATMENT IMMEDIATELY (WITHIN SEVEN DAYS) AND ALL PILES OF DIRT AND SOIL STOCKPILES. GRASS SEED MIX FOR EROSION AND SEDIMENT CONTROL MAY BE APPLIED BY EITHER MECHANICAL OR HYDROSEEDING METHODS. HYDROSEEDING SHALL BE PERFORMED IN ACCORDANCE WITH THE AMERICAN ASSOCIATION OF NURSERYMAN, AMERICAN STANDARD FOR NURSERY STOCK, LATEST EDITION. SEEDING RATES AND DATES OF APPLICATION SHALL BE DETERMINED AS FOLLOWS. SEED MIXTURE: OATS APPLIED RATE: 1.8 LBS/1,000 S.F. SEED MIXTURE: ANNUAL RYEGRASS APPLIED RATE: 0.9 LBS/1,000 S.F. SEED MIXTURE: SUDANGRASS APPLIED RATE: 0.9 LBS/1,000 S.F. SEED MIXTURE: WINTER RYE APPLIED RATE: 2.6 LBS/1,000 S.F.
MULCH	APRIL 1 – NOVEMBER 30	ON ALL AREAS OF EXPOSED SOIL WHICH WILL NOT BE DISTURBED AGAIN WITHIN 7 DAYS, APPLY AT A RATE OF 1.5 TO 2.0 TONS PER ACRE.
WINTER MULCH	DECEMBER 1 – MARCH 31	ON ALL AREAS OF EXPOSED SOIL WHICH WILL NOT BE DISTURBED AGAIN WITHIN 7 DAYS, APPLY AT A RATE OF 3.0 TO 4.0 TONS PER ACRE. EROSION CONTROL BLANKET MAY BE USED AS A SUBSTITUTE FOR WINTER MULCH.
EROSION CONTROL BLANKET	ALL	INSTALL IMMEDIATELY FOLLOWING SEEDING, WITHIN DRAINAGE CHANNELS AND ON ALL EXPOSED SOIL SLOPES WHICH ARE 25% OR STEEPER GRADE, AND LOCATIONS SHOWN ON PLAN. ECB MAY ALSO BE SUBSTITUTED FOR WINTER MULCH.
INSPECTIONS	UNTIL SITE IS PERMANENTLY STABILIZED	ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSPECTED BY THE CONTRACTOR IMMEDIATELY AFTER EACH RAINFALL EVENT AND AT LEAST DAILY DURING PROLONGED RAINFALL. ANY REQUIRED REPAIRS SHALL BE MADE BY THE CONTRACTOR. SEDIMENT DEPOSITS SHALL BE REMOVED BY THE CONTRACTOR WHEN THEY REACH APPROXIMATELY ONE-THIRD THE HEIGHT OF THE SILT FENCE. SEDIMENTS SHALL BE DISPOSED OF IN A MANNER THAT DOES NOT RESULT IN ADDITIONAL EROSION OR POLLUTION.

SCHEDULE OF PERMANENT EROSION CONTROL MEASURES:

MEASURE	DATES FOR USE	TIMING, ACTIVITY, AND LOCATION
PAVEMENT – BASE COURSE/ FINAL COURSE	WHEN OUTSIDE AMBIENT TEMP. IS ABOVE 40°F	INSTALL ONLY IN AREAS SHOWN ON THE PLAN, SHORTLY AFTER PAVEMENT BASE IS BROUGHT TO FINAL GRADE. INSTALL NEAR COMPLETION OF PROJECT.
PERMANENT SEEDING	APRIL 15 TO SEPT. 15	ON FINAL GRADE AREAS, WITHIN 10 DAYS OF FINAL GRADE PREPARATION. PREPARE TOPSOIL, FOLLOWED WITH SEEDING AND MULCH APPLICATION. PERMANENT VEGETATION MUST BE SEEDED OR SODDED ON ALL EXPOSED AREAS. MULCH MUST BE USED AS NECESSARY FOR PROTECTION, UNTIL SEEDING IS ESTABLISHED.
DORMANT SEEDING	SEPT. 16 TO APRIL 15	ON FINAL GRADE AREAS, WITH PREPARED TOPSOIL. APPLY SEED AT DOUBLE THE SPECIFIED RATE, ON BARE SOIL, AND FOLLOW WITH AN APPLICATION OF WINTER MULCH.
GROUND COVER, TREES, SHRUBS	APRIL 15 TO NOV. 1	INSTALL WITH FINAL LANDSCAPING.
PERMANENT MULCH	ALL	INSTALL WITH FINAL LANDSCAPING.

NOTES:

- ALL PROPOSED EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSTALLED AS INDICATED ON THE PLANS, AND IN ACCORDANCE WITH THE NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION (NYSDEC) "NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL," LATEST REVISION.
- ADDITIONAL SOIL STABILIZATION WILL BE REQUIRED IF THERE IS THE POSSIBILITY OF WIND EROSION.
- SEDIMENT AND DEBRIS DROPPED, WASHED, SPILLED OR TRACKED ONTO PUBLIC OR PRIVATE ROADWAYS OR OTHER HARDSCAPE SURFACES SHALL IMMEDIATELY BE REMOVED.

LEGEND		
---	PROPERTY LINE	PROPOSED BUILDING LINE
---	EXISTING CURB	PROPOSED CONTOUR
---	EXISTING EDGE OF PAVEMENT	PROPOSED SPOT GRADE
---	EXISTING CHAIN LINK FENCE	PROPOSED SLOPE AND DIRECTION OF FLOW
---	EXISTING RETAINING WALL	PROPOSED DRAIN INLET
---	EXISTING SIGN	PROPOSED MANHOLE
---	EXISTING UTILITY POLE	PROPOSED BYPASS MANHOLE
---	EXISTING FIRE HYDRANT	PROPOSED HYDRODYNAMIC SEPARATOR (OPEN GRATE)
---	EXISTING MANHOLE	PROPOSED WATER QUALITY FILTER STRUCTURE
---	EXISTING CATCH BASIN (TYP.)	PROPOSED CLEANOUT
---	EXISTING LIGHT POLE	PROPOSED HYDRANT
---	EXISTING STORM LINE AND SIZE	PROPOSED SOLID WALL STORM PIPE
---	EXISTING SANITARY SEWER	PROPOSED PERFORATED STORM DRAIN
---	EXISTING CONTOUR	PROPOSED SANITARY SEWER PIPE
---	EXISTING SPOT GRADE	PROPOSED GAS LINE
---	EXISTING FEATURE TO BE REMOVED	PROPOSED WATER LINE
---		PROPOSED UNDERGROUND ELECTRIC LINE
---		PROPOSED CONSTRUCTION ENTRANCE
---		PROPOSED SOIL STOCKPILE AREA
---		PROPOSED TRENCH DRAIN
---		PROPOSED CONSTRUCTION FENCE WITH PRIVACY SCREENING
---		PROPOSED CONSTRUCTION FENCE
---		PROPOSED SILT FENCE
---		PROPOSED INLET PROTECTION
---		LIMIT OF DISTURBANCE/ CONTRACT LIMIT LINE

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CONTRACTOR CERTIFICATION STATEMENT:

New York State Department of Environmental Conservation (NYSDEC)
SPDES General Permit for
Stormwater Discharges from
CONSTRUCTION ACTIVITY
Permit No. GP-6-20-001

CONTRACTOR CERTIFICATION STATEMENT

Project Address: Harrison Recreation & Community Center
270 Harrison Avenue
Harrison, NY 10528

Date: _____

Certification Statement:

"I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the qualified inspector during a site inspection. I also understand that the owner or operator must comply with the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations."

Signature: _____
Name (Print): _____
Title: _____

Contracting Firm: _____
Address: _____
Telephone: _____

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3	06/09/2023	ISSUE FOR PERMIT - PHASE 1
2	06/01/2020	DESIGN DEVELOPMENT
1	03/31/2020	50% DESIGN DEVELOPMENT

No. _____ Date _____ Issue _____

Sheet Title

CONSTRUCTION NOTES & LEGENDS

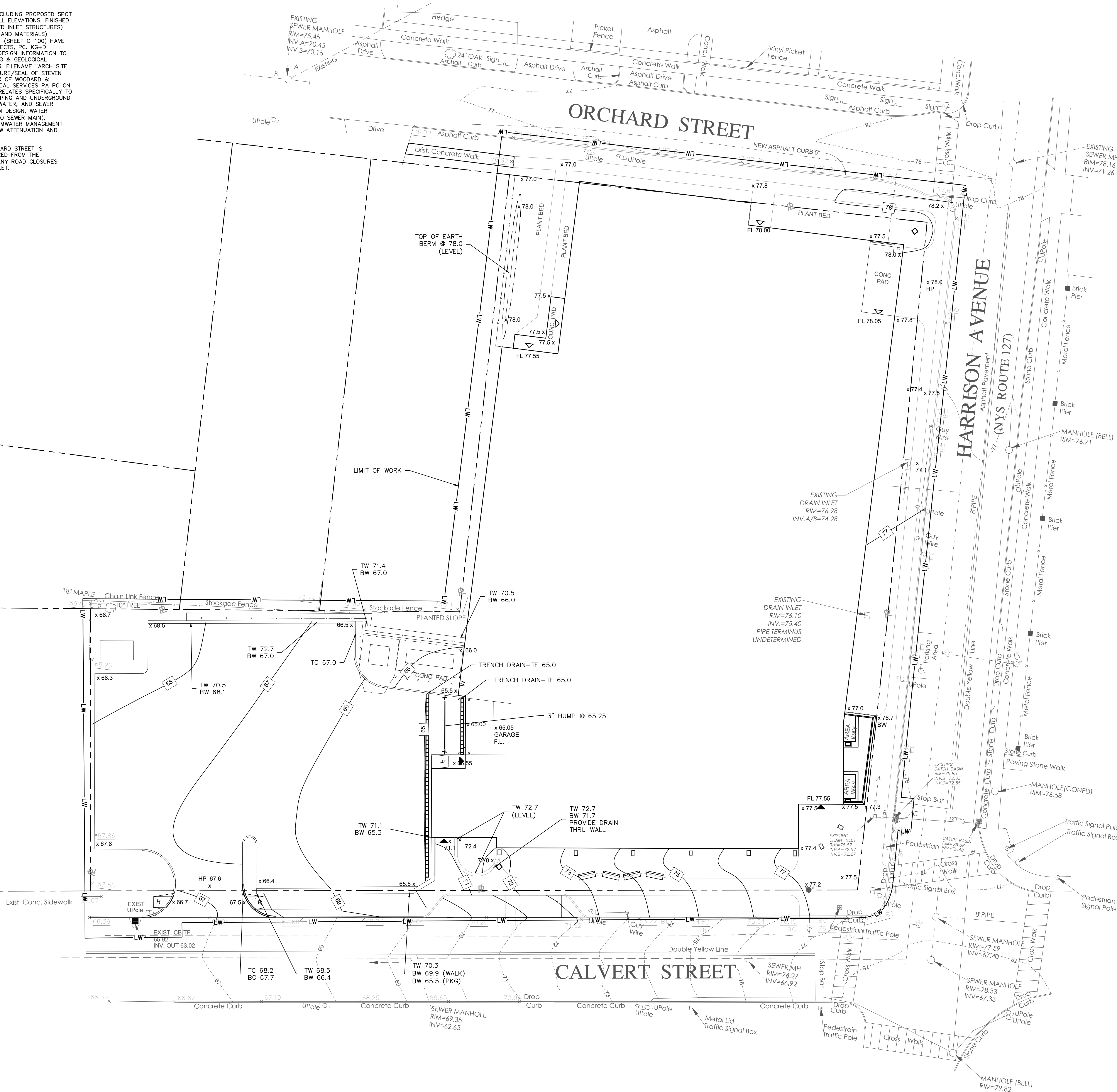
Job No.	2020-1005	Date	06/29/2023
Scale	AS NOTED	Drawn / Checked	CJJ/JLMT

Sheet Number

C-001

GENERAL NOTES:

1. THE PROPOSED SITE GRADING (INCLUDING PROPOSED SPOT ELEVATIONS, CONTOURS, AND WALL ELEVATIONS; FINISHED GRADE ELEVATIONS FOR PROPOSED INLET STRUCTURES) AND LAYOUT PLAN (ALL LAYOUT AND MATERIALS) DEPICTED ON THIS GRADING PLAN (SHEET C-100) HAVE BEEN DESIGNED BY KG+D ARCHITECTS, PC. KG+D ARCHITECTS, PC PROVIDED THIS DESIGN INFORMATION TO WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON 06/30/2023. FILENAME "ARCH SITE PLAN 2023.DWG". THE PE SIGNATURE/SEAL OF STEVEN LAURIA, SENIOR PROJECT MANGER OF WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON SHEETS C-100, C-200, C-300, RELATES SPECIFICALLY TO THE DESIGN OF UNDERGROUND PIPING AND UNDERGROUND STRUCTURES FOR STORMWATER, WATER, AND SEWER (INCLUDING STORM SEWER SYSTEM DESIGN, WATER SERVICE LINE AND CONNECTION TO SEWER MAIN), SURFACE AND SUBSURFACE STORMWATER MANAGEMENT FEATURES FOR STORMWATER FLOW ATTENUATION AND WATER QUALITY TREATMENT.
2. CONSTRUCTION TRAFFIC ON ORCHARD STREET IS PROHIBITED. APPROVAL IS REQUIRED FROM THE TOWN/VILLAGE OF HARRISON IF ANY ROAD CLOSURES REQUIRES USE OF ORCHARD STREET.



KEY TO SPOT GRADES:

- x 67.55 EXISTING ELEVATION TO REMAIN
- x 67.55 PROPOSED SPOT GRADE
- 67 PROPOSED CONTOUR
- HP HIGH POINT
- TW/BW TOP/BOTTOM OF WALL ELEV.
- TC/BC TOP/BOTTOM OF CURB

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3	06/09/2023	ISSUE FOR PERMIT - PHASE 1
2	06/01/2020	DESIGN DEVELOPMENT
1	03/31/2020	50% DESIGN DEVELOPMENT

Sheet Title

GRADING PLAN

Job No. 2020-1005 Date 07/07/2023

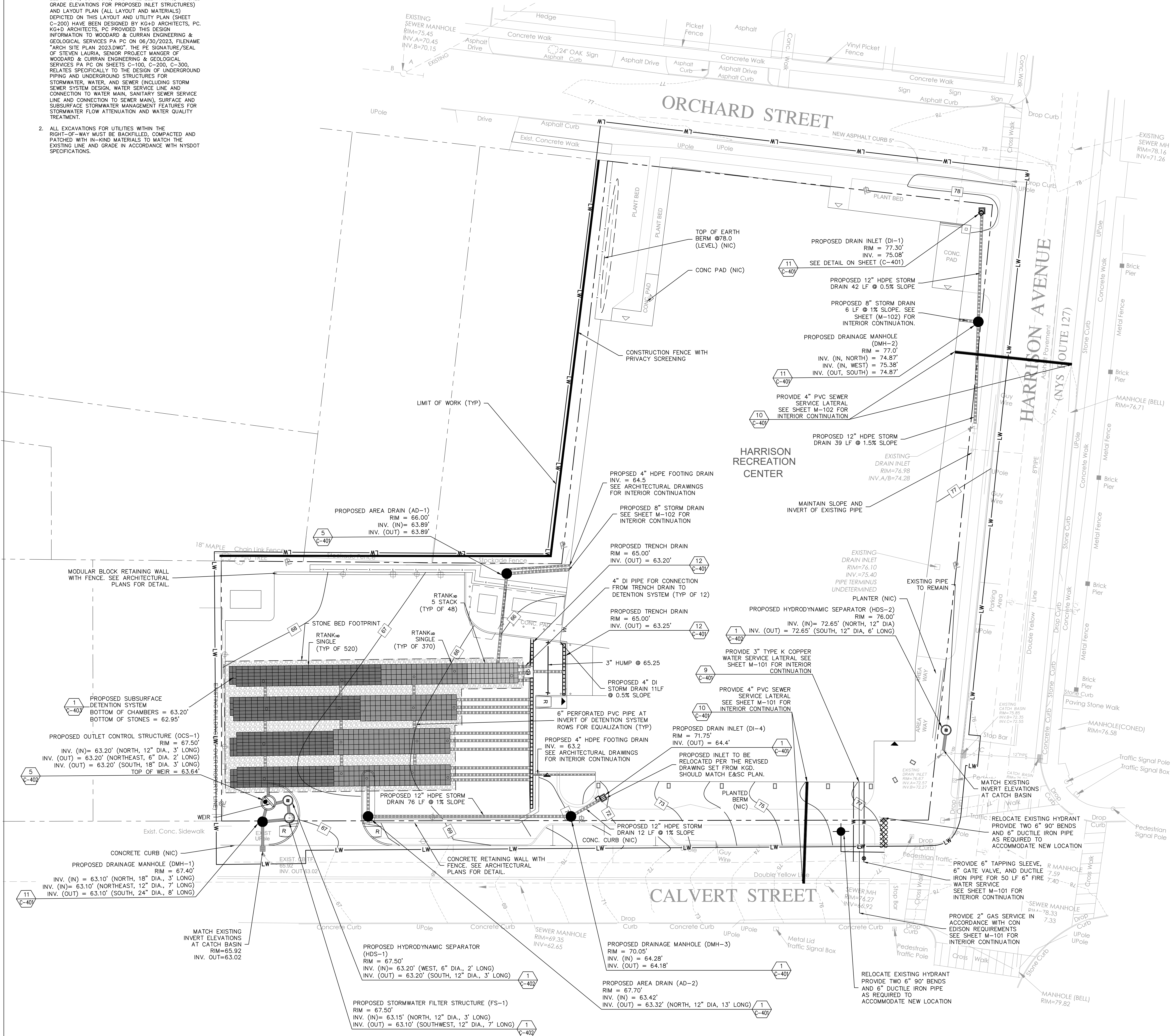
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Sheet Number

C-100

GENERAL NOTES:

1. THE PROPOSED SITE GRADING (INCLUDING PROPOSED SPOT ELEVATIONS, CONTOURS, AND WALL ELEVATIONS, FINISHED GRADE ELEVATIONS FOR PROPOSED INLET STRUCTURES) AND LAYOUT PLAN (ALL LAYOUT AND MATERIALS) DEPICTED ON THIS LAYOUT AND UTILITY PLAN (SHEET C-200) HAVE BEEN DESIGNED BY KG+D ARCHITECTS, PC. KG+D ARCHITECTS, PC PROVIDED THIS DESIGN INFORMATION TO WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON 06/30/2023, FILENAME "ARCH SITE PLAN 2023.DWG". THE PE SIGNATURE/SEAL OF STEVEN LAURIA, SENIOR PROJECT MANAGER OF WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON SHEETS C-100, C-200, C-300, RELATES SPECIFICALLY TO THE DESIGN OF UNDERGROUND PIPING AND UNDERGROUND STRUCTURES FOR STORMWATER, WATER, AND SEWER (INCLUDING STORM SEWER SYSTEM DESIGN, WATER SERVICE, LINE AND CONNECTION TO WATER MAIN, SANITARY SEWER SERVICE LINE AND CONNECTION TO SEWER MAIN), SURFACE AND SUBSURFACE STORMWATER MANAGEMENT FEATURES FOR STORMWATER FLOW ATTENUATION AND WATER QUALITY TREATMENT.
2. ALL EXCAVATIONS FOR UTILITIES WITHIN THE RIGHT-OF-WAY MUST BE BACKFILLED, COMPACTED AND PATCHED WITH IN-KIND MATERIALS TO MATCH THE EXISTING LINE AND GRADE IN ACCORDANCE WITH NYS DOT SPECIFICATIONS.

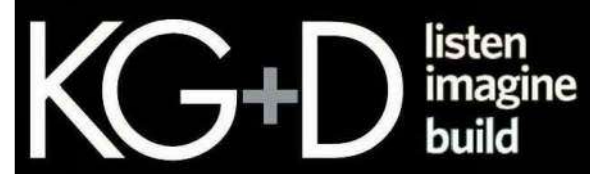


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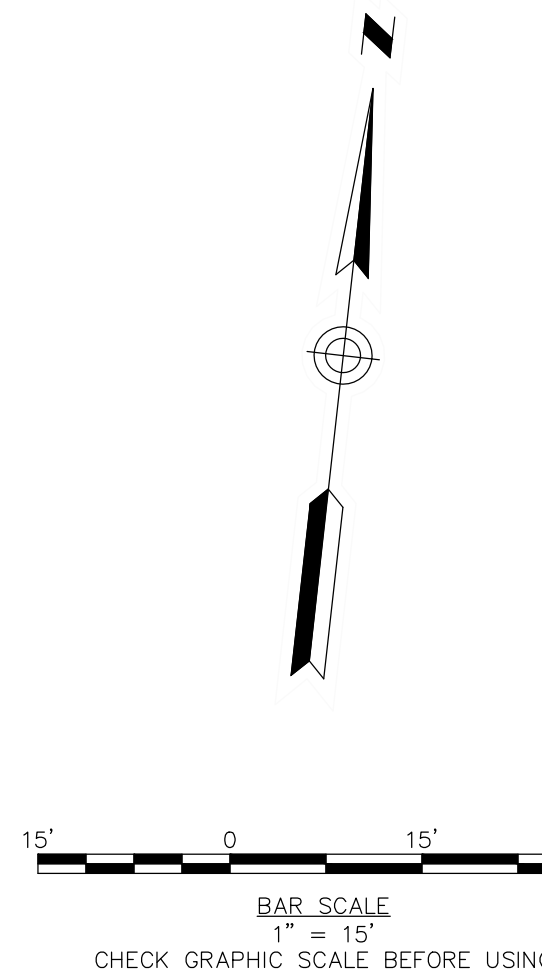
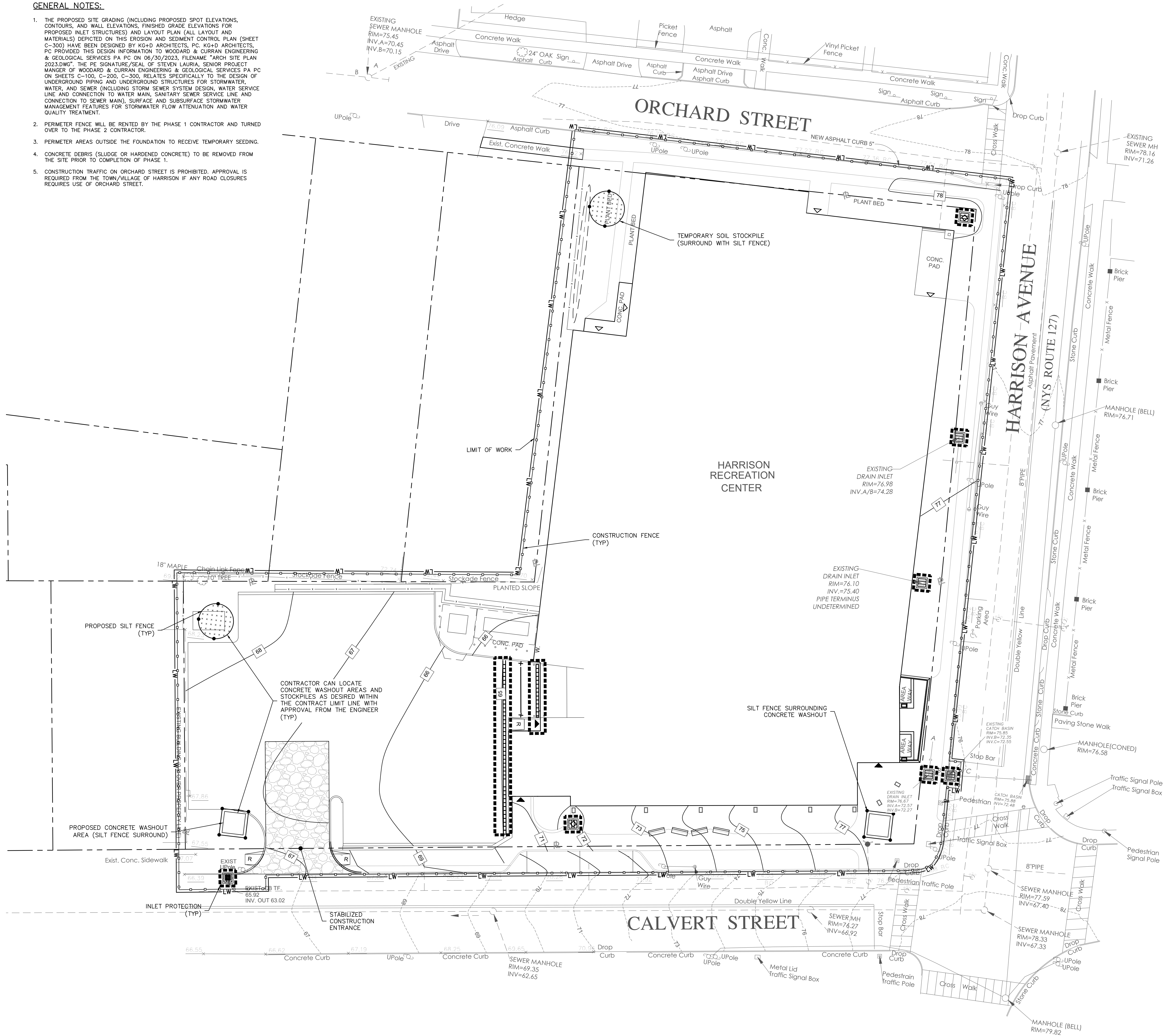
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UTILITIES
PLAN

Job No.	2020-1005	Date	07/07/2023
Scale	AS NOTED	Drawn / Checked	CJ/JJLMT
Sheet Number	C-200		

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1. THE PROPOSED SITE GRADING (INCLUDING PROPOSED SPOT ELEVATIONS, CONTOURS, AND WALL ELEVATIONS, FINISHED GRADE ELEVATIONS FOR PROPOSED INLET STRUCTURES) AND LAYOUT PLAN (ALL LAYOUT AND MATERIALS) DEPICTED ON THIS EROSION AND SEDIMENT CONTROL PLAN (SHEET C-300) HAVE BEEN DESIGNED BY KG+D ARCHITECTS, PC. KG+D ARCHITECTS, PC PROVIDED THIS DESIGN INFORMATION TO WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON 06/30/2023, FILENAME "ARCH SITE PLAN 2023.DWG". THE PE SIGNATURE/SEAL OF STEVEN LAURIA, SENIOR PROJECT MANAGER OF WOODARD & CURRAN ENGINEERING & GEOLOGICAL SERVICES PA PC ON SHEETS C-100, C-200, C-300, RELATES SPECIFICALLY TO THE DESIGN OF UNDERGROUND PIPING AND UNDERGROUND STRUCTURES FOR STORMWATER, WATER, AND SEWER (INCLUDING STORM SEWER SYSTEM DESIGN, WATER SERVICE LINE AND CONNECTION TO WATER MAIN, SANITARY SEWER SERVICE LINE AND CONNECTION TO SEWER MAIN), SURFACE AND SUBSURFACE STORMWATER MANAGEMENT FEATURES FOR STORMWATER FLOW ATTENUATION AND WATER QUALITY TREATMENT.
2. PERIMETER FENCE WILL BE RENTED BY THE PHASE 1 CONTRACTOR AND TURNED OVER TO THE PHASE 2 CONTRACTOR.
3. PERIMETER AREAS OUTSIDE THE FOUNDATION TO RECEIVE TEMPORARY SEEDING.
4. CONCRETE DEBRIS (SLUDGE OR HARDENED CONCRETE) TO BE REMOVED FROM THE SITE PRIOR TO COMPLETION OF PHASE 1.
5. CONSTRUCTION TRAFFIC ON ORCHARD STREET IS PROHIBITED. APPROVAL IS REQUIRED FROM THE TOWN/VILLAGE OF HARRISON IF ANY ROAD CLOSURES REQUIRES USE OF ORCHARD STREET.



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1	03/31/2020	50% DESIGN DEVELOPMENT

Sheet Title

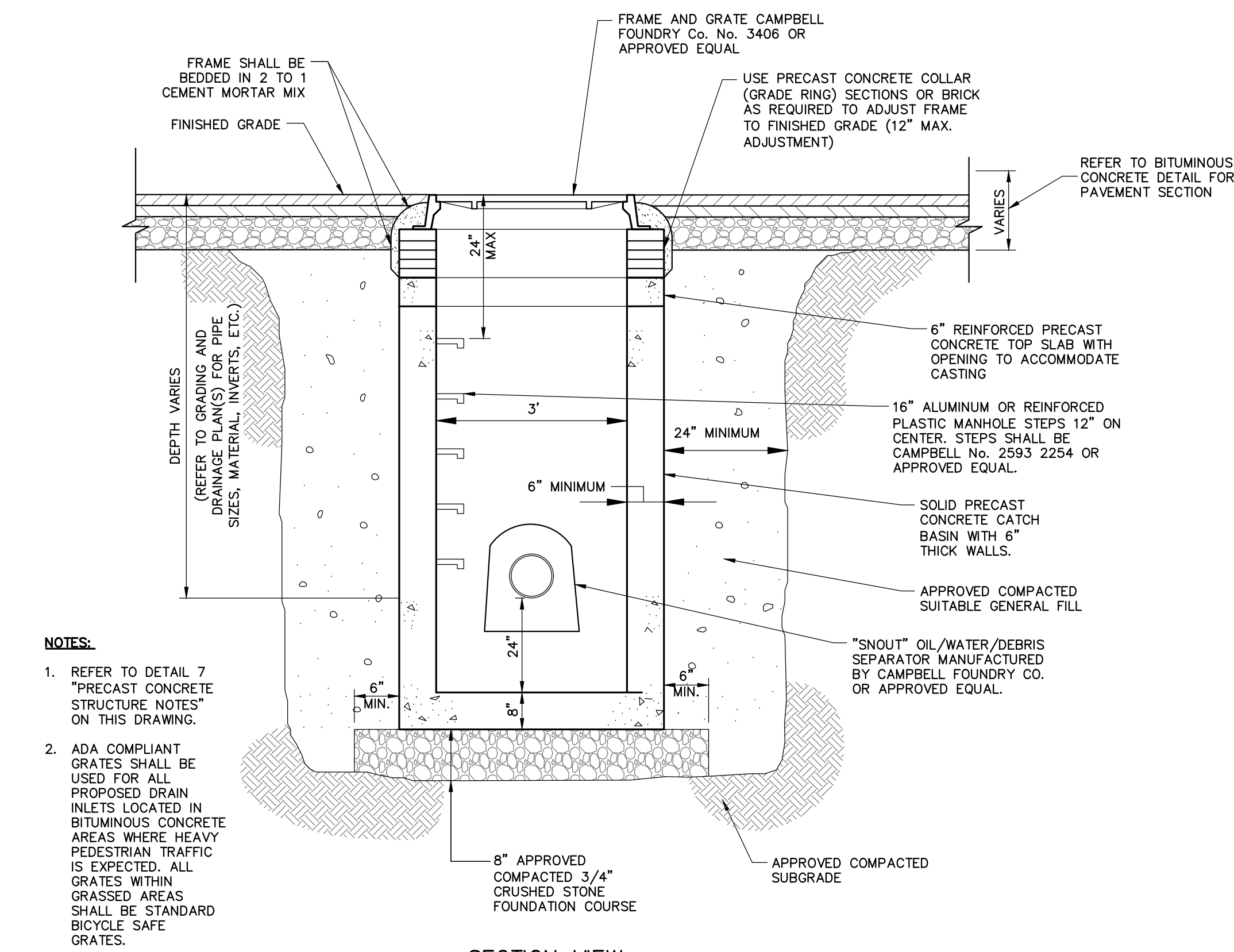
**EROSION &
SEDIMENT
CONTROL
PLAN**

Job No. 2020-1005 Date 07/07/2023

Scale AS NOTED Drawn / Checked MBA/JM

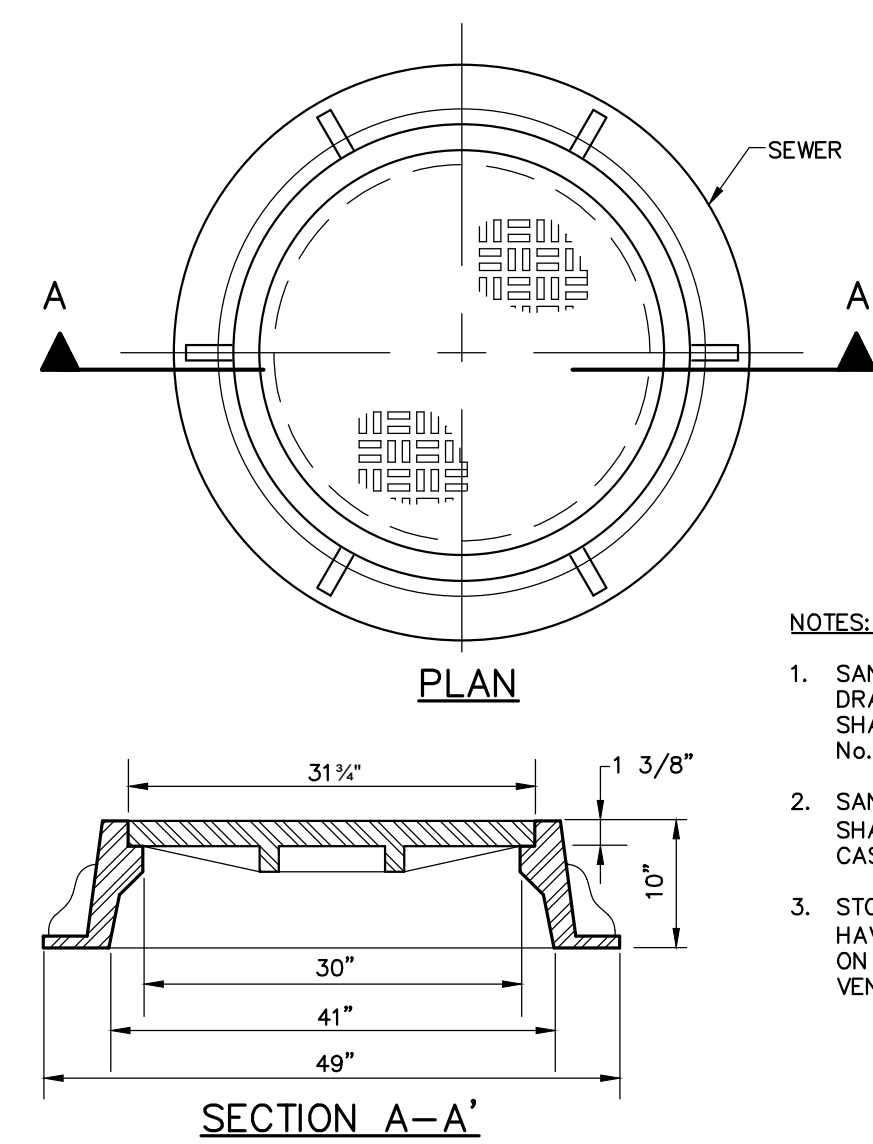
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C-300

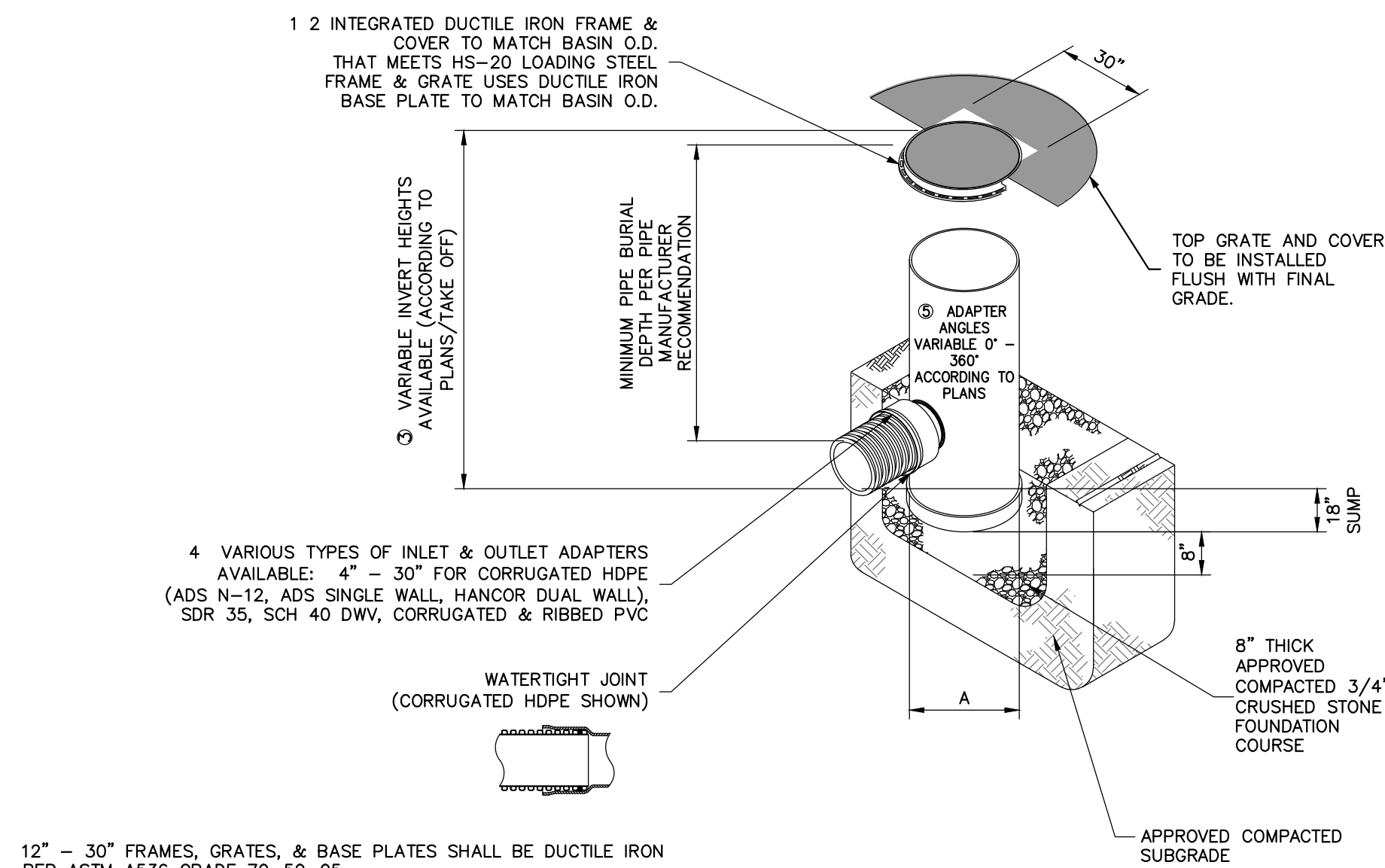


SECTION VIEW

1 DRAIN INLET (CATCH BASIN) C-401 NOT TO SCALE

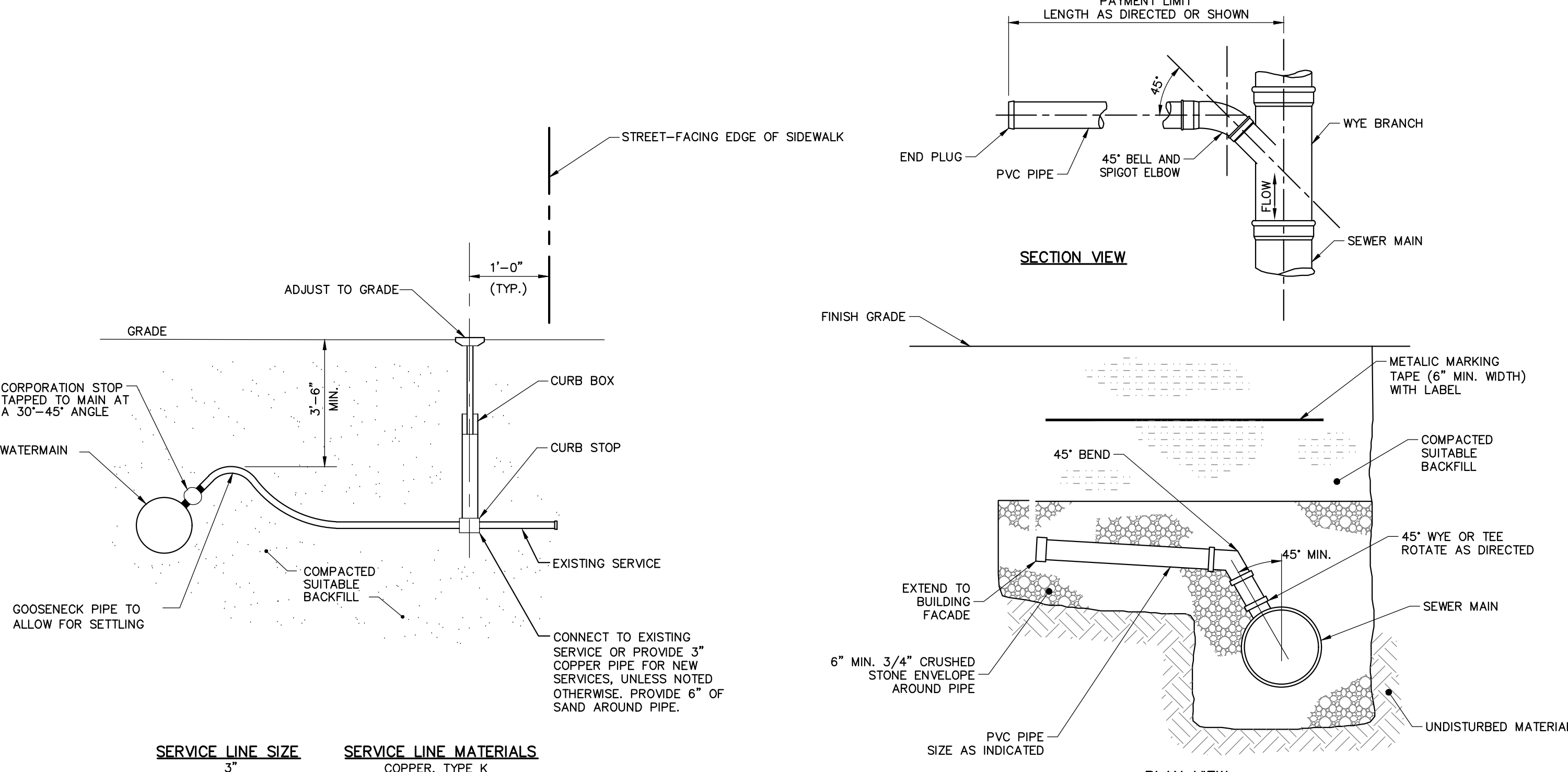


2 MANHOLE FRAME & COVER C-401 NOT TO SCALE



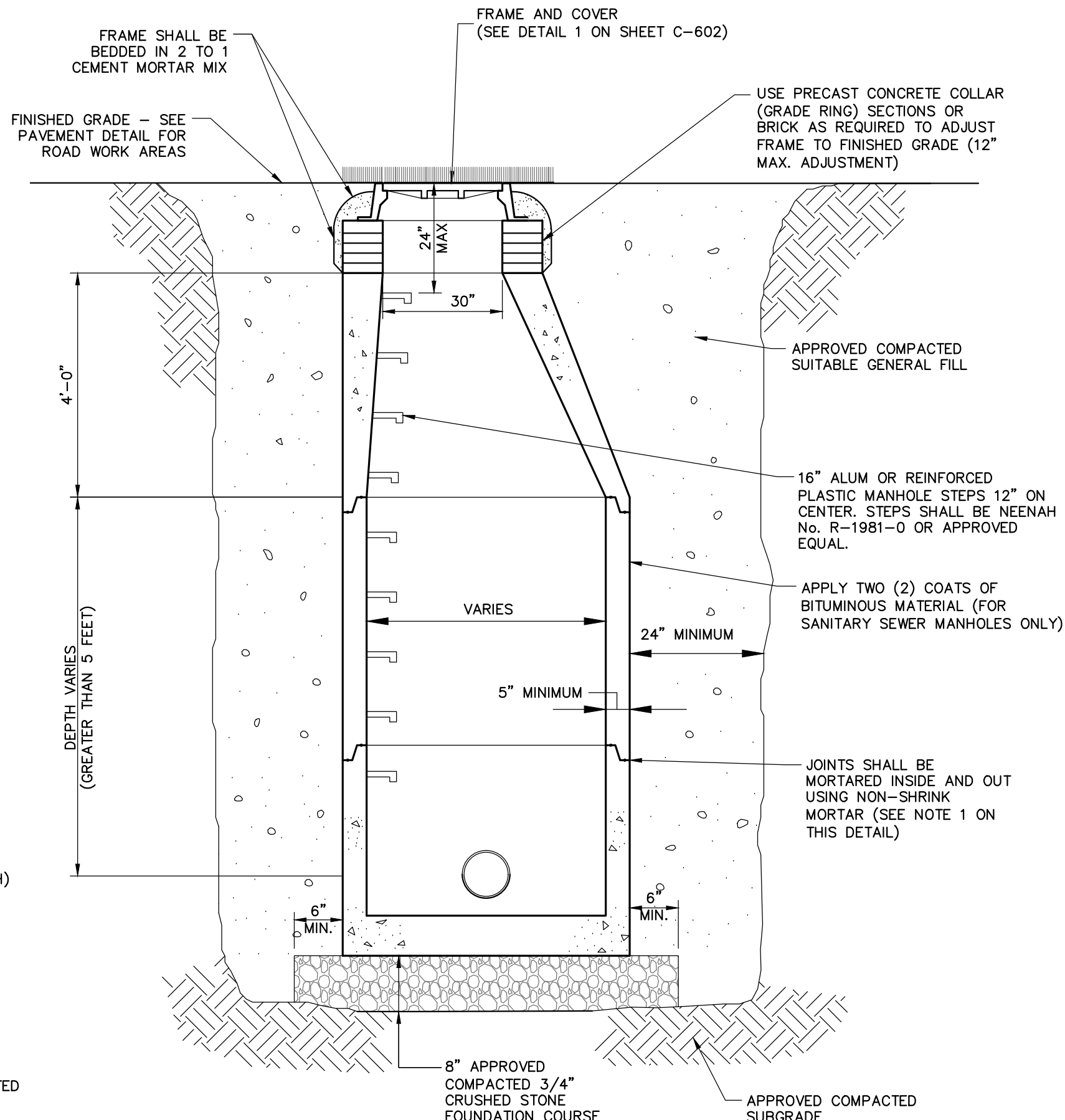
5 AREA DRAIN C-401 NOT TO SCALE

- 12" - 30" FRAMES, GRATES, & BASE PLATES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05.
- 12" - 30" FRAMES & GRATES SHALL BE STEEL PER ASTM A36.
- DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS.
- DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS & HANCOR DUAL WALL) & SDR 35 PVC.
- ADAPTERS CAN BE MOUNTED ON ANY ANGLE 0° TO 360° TO DETERMINE MINIMUM ANGLE BETWEEN ADAPTERS SEE DRAWING NO. 7001-110-012, 7001-110-013, & 7001-110-014.
- ALL OPTIONS SHALL MEET HS-20 LOAD RATING.



8 TYPICAL WATER SERVICE CONNECTION C-401 SCALE: N.T.S.

9 TYPICAL SEWER SERVICE CONNECTION C-401 SCALE: N.T.S.



10 DRAINAGE MANHOLE C-401 NOT TO SCALE

NOTES:

- IT IS NECESSARY TO ENSURE MINIMUM DIMENSIONS SHOWN ARE SUITABLE FOR EXISTING GROUND CONDITIONS. ENGINEERING ADVICE MAY BE REQUIRED.
- MINIMUM CONCRETE STRENGTH OF 4,000 PSI IS RECOMMENDED. CONCRETE SHOULD BE VIBRATED TO ELIMINATE AIR POCKETS.
- EXPANSION AND CONTRACTION CONTROL JOINTS AND REINFORCEMENT ARE RECOMMENDED TO PROTECT CHANNEL AND CONCRETE SURROUND. ENGINEERING ADVICE MAY BE REQUIRED.
- THE FINISHED LEVEL OF THE CONCRETE SURROUND MUST BE APPROX. 1/8" (3mm) ABOVE THE TOP OF THE CHANNEL EDGE.
- CONCRETE BASE THICKNESS SHOULD MATCH SLAB THICKNESS. ENGINEERING ADVICE MAY BE REQUIRED TO DETERMINE PROPER LOAD CLASS.
- REFER TO ACO'S LATEST INSTALLATION INSTRUCTIONS FOR FURTHER DETAILS.

SPECIFICATION CLAUSE

K300 KLASIKDRAIN 'DRAINLOK' LOAD CLASS C

GENERAL

THE SURFACE DRAINAGE SYSTEM SHALL BE POLYMER CONCRETE K300 CHANNEL SYSTEM WITH GALVANIZED STEEL EDGE RAILS AS MANUFACTURED BY ACO POLYMER PRODUCTS, INC.

MATERIALS

CHANNELS SHALL BE MANUFACTURED FROM POLYESTER RESIN POLYMER CONCRETE WITH AN INTEGRALLY CAST-IN GALVANIZED STEEL EDGE RAIL. MINIMUM PROPERTIES OF POLYMER CONCRETE WILL BE AS FOLLOWS:

COMPRESSIVE STRENGTH:	14,000 PSI
FLEXURAL STRENGTH:	4,000 PSI
TENSILE STRENGTH:	1,500 PSI
WATER ABSORPTION:	0.07%
FROST PROOF:	YES
DILUTE ACID AND ALKALI RESISTANT:	YES
B117 SALT SPRAY TEST COMPLIANT:	YES

THE SYSTEM SHALL BE 12" (300mm) NOMINAL INTERNAL WIDTH WITH A 14.2" (360mm) OVERALL WIDTH AND A BUILT-IN SLOPE OF 0.1%. CHANNEL INVERT SHALL HAVE DEVELOPED "V" SHAPE. ALL CHANNELS SHALL BE INTERLOCKING WITH A MALE/FEMALE JOINT.

THE COMPLETE DRAINAGE SYSTEM SHALL BE BY ACO POLYMER PRODUCTS, INC. ANY DEVIATION OR PARTIAL SYSTEM DESIGN AND/OR IMPROPER INSTALLATION WILL VOID ANY AND ALL WARRANTIES PROVIDED BY ACO POLYMER PRODUCTS, INC.

CHANNEL SHALL WITHSTAND LOADING TO PROPER LOAD CLASS AS OUTLINED BY EN 1433. GRATE TYPE SHALL BE APPROPRIATE TO MEET THE SYSTEM LOAD CLASS SPECIFIED AND INTENDED APPLICATION. GRATES SHALL BE SECURED USING 'DRAINLOK' BOLTLESS LOCKING SYSTEM CHANNEL AND GRATE SHALL BE CERTIFIED TO MEET THE SPECIFIED EN 1433 LOAD CLASS. THE SYSTEM SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS AND RECOMMENDATIONS.

K3-C-HAP

K300 - KLASIKDRAIN - LOAD CLASS: C

Haunched Asphalt Pavement

INSTALLATION DRAWING - ACO DRAIN

DATE: 08/18/16 Arizona Tel: 888-490-9552 e-mail: sales@acousa.com Ohio Tel: 800-543-4764 www.acousa.com South Carolina Tel: 800-543-4764

ACO Polymer Products, Inc.

825 W. Brookhurst St. Casa Grande, AZ 85122 Tel: 520-421-9899 Fax: 520-421-9899	9470 Pleasanton Dr. Menlo Park, CA 94026 Tel: 415-959-7200 Fax: 415-959-7235	4211 Pleasanton Rd. Port Hill, SC 29708 Tel: 404-839-7200 Fax: 404-839-7200
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11 TRENCH DRAIN C-401 SCALE: N.T.S.

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New York, New York 10523
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No.	Date	Issue
4	07/12/2023	ISSUE FOR BID - PHASE 1
3	06/09/2023	ISSUE FOR PERMIT - PHASE 1
2	06/01/2020	DESIGN DEVELOPMENT
1	03/31/2020	50% DESIGN DEVELOPMENT

Sheet Title

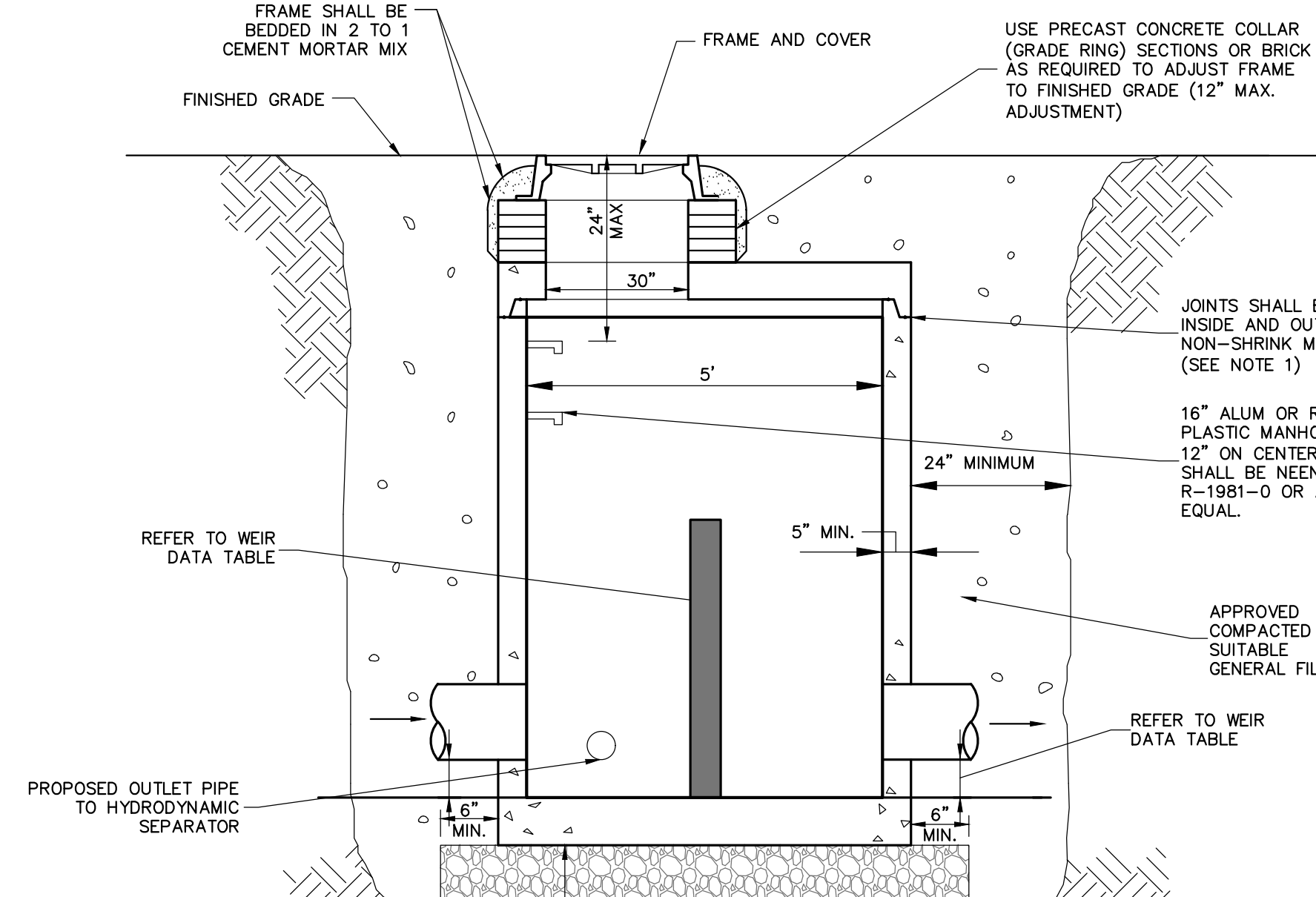
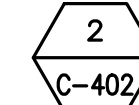
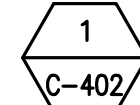
CONSTRUCTION DETAILS

Job No. 2020-1005 Date 06/29/2023

Scale AS NOTED Drawn / Checked C.J./J.M.T.

Sheet Number

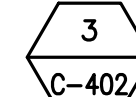
C-401



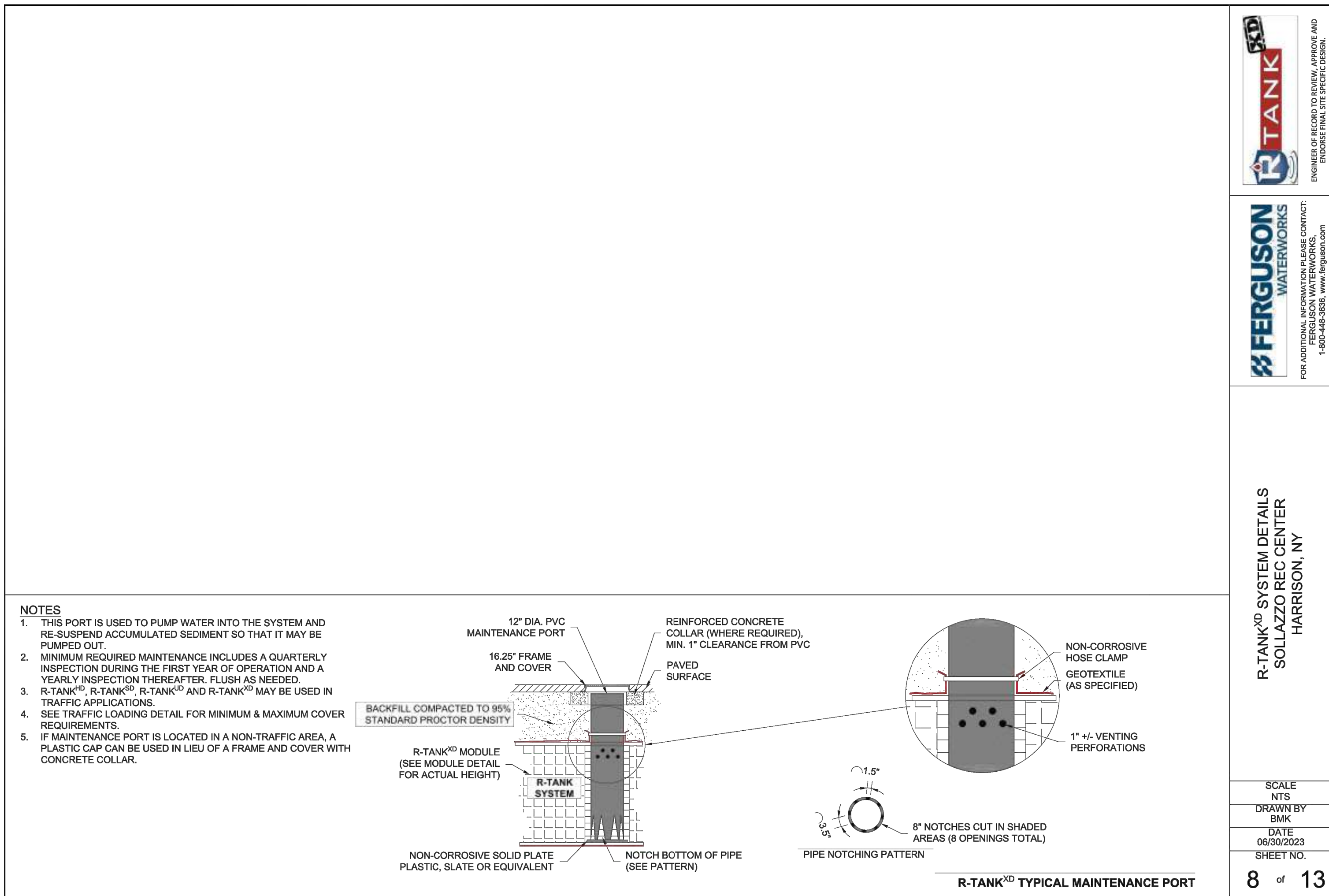
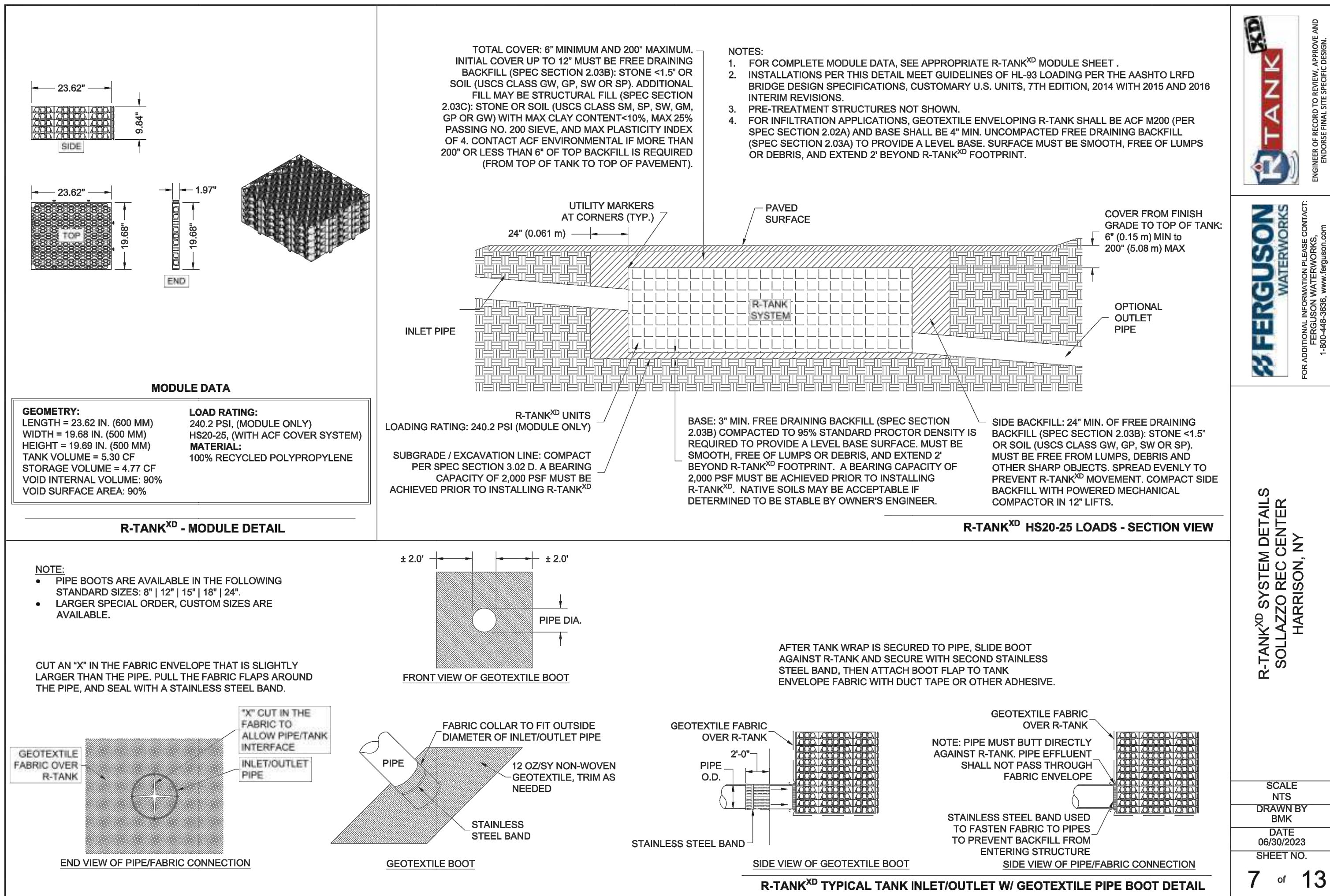
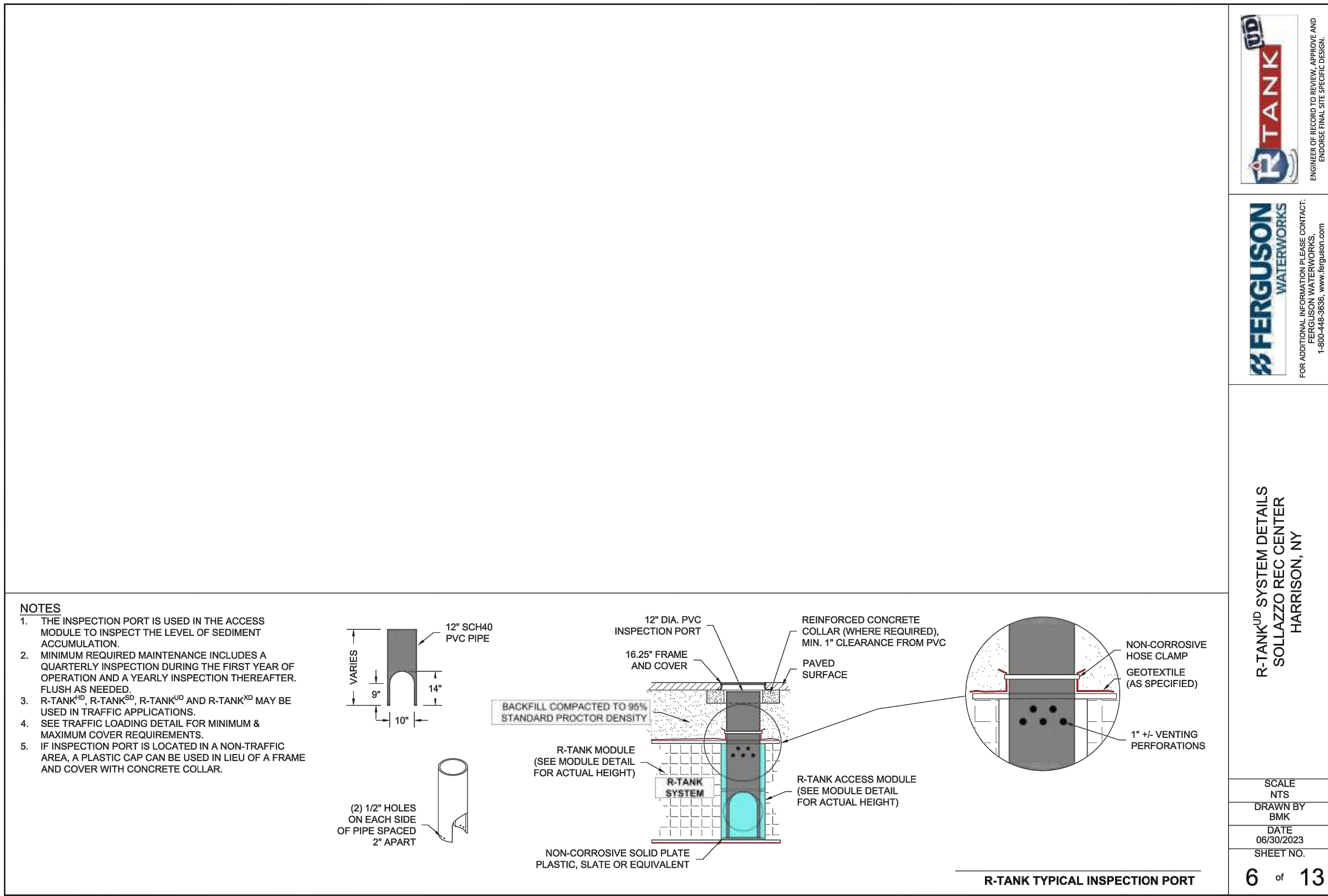
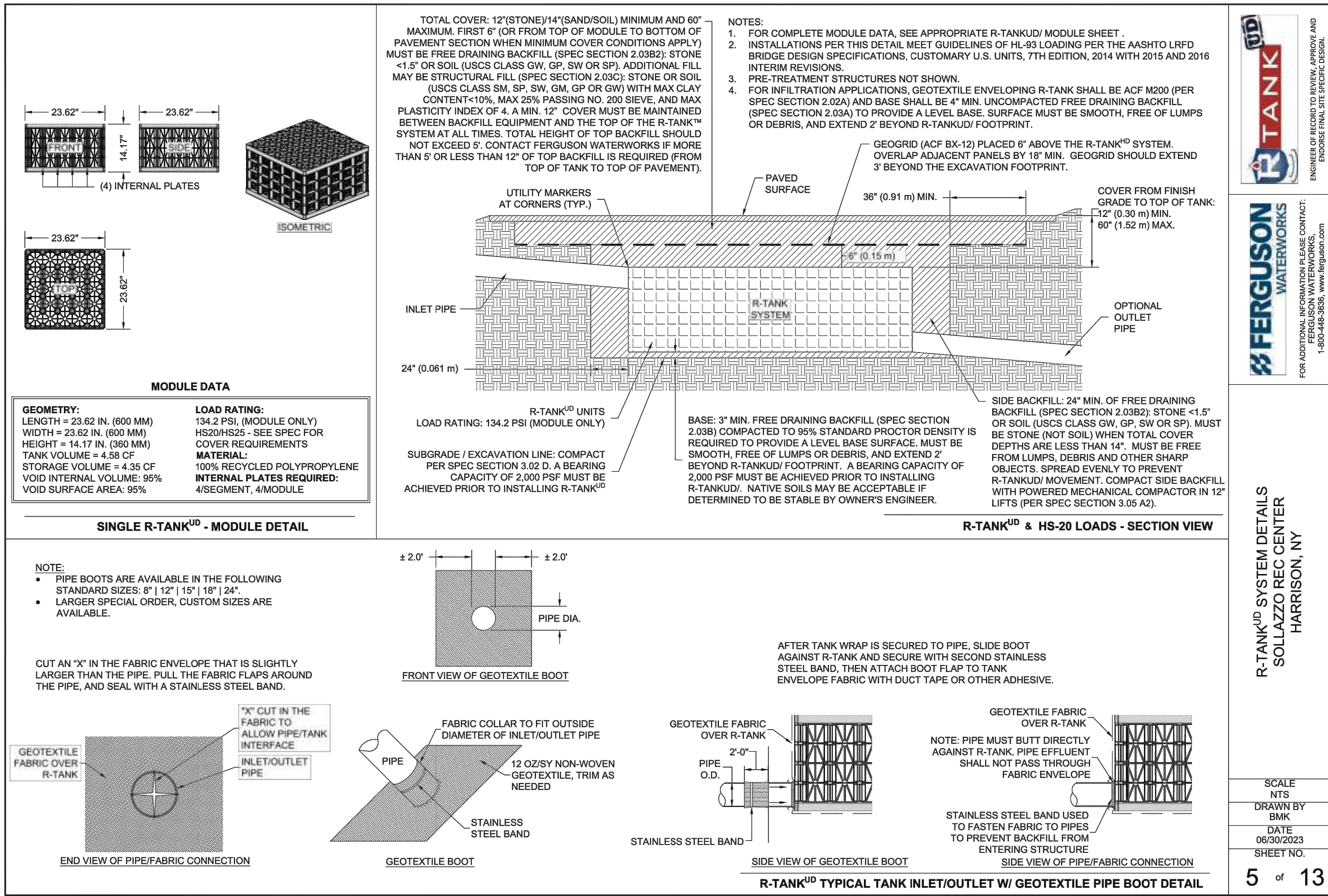
WEIR DATA TABLE	
STAGE	ELEVATION (FT)
	OCS-1
SUMP	62.70
WEIR	63.64

1. FOUNDATION COURSE FOR PRECAST CONCRETE STRUCTURES SHALL BE PLACED ON A COMPACTED LAYER OF APPROVED CRUSHED STONE HAVING A MINIMUM COMPACTED THICKNESS OF EIGHT (8) INCHES.
2. ALL PRECAST CONCRETE STRUCTURES SHALL BE DESIGNED TO ACCOMMODATE AN HS-20 DESIGN LOAD.
3. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FURNISH AND CONSTRUCT THE PROPER SIZE STRUCTURE INCLUDING THE NECESSARY OPENINGS TO ACCOMMODATE THE WORK AS SHOWN ON THE DRAWINGS OR AS ORDERED BY THE ENGINEER (AOBE), AT NO ADDITIONAL COST TO THE OWNER.
4. ALL REQUIRED PATCHING FOR DRAINAGE STRUCTURES SHALL BE COMPLETED WITH NON-SHRINKING CEMENT MORTAR GROUT WITH MINIMUM 1,000 PSI COMPRESSIVE STRENGTH IN (1) DAY AND 5,000 PSI COMPRESSIVE STRENGTH IN (28) DAYS AS TESTED PER ASTM C109 AND ASTM C-707. PATCHING PRODUCTS INCLUDE "DURAGROUT" BY L & M CONST. CHEMICAL CO., "TIE STAR GROUT" BY U.S. GROUT COMPANY OR APPROVED EQUAL.
5. REFER TO PROJECT SPECIFICATIONS FOR DRAINAGE AND SANITARY PIPE CONNECTIONS INTO PRECAST CONCRETE STRUCTURES.
6. PIPES SHALL BE CUT FLUSH WITH THE INSIDE WALL OF THE STRUCTURE.
7. MINIMUM COMPRESSIVE STRENGTH OF CLASS "A" CONCRETE (4,000 PSI).
8. PRECAST CONCRETE STRUCTURE LIFT HOLES SHALL BE PLUGGED AND MORTARED.
9. STRUCTURES SHALL CONFORM TO CURRENT ASTM AND OSHA REQUIREMENTS.
10. PRECAST CONCRETE STRUCTURE JOINTS SHALL CONFORM TO THE CURRENT ASTM DESIGNATION C-443.
11. PRECAST CONCRETE STRUCTURES SHALL COMPLY WITH ASTM STANDARD C-478. MANHOLE JOINTS SHALL COMPLY WITH ASTM STANDARD C-443.

1. FOR PRECAST CONCRETE MANHOLES FIVE (5) FEET OR LESS IN HEIGHT, TOP CONE SECTION SHALL BE REPLACED WITH PRECAST REINFORCED CONCRETE SLAB (6" MIN. THICKNESS) WITH OPENING OF SUFFICIENT SIZE TO ACCOMMODATE MANHOLE CASTING.
2. FOR MANHOLES 10 FEET OR MORE IN DEPTH, MANHOLE DIAMETER SHALL BE FIVE (5) FEET.
3. TERMINAL MANHOLE FLOORS SHALL BE SLOPED TOWARD OUTFALL PIPE.



C-402



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4

07/12/2023

ISSUE FOR BID - PHASE 1

3

06/09/2023

ISSUE FOR PERMIT - PHASE 1

2

06/01/2020

DESIGN DEVELOPMENT

1

03/31/2020

50% DESIGN DEVELOPMENT

No.

Date

Issue

Sheet Title

CONSTRUCTION
DETAILS

Job No.

2020-1005

Date

06/29/2023

Scale

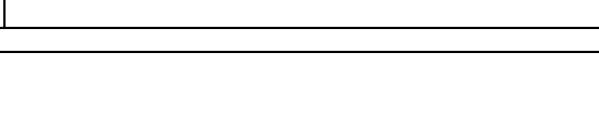
AS NOTED

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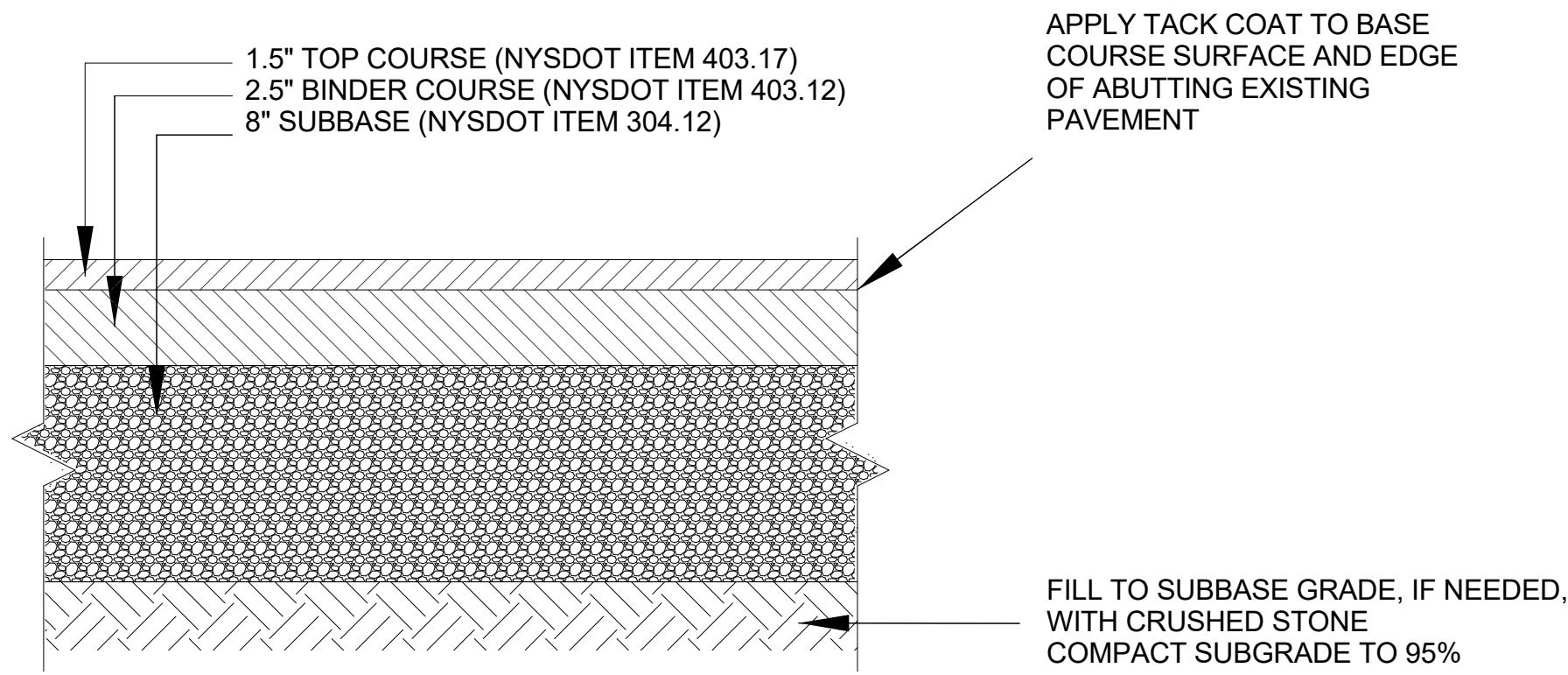
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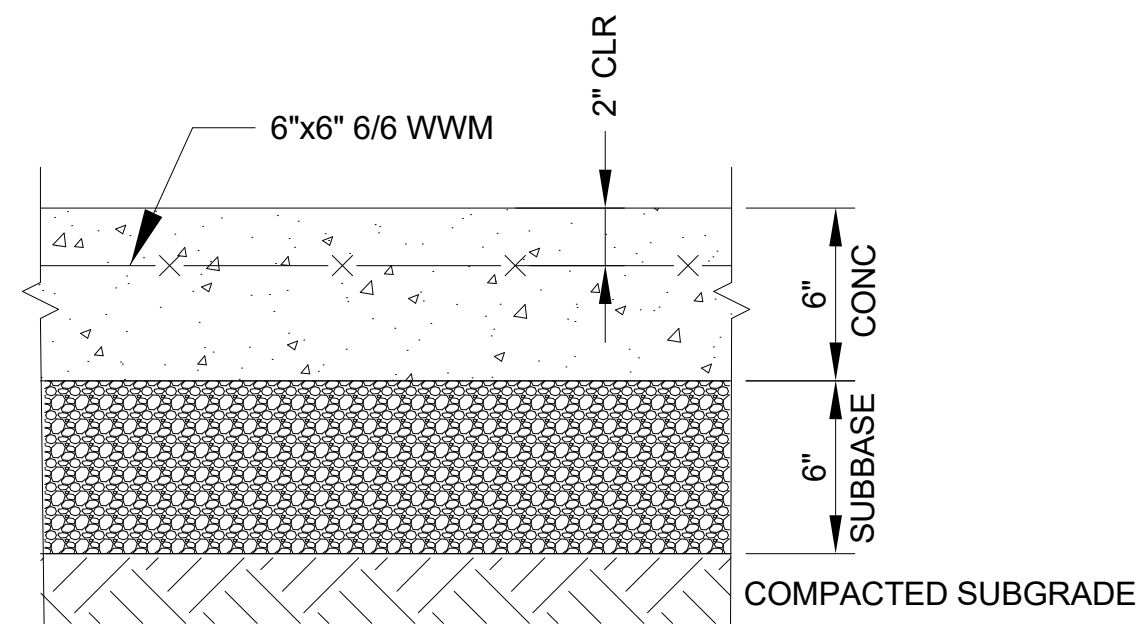
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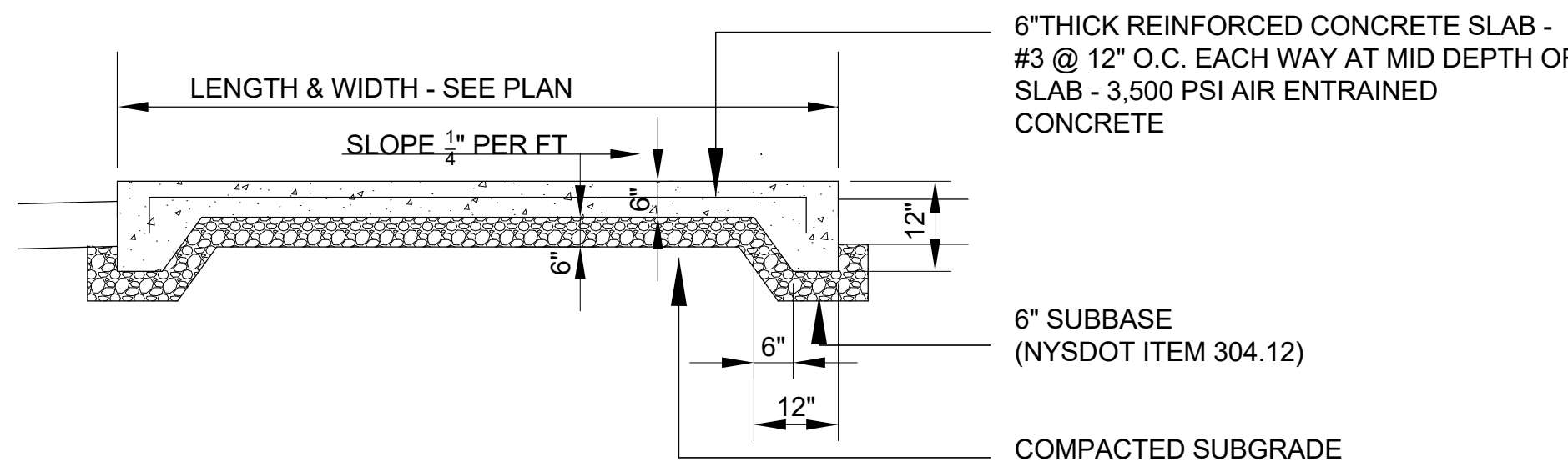
FOR REFERENCE - TYPICAL PAVEMENT SECTIONS
- PHASE 2



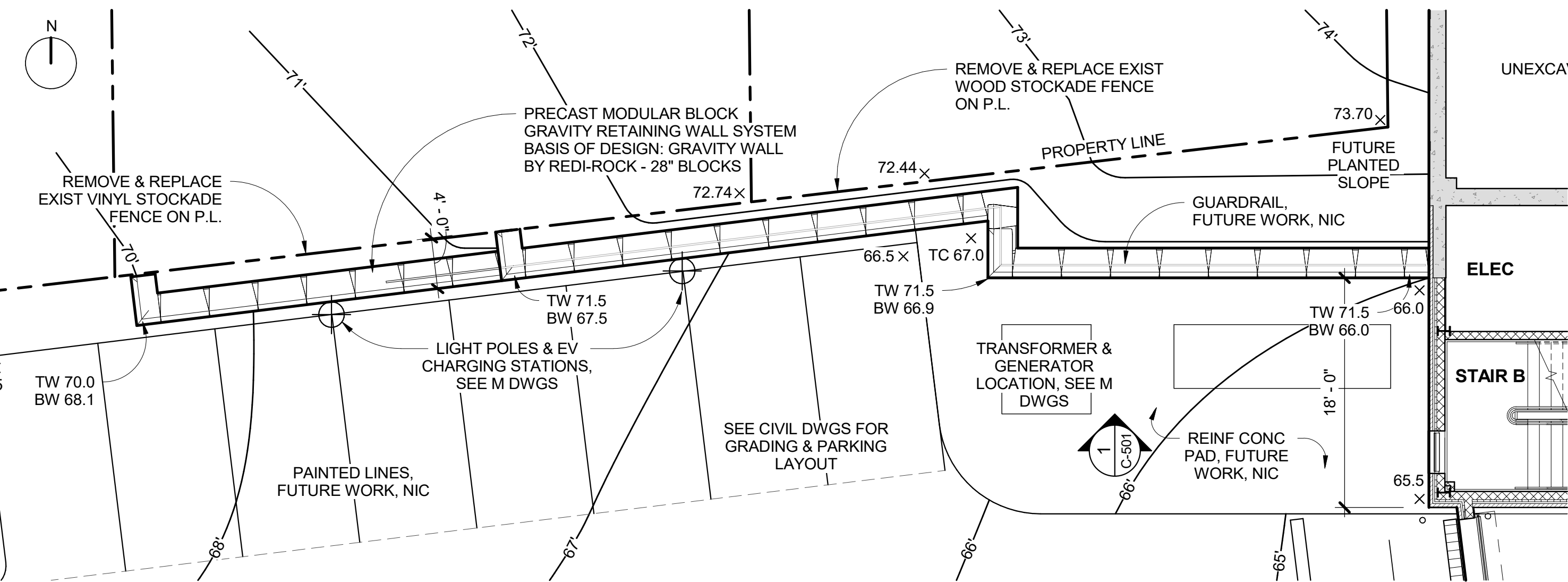
7 FULL DEPTH ASPHALT PAVEMENT
SCALE: NTS



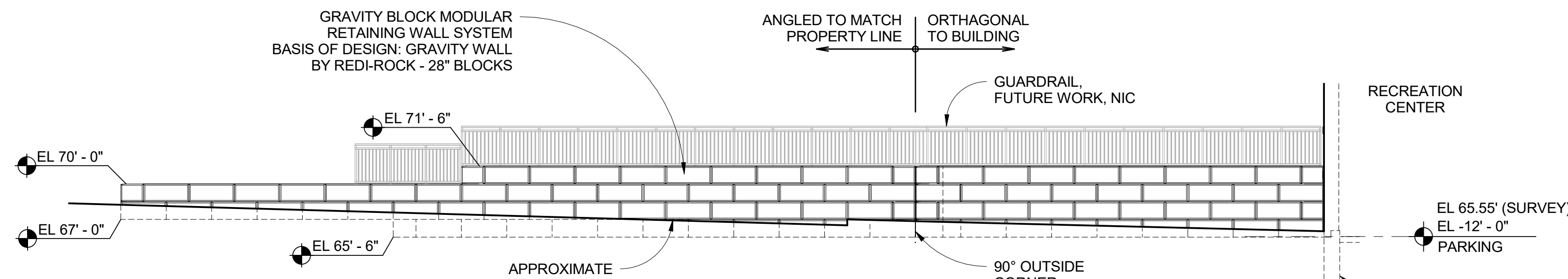
6 CONCRETE SIDEWALK
SCALE: NTS



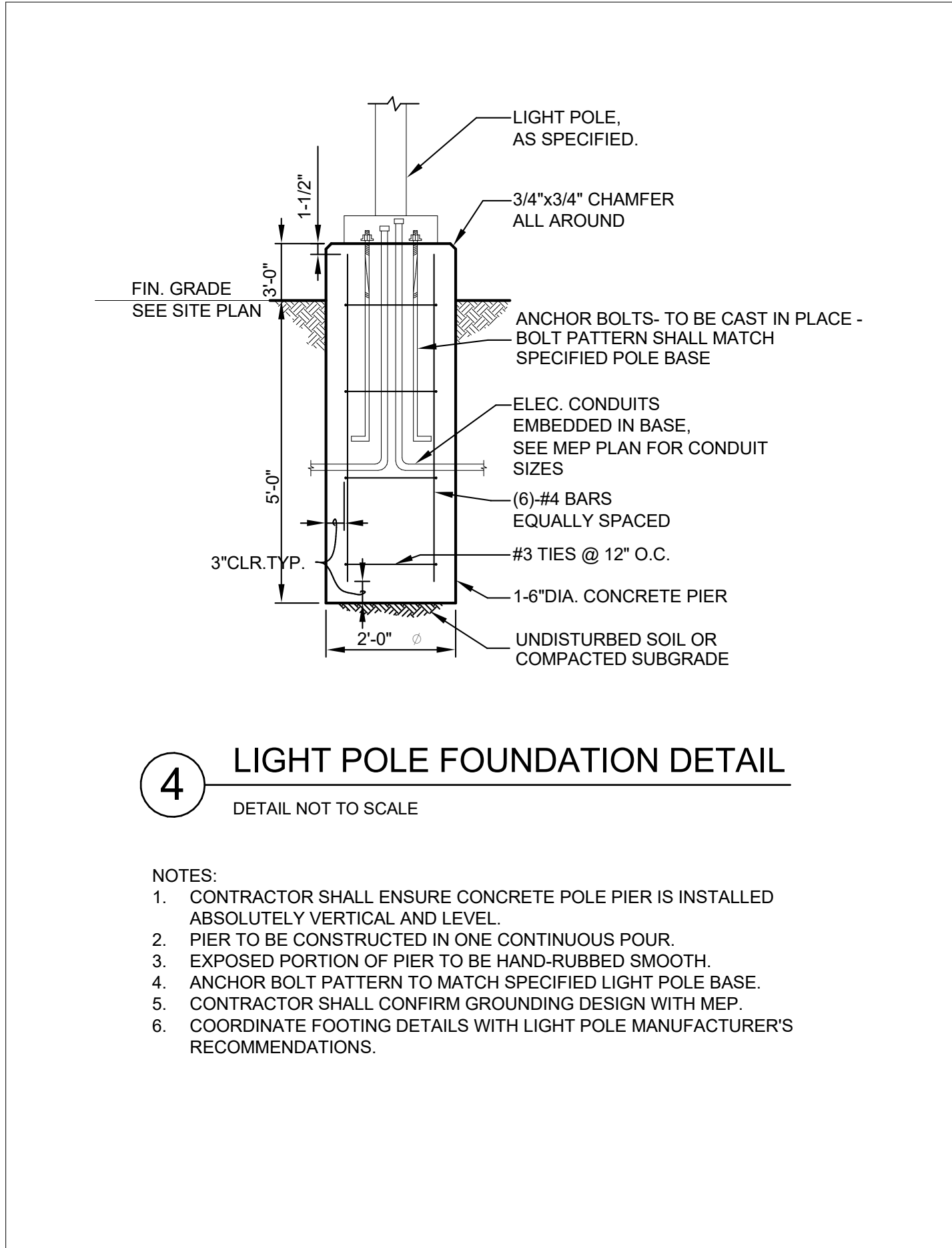
5 TRANSFORMER CONCRETE PAD
SCALE: NTS



2 NORTH RETAINING WALL PLAN
1/8" = 1'-0"

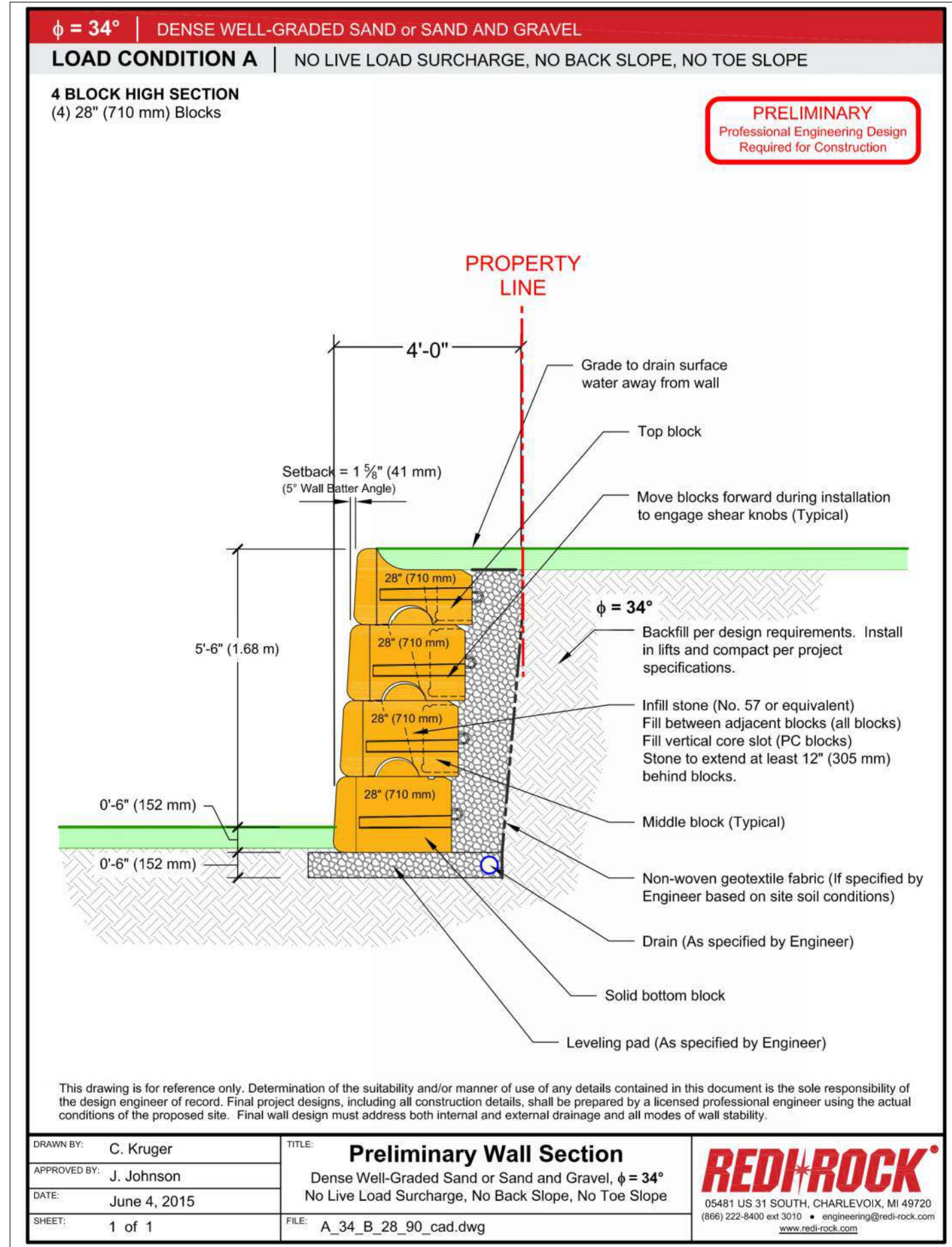


1 NORTH RETAINING WALL
1/8" = 1'-0"



4 LIGHT POLE FOUNDATION DETAIL
DETAIL NOT TO SCALE

- NOTES:
1. CONTRACTOR SHALL ENSURE CONCRETE POLE PIER IS INSTALLED ABSOLUTELY VERTICAL AND LEVEL.
 2. PIER TO BE CONSTRUCTED IN ONE CONTINUOUS POUR.
 3. EXPOSED PORTION OF PIER TO BE HAND-RUBBED SMOOTH.
 4. ANCHOR BOLT PATTERN TO MATCH SPECIFIED LIGHT POLE BASE.
 5. CONTRACTOR SHALL CONFIRM GROUNDING DESIGN WITH MEP.
 6. COORDINATE FOOTING DETAILS WITH LIGHT POLE MANUFACTURER'S RECOMMENDATIONS.



3 MODULAR BLOCK WALL SECTION
DETAIL NOT TO SCALE

ARCHITECTURAL PLAN & ELEVATION OF MODULAR BLOCK RETAINING WALL BY GC. SEE CIVIL DWGS FOR GRADING & LAYOUT PLANS. BLOCK LAYOUT SHOWN IS APPROXIMATE. GC SHALL BE RESPONSIBLE FOR DELEGATED DESIGN ENGINEERING OF WALL.

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Professional Seal

3 07/12/2023 ISSUE FOR BID - PHASE 1

SITE DETAILS

Job No. 2020-1005 Date 07/12/2023
Scale AS NOTED Drawn / Checked --
Sheet Number

C-501

General Notes:

- 1 The purpose of these drawings is to show the structural work associated with Harrison Recreational Center in Harrison, NY.
- 2 The work shown on these drawings has been designed in accordance with the structural requirements of the 2020 edition of the Building Code of New York State.
- 3 The structural components have been designed for the following loads:

A.	Uniform live load:	
	Corridors	100 psf
	First floor	
	Garages	
	Passenger vehicles	40 psf
	Office buildings	
	Corridors above first floor	80 psf
	Lobbies and first-floor corridors	100 psf
	Offices	50 psf + 15 psf partitions
	Recreational uses	
	Gymnasiums	100 psf
	Stairs and exits	
	All other	100 psf
	Live load has been reduced on girders, columns and footings in accordance with the building code	
B.	Roof loads:	
	Snow:	
	Ground snow load, p_g	20 psf
	Flat roof snow load, p_f	30 psf*
	Exposure factor, C_e	1.0
	Importance factor, I_s	1.1
	Thermal factor, C_t	1.0
	Rain loads: in accordance with Section 1611	
	Roof live load:	20 psf min
	Rainfall intensity (15-min. duration/100 year avg. interval)	2.90 in./hr.
	*Note: the flat roof snow load shall be no less than 30 psf.	
C.	Wind design data:	
	Wind loads have been determined based on Section 1609.1.1 in accordance with ASCE 7-16, Chapters 26, 27, 29 and 30, Directional Procedure	
	Risk category	III
	Basic wind speed (3-second gust):	
	Ultimate design wind speed, V_{ult}	126 mph
	Nominal design wind speed, V_{asd}	98 mph
	Exposure	B
	Internal pressure coefficient	0.18
	*a" dimension for use with components and cladding	
		13.2 ft

Design wind pressure (Nominal design wind pressures) for components and cladding on building walls (use Zone 4 generally; use Zone 5 within "a" of building wall corners):

	Surface pressure (psf)			
Area	10 sq.ft	20 sq. ft.	50 sq. ft.	100 sq. ft.
Negative Zone 4	-17.5	-16.7	-15.8	-15.1
Negative Zone 5	-21.5	-20.1	-18.2	-16.7
Positive Zones 4 & 5	16.1	15.4	14.5	13.7

Design wind pressure (Nominal design wind pressures) for components and cladding on building roofs (for locations of zones 1, 2 and 3, refer to building code):

	Surface pressure (psf)			
Area	10 sq.ft	20 sq. ft.	50 sq. ft.	100 sq.ft.
Negative Zone 1	-28.1	-26.2	-23.8	-21.9
Negative Zone 1'	-16.1	-16.1	-16.1	-16.1
Negative Zone 2	-37.0	-34.6	-31.5	-29.1
Negative Zone 3	-37.0	-34.6	-31.5	-29.1
Positive Zone 1 & 1'	10.0	10.0	10.0	10.0
Positive Zone 2 & 3	16.1	15.4	14.5	13.7
Overhang Zones 1&1'	-25.4	-24.9	-24.3	-23.9
Overhang Zone 2	-34.3	-31.1	-27.0	-23.8
Overhang Zone 3	-34.3	-31.1	-27.0	-23.8

D. Earthquake design data:

Risk category	III
Seismic importance factor, I_e :	1.25
Mapped short period spectral response accelerations, S_{S1} :	0.285g
Mapped 1 second period spectral response accelerations, S_{S1} :	0.060g
Site class:	B
Design short period spectral response accelerations, S_{DS} :	0.171g
Design 1 second period spectral response accelerations, S_{D1} :	0.032g
Seismic design category:	B
Seismic force resisting system: structural steel systems not specifically detailed for seismic resistance	
Design base shear:	125 KIPS
Seismic response coefficient, C_s :	0.028
Response modification factor, R :	3
Deflection amplification factor, C_d :	3
Analysis procedure: Equivalent Lateral Force	

E. Other loads:

Concentrated loads:	
All floors except as noted (on 2-1/2 feet square)	0 lbs
Stair treads and catwalks (on 4 inches square)	300 lbs
Garages with passenger vehicles (on 4 ½ inches square)	3000 lbs
All other roof members	300 lbs
Impact loads	
Loads increased as follows:	
Elevator machinery	100%
Hangers for floors or balconies	33%
Vehicle and wheel loads	30%

F. Special loads:

Retaining walls	
Lateral equivalent fluid pressure	32.5 pcf
Seismic load (h = height of wall)	5.5 h*2
Vertical live load surcharge	100 psf

- 4 This structure has been designed to be self-supporting and stable after the construction of the building has been completed. The stability of the structure prior to completion is solely the responsibility of the contractor. This responsibility extends to all related aspects of the construction activity including, but not limited to, erection methods, erection sequence, temporary bracing, forms, shoring, use of equipment, and similar construction procedures. Review of the construction by the engineer is for conformance with design aspects only, not to review the contractor's construction procedures. Lack of comment on the part of the engineer with regard to construction procedures is not to be interpreted as approval of those procedures.
- 5 Jobsite safety and construction procedures are solely the responsibility of the contractor. Review of the construction by the engineer is for conformance with design aspects only, not to review the contractor's provisions for job site safety. Lack of comment by the engineer is not to be interpreted as approval of those aspects of work.
- 6 PDF digital files of all erection and detail shop drawings for steel reinforcing bars, indicating the fabricator, manufacturer, finish, layout, and all accessories, must be submitted to and be checked by the contractor and subcontractor and bear the checker's initials before submission to the architect for review prior to fabrication. Fabrication and/or delivery to the site of components prior to receiving approved shop drawings shall be at the fabricator's own risk.
- 7 Testing and inspection of concrete, steel reinforcing bars, and other work are described in the project "Statement of Special Inspections". The contractor shall review the "Statement of Special Inspections" and coordinate the scheduling of inspections with the special inspector. Uninspected work that required inspections may be rejected solely on that basis.
- 8 If faulty construction procedures, or material, result in defective work that requires additional engineering time to devise corrective measures, professional fees may be charged to the contractor at the standard hourly rate of additional services. Such fees may be withheld from the general contractor's payment.
- 9 Loads, openings and structure in any way related to requirements of other (non-structural) disciplines are shown for bidding purposes only. However, these plans do not show the full scope of openings, in roofs, floors and walls. For size and location of all openings, see architectural and mechanical drawings. Do not scale openings. The contractor shall obtain from the heating and ventilating, electrical, plumbing and other trades the final approved size and location of all openings, equipment and work to be provided for their trade for roofs, floors and walls, whether shown or not shown on structural drawings. Excess cost related to variation in requirements or equipment are not to be borne by the owner.
- 10 The contractor shall verify all dimensions, elevations and angles with architectural drawings and existing conditions before proceeding with any work.

- 11 The contractor and subcontractors shall obtain the latest copies of approved plans and surveys and they shall familiarize themselves thoroughly with these plans before commencing any work.
- 12 These drawings are supplemented by a detailed technical specification. The notes shown under certain categories of work are intended to summarize basic requirements.
- 13 Work shown as "Typical Details" apply throughout the project as required. Work shown as "Sections" shall be considered to apply for the same and similar conditions in the building.
- 14 Some details of the work are shown on the architectural drawings. A careful review and study of these details are necessary before the full scope of the work can be comprehended.
- 15 Do not scale drawings.

Codes and Standards References

- 1 Concrete:
- Concrete work shall conform to the requirements of:
- ACI 301-10, "Specifications for Structural Concrete in Buildings" and
- ACI 318-14, "Building Code Requirements for Structural Concrete".
- 2 Structural steel:
- Design, fabrication and erection of structural steel shall conform to the "Specification for Structural Steel for Buildings" as adopted on July 7, 2016, by the American Institute of Steel Construction (AISC) and the 15th Edition of the AISC Steel Construction Manual.

Foundation notes:

- 1 Recommendations for the site preparation and earthwork within the perimeter of the proposed new structure, preparation of soil bearing surfaces, material specification and placement of structural backfill for support of foundations and slabs-on-grade, and drainage are stated in the geotechnical engineering report prepared for this project by Skylands Engineering, LLC and dated 19 April, 2019. The recommendations of this report are project requirements.
- 2 The foundations have been designed to rest on inorganic, undisturbed soil or compacted granular fill having a bearing value of 4000 psf as recommended in the geotechnical engineering report referenced above. Such bearing strata are anticipated at the bottom of footing elevations noted on the foundation plan. All bearing strata shall be reviewed by the geotechnical engineer prior to placing concrete in order to verify the bearing value.
- 3 If ledge rock is encountered above the proposed bottom of footing elevations indicated, then it shall be removed to the extent necessary to provide a minimum of 1'-0" soil cover above the top of the concrete footing.
- 4 Footings supported on ledge shall rest on broom clean solid rock. If the slope of the rock surface exceeds 1 on 6, then the footing shall be dowelled to the ledge with 3/4" steel rods drilled 10 inches into the rock surface at 2 feet on center.
- 5 If ledge rock and soil both occur at the bottom of footing bearing elevation, then a minimum of one foot of ledge rock below planned bottom of footing elevation shall be removed and replaced with a "cushion" layer of crushed stone to minimize differential settlement. The crushed stone "cushion" layer shall extend beyond the end of the ledge rock for a minimum of 10 feet along the length of the supported footing.
- 6 The slab-on-grade sub-base shall be a crusher run stone free from soft disintegrated pieces, mud, dirt, or other injurious material. The material shall have no stone greater than 2 inches in any one dimension and with less than 10 percent by weight passing a No.100 sieve.
- 7 The bottom of exterior footings not on solid rock shall be at least 3' - 6" below finished grade.
- 8 All soil surrounding and under footings shall be protected from freezing and frost action during the course of construction.
- 9 Step footings where elevations change at a maximum slope of one vertical on two horizontal and place lower footings first.
- 10 Foundation basement walls shall not be backfilled before they are temporarily braced or before the concrete has attained its specified compressive strength and the framed floor system at the top of the wall is in place.
- 11 Foundation frost walls shall be backfilled by placing fill on both sides simultaneously and to the same level.
- 12 Keep foundation excavations free of water at all times.
- 13 Use crushed stone backfill or controlled compacted fill or lean concrete (f_c =1500 psi) for over-excavation of footings.
- 14 Existing utilities: locate existing underground utilities in areas of excavation work. Provide adequate means of support and protection during earthwork operations.
- 15 Where footings are in close proximity to sub-surface piping bottom of footings shall be at least 8" below elevation of piping unless otherwise shown on the drawings.
- 16 Submittals to the engineer are required for structural fill and slab sub-base.

Concrete Notes:

- 1 All concrete work shall conform to all the requirements of ACI 301, "Specifications for Structural Concrete in Buildings" and ACI 318 "Building Code Requirements for Structural Concrete", as specified in the code reference section of these general notes.
- 2 Concrete shall be the specified weight and develop a minimum compressive strength in 28 days as follows:

Location	Weight	Minimum Strength	Maximum W/C Ratio (or slump where indicated)
Footings	Normal	3,000 psi	0.55
Walls and piers:			
Interior	Normal	3,000 psi	0.55
Exterior	Normal	4,000 psi	0.45
Exterior exposed	Normal	5,000 psi	0.40

- 3 All detailing fabrication, and erection of reinforcing bars, unless otherwise noted, must follow the latest ACI code and the latest ACI "Manual of Standard Practice for Detailing Reinforced Concrete Structures".
- 4 Concrete design mix will be submitted to the engineer for review, together with laboratory reports attesting that the mixes can attain the minimum strength required in accordance with ACI 301 indicated above.
- 5 No admixtures are permitted without the engineer's written permission other than entrained air. Concrete exposed to the weather, such as that used in foundation walls, shall contain 5% +/- 1 1/2% entrained air. Concrete exposed to the weather and to de-icing compounds shall contain 6% +/- 1 1/2% entrained air. Do not use air entrainment admixture for interior normal weight concrete slabs.
- 6 Limit water-soluble, chloride-ion content in hardened concrete to the following percent by weight of cement: 1.00 for reinforced concrete that will be dry and protected from moisture, 0.30 for reinforced concrete that will be exposed to moisture but not exposed to chlorides, 0.15 for reinforced concrete exposed to moisture and chlorides from deicing chemicals and salt/seawater, and 0.06 for prestressed (post-tensioned) concrete.
- 7 Reinforcing steel shall conform to ASTM A 615, Grade 60.
- 8 The following concrete cover shall be provided for reinforcement:

Location	Cover (inches)
Concrete cast against and Permanently exposed to earth	3
Concrete exposed to earth or weather:	
#6 through #18 bars	2
#5 bar and smaller	1 1/2
Concrete not exposed to weather or in contact with ground:	
Slabs, walls, joists:	
#14 and #18 bars	1 1/2
#11 bar and smaller	3/4
Beams, columns	
Primary reinforcement, ties, stirrups, spirals	1 1/2

- 9 The conveyance, placement and protection of the concrete shall conform to the requirements of ACI 318, indicated above, and ACI 304R, "Guide for Measuring, Mixing, Transporting and Placing Concrete". Mechanical vibrators are to be used to consolidate the freshly cast concrete around the reinforcing and against form surfaces and to prevent the formation of air or stone pockets, honeycombing, pitting or planes of weakness. However, care must be used to avoid over vibration that can lead to aggregate segregation.
- 10 No welding of reinforcing will be permitted.
- 11 All lap splices shall be Class B, in accordance with ACI 318 indicated above.
- 12 Concrete piers: Place concrete piers and walls together. Set pier reinforcing and set wall reinforcing through pier vertical bars. Provide dowels with standard hook from footing at all piers. Size and quantity of dowels to match vertical pier reinforcing (Class "B" splice).
- 13 The contractor shall be responsible for limiting pours to minimize shrinkage cracking. In general, walls shall not be poured in continuous lengths exceeding 30 feet without providing construction joints or control joints. The location and configuration of joints exposed to view shall be coordinated with the architect.
- 14 The installation of slabs shall conform to the requirements of ACI 302.1R, "Guide to Concrete Floor and Slab Construction". Interior finish slab surfaces are to have a steel trowel finish. Exterior slab surfaces are to have a broom finish unless specified on the architectural drawings.
- 15 Sizes and locations of all required embedded items, such as anchor bolts, piping sleeves, etc., for all trades shall be coordinated by the general contractor with other trades.
- 16 Submittals to the engineer are required for concrete mix designs, cement, reinforcing bars, admixtures, and aggregates.

Connections to Hardened Concrete:

- 1 All proprietary anchoring systems (expansion, adhesive anchoring systems, etc.) to be installed into hardened concrete elements are to be installed in strict accordance with the manufacturer's instructions for drilling and preparation of holes, for spacing and edge distance requirements, and for the utilization of supplemental components for the anchoring systems such as screen tubes, doweling adhesives, etc.
- 2 Connections to hardened concrete shall be made with anchors conforming to ACI 318, as specified in the code reference section of these general notes, for cracked concrete, and Chapter 19 of the state building code indicated at the beginning of these general notes.
- A. Mechanical anchors shall be either
- Hilti "Kwik Bolt TZ" expansion anchor.
 - Hilti "Kwik HUS-EZ" screw anchor (use only in permanently dry, interior non-corrosive environments)
 - Simpson "Strong Bolt 2" expansion anchor
 - Simpson "Titen HD" screw ancho, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments)
 - Dewalt "Power-Stud + SD2" expansion anchor.
 - Dewalt "Power-Stud + SD4/SD6", Type 304/316 SS expansion anchor
 - Dewalt "Screw-Bolt+" screw anchor, zinc-plated or galvanized (use only in permanently dry, interior non-corrosive environments)

- Size, embedment, spacing and edge distance of anchors shall be as indicated on the drawings.
- B. Adhesive anchor rods or reinforcing bars shall be installed in rotary hammered drilled holes with carbide drill bits using one of the following adhesive anchoring systems:
- Hilti "HIT-HY 200 safe set adhesive anchoring system with Hilti "HAS" ASTM F1554, Grade 36 anchor rods.
 - Simpson "AT-XP" adhesive anchoring system for base material temperatures between 14 degrees and 80 degrees or Simpson "Set-3G" adhesive anchoring system for temperatures above 40 degrees, with Simpson "RFB" ASTM F1554 Grade 36 anchor rods.
 - Dewalt "AC2004+" two part adhesive, cold temperature cure with ASTM F1554, Grade 36 anchor rods. Reinforcing bars shall conform to the requirements of the Concrete General Notes.
 - Reinforcing bars shall conform to the requirements of the Concrete General Notes.

Adhesive for reinforcing bars and anchors shall have been tested in accordance with ACI 355.4 "Qualification of Post-Installed Adhesive Anchors in Concrete" and ICC-ES (ICC Evaluation Service) "Acceptance Criteria for Post-Installed Adhesive Anchors in Concrete Elements" (AC308) for cracked concrete and seismic applications.

Adhesive bond design strength is based upon concrete that has cured at least 21 days with a minimum compressive strength of 2,500 psi and an in-service temperature in accordance with ACI 355.4 Temperature Category B.

Installation method shall be in accordance with the Manufacturer's Printed Installation Instructions (MPII)

Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by the ACI/CRSI Adhesive Anchor Installer Certification program or equal.

Unless otherwise noted on the drawings, embed anchor rods and reinforcing bars into drilled holes a minimum of 9 anchor diameters, with a minimum edge distance of 4 inches, measured from the edge of the concrete to the centerline of the anchor/reinforcing bar. Increased embedment depths or edge distances may be required at certain locations, see plans and details.

Structural Steel Notes:

- 1 Design fabrication and erection of structural steel shall conform to the American Institute of Steel Construction's "Specification for Structural Steel for Buildings", as specified in the code reference section of these general notes.
- 2 Materials:
- American standard shapes, angles,
- Plates and bars: ASTM A 36
- Anchor rods: ASTM F 1554, Grade 36
- Welding electrode: ASTM E 70xx, low hydrogen
- 3 All welding shall conform to American Welding Society's AWS D1.1 "Structural Welding Code-Steel" code for arc and gas welding and be performed by a certified welder in accordance with A.W.S. standards.
- 4 Structural steel shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 3 for Power Tool Cleaning (except for steel exposed to weather).
- 5 All steel members and bolting exposed to weather shall be cleaned in accordance with the Steel Structures Painting Council Specification SP 6 for Commercial Blast Cleaned and hot-dipped galvanized in accordance with ASTM A 123 and ASTM A 153. Minimum acceptable zinc coating weight shall be 2 oz./sq. Ft. See architectural specifications for finished paint if required. Clean areas where galvanizing is damaged or missing and repair galvanizing to comply with ASTM A 780.
- 6 Provide bitumastic protection coating for all structural steel below grade.
- 7 Fabricator shall hold a current AISC certification for "Certified Building Fabricator (BU)", (formerly known as "Standard for Steel Building Structures (STD)".)
- 8 For miscellaneous steel, see architectural drawings.
- 9 Submittals to the engineer are required for certificates of compliance for structural steel, bolts, nuts, and weld filler material prior to the fabrication of any steel
- 10 At the completion of fabrication, the fabricator shall submit a certificate of compliance stating that the work was performed in accordance with the approved contract documents, as required by Section 1704.2 of the building code indicated at the beginning of these General Notes.

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New Construction - Phase 1

Town / Village of Harrison

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P 914.666.5900 KGDARCHITCTS.COM



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CONSTRUCTION DOCUMENTS

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GENERAL
STRUCTURAL
NOTES

Job No. 20029.00 Date 2020-06-01

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Sheet Number

S-001

HARRISON
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New Construction - Phase 1

Town / Village of Harrison

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LOWER LEVEL
FOUNDATION
PLAN

Job No. 20029.00 Date 2020-06-01

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Sheet Number

S-100

NOTE:
SEE SITE/ELEC MECH. DNG'S
FOR EQUIP. PAD
LOCATIONS. SEE 2/S202
FOR TYP. PAD DETAIL

NOTE:
SEE SITE/CIVIL DNG'S
FOR EXTENT OF SITE
RETAINING WALLS IN
PARKING LOT



LOWER LEVEL FOUNDATION PLAN

1/8"=1'-0"

NORTH

- TOP OF PARKING LEVEL = -12'-0" FROM FIRST FLOOR DATUM
- FOR FOUNDATION CONDITION AT EXTERIOR DOORS, SEE 'TYPICAL DETAIL AT EXTERIOR DOORS'.
- COORDINATE ALL PITS, TRENCHES, POCKETS, BRICK SHELVEES, SLEEVES, PENETRATIONS AND INSERTS IN CONCRETE WALLS AND SLABS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SEE GENERAL NOTES ON SHEET 5001 FOR ADDITIONAL INFORMATION.

LEGEND:

- [/- ...] INDICATES BOTTOM OF FOOTING ELEVATION FROM FIRST FLOOR DATUM ELEVATION.
- [+/- ...] INDICATES TOP OF WALL/PIER ELEVATION FROM FIRST FLOOR DATUM ELEVATION.
- 'SF' INDICATES APPROXIMATE LOCATION OF STEPS IN FOOTINGS ON PLAN. COORDINATE LOCATION AND ELEVATION WITH SITE GRADING AND MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. FOR DETAILS, SEE 'TYPICAL STEPPED FOOTING DETAIL'.
- F-## INDICATES FOOTING TYPE. SEE 'FOOTING SCHEDULE AND FOOTING DETAIL'.
- P-# INDICATES PIER TYPE. SEE 'PIER SCHEDULE AND PIER DETAILS'.
- 'AR-#' INDICATES BASEPLATE AND ANCHOR BOLT SIZES AND DIMENSIONS. SEE 5/S202.
- NOTE 'A': PROVIDE (3)-5'-0" WIDE X 1'-6" HIGH OPN'S IN FDN. WALL FOR PIPE PENS. MAINTAIN 1'-0" WIDE PIER BTWN. OPN'S. PROVIDE ADD'L (3)-#5 BARS ABOVE 4' BELOW OPN'S & EXTEND 2'-6" BEYOND OPN'S, EA. SIDE. PROVIDE (4)-#5 VERT. AT PIERS. COORD. W/ MEP DNGS FOR LOCATION AND ELEVATION.
- NOTE 'B': PROVIDE (2)-3'-0" WIDE X 1'-0" HIGH OPN'S IN FDN. WALL FOR CONDUIT PENS. MAINTAIN 1'-0" WIDE PIER BTWN. OPN'S. PROVIDE ADD'L (3)-#5 BARS ABOVE 4' BELOW OPN'S & EXTEND 2'-6" BEYOND OPN'S, EA. SIDE. PROVIDE (4)-#5 VERT. AT PIER. COORD. W/ MEP DNGS FOR LOCATION AND ELEVATION.

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Harrison, NY 10528



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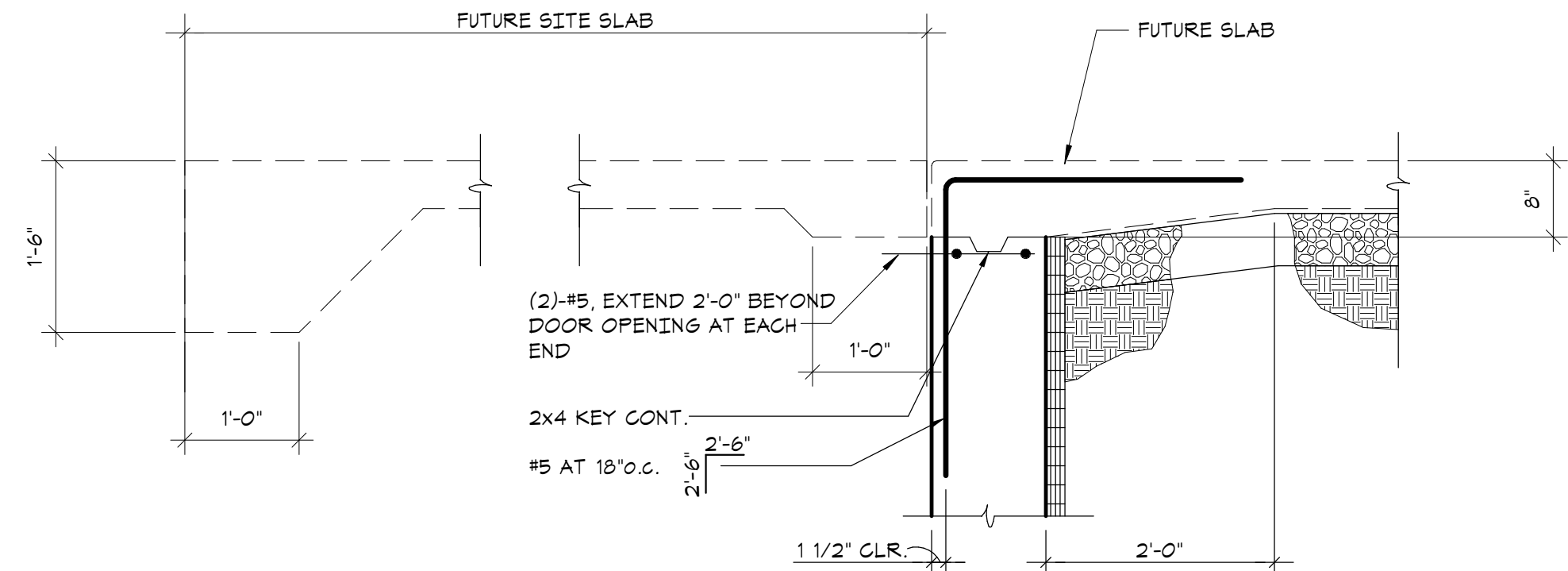
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4	06/09/2023	ISSUE FOR PERMIT - PHASE
2	2023-03-31	PHASE 1 - 50% CD'S
1	2020-06-01	DESIGN DEVELOPMENT
No.	Date	Issue

No.	Da
Sheet Title	

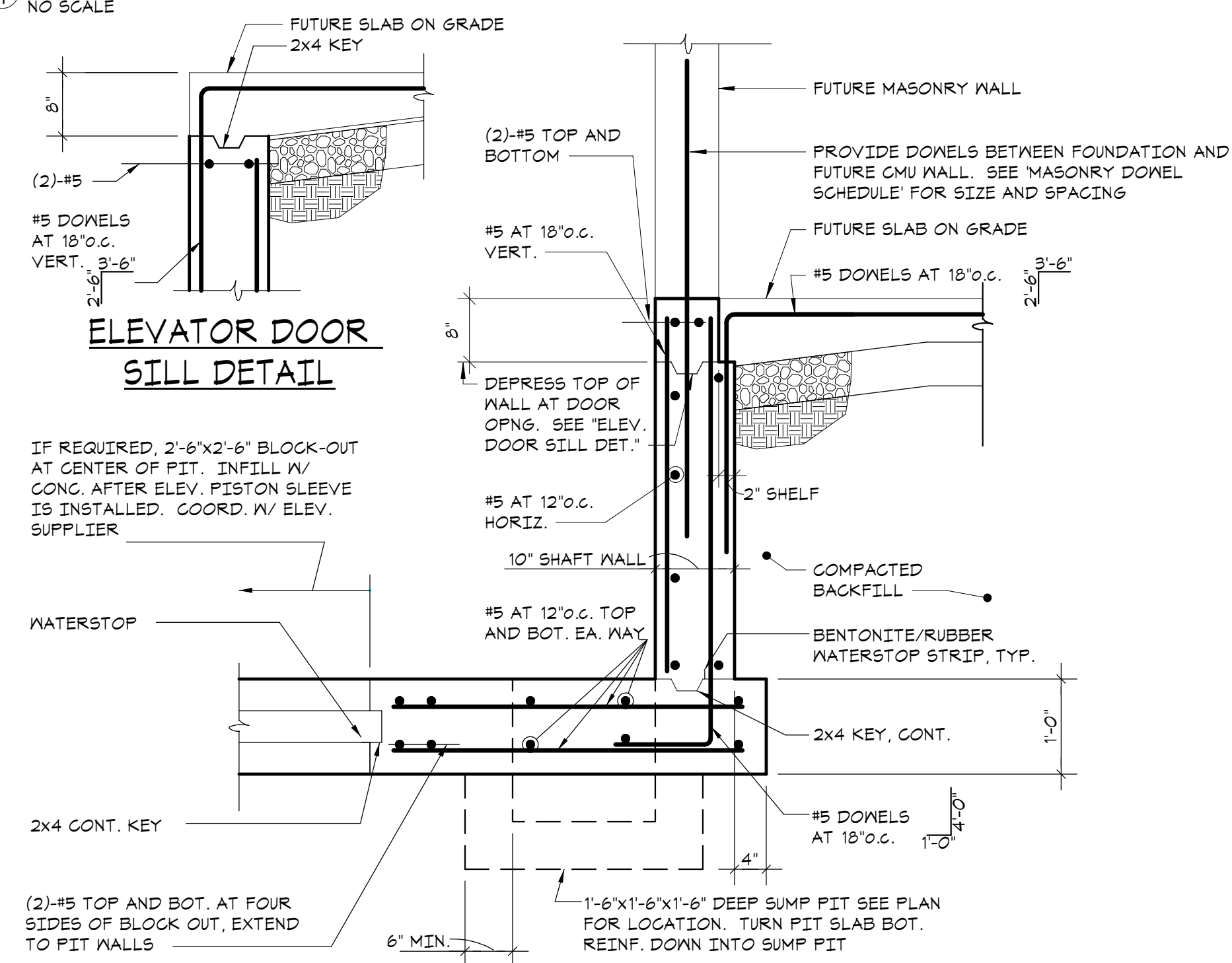
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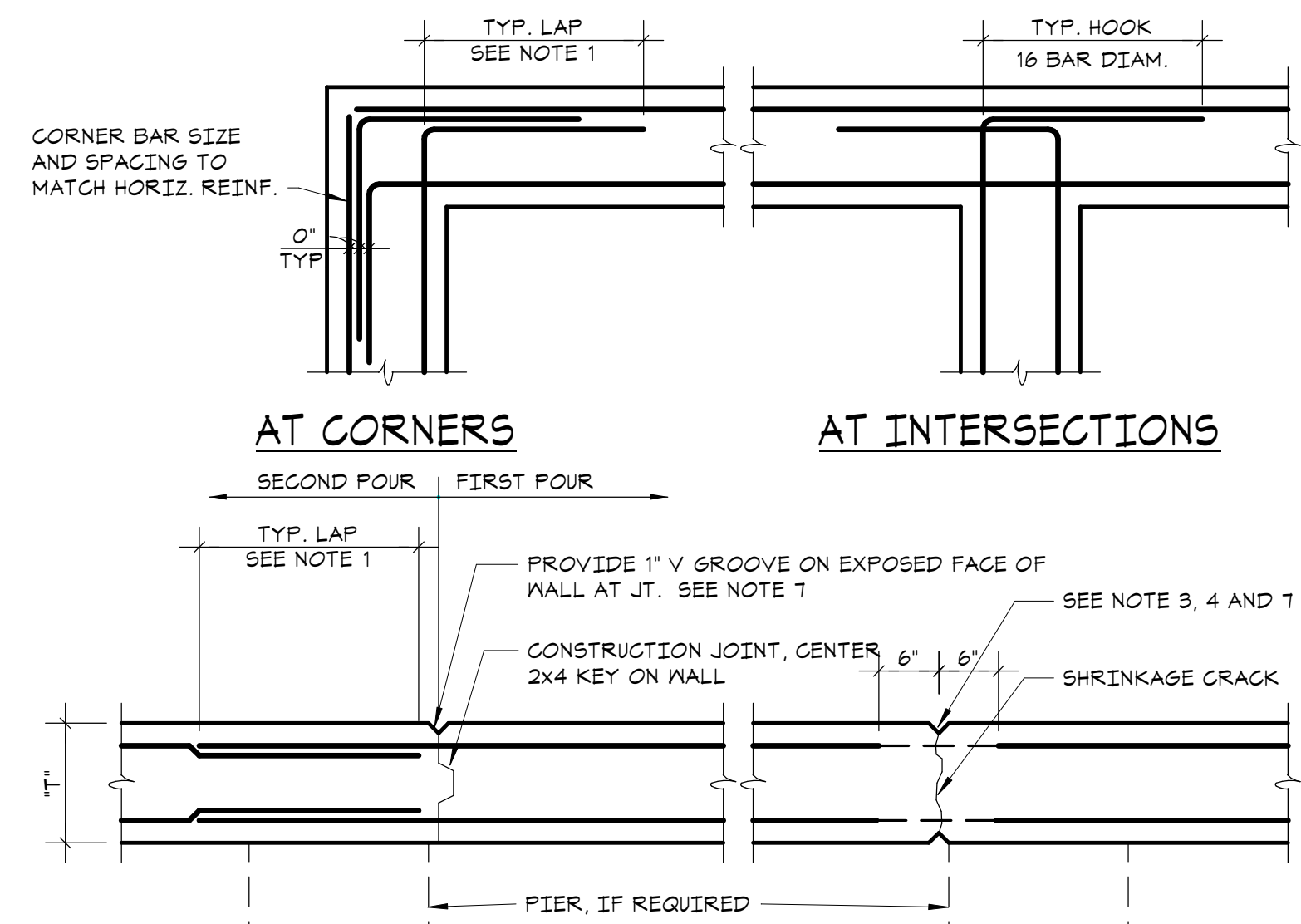


- NOTES:
- SEE APPROPRIATE SECTION FOR REMAINDER OF FOUNDATION INFORMATION NOT REPEATED HERE.
 - SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR DOORS.

1 TYPICAL FOUNDATION WALL DETAIL AT EXTERIOR DOORS



2 TYPICAL ELEVATOR PIT DETAIL



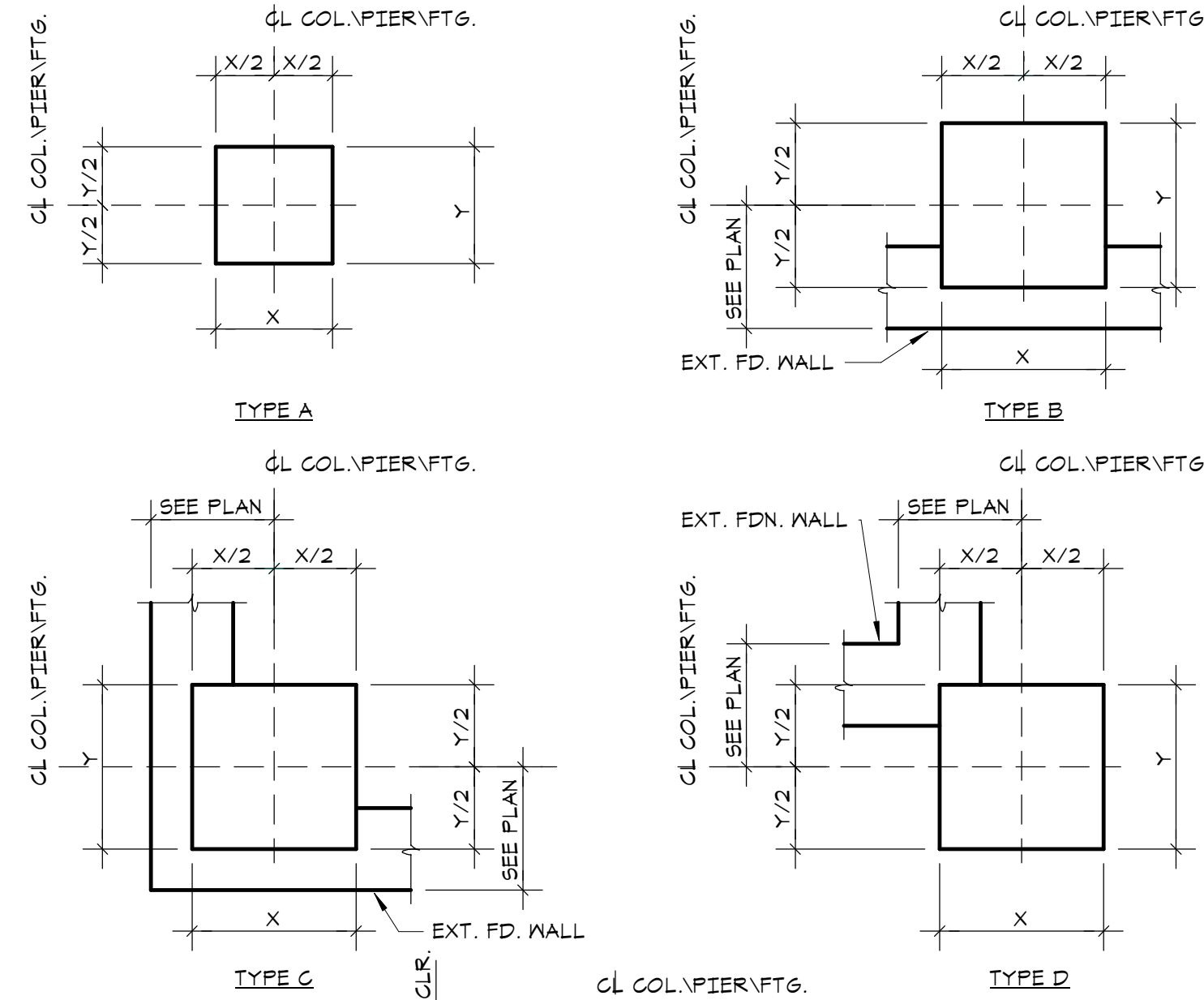
3 TYPICAL PIER SCHEDULE AND PIER DETAILS

CONSTRUCTION JOINT CONTROL JOINT

6 TYPICAL CONCRETE WALL CONSTRUCTION DETAIL

SCHEDULE OF EMBEDMENT AND SPLICE LENGTH																		
BAR SIZE	COMPRESSION		TENSION (F _c = 3000 psi)								TENSION (F _c = 4000 psi)							
	EMBED. LENGTH	LAP SPLICE LENGTH	EMBEDMENT LENGTH				LAP SPLICE LENGTH				EMBEDMENT LENGTH				LAP SPLICE LENGTH			
			TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
			CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1		CATEGORY SEE NOTE 1	
			I	II	I	II	I	II	I	II	I	II	I	II	I	II	I	II
#3	9"	12"	22"	33"	17"	25"	24"	43"	23"	33"	19"	28"	15"	22"	25"	31"	20"	24"
#4	11"	15"	24"	43"	22"	33"	38"	56"	24"	43"	25"	37"	14"	24"	33"	44"	25"	38"
#5	14"	19"	36"	54"	26"	42"	47"	71"	31"	55"	31"	47"	24"	36"	41"	62"	32"	47"
#6	17"	23"	43"	65"	33"	50"	56"	85"	43"	65"	37"	56"	24"	43"	44"	73"	38"	56"
#7	20"	27"	63"	94"	48"	72"	82"	123"	63"	94"	54"	81"	42"	63"	71"	106"	55"	82"
#8	22"	30"	72"	107"	55"	83"	94"	140"	72"	108"	62"	93"	48"	72"	81"	121"	63"	94"
#9	25"	34"	81"	121"	62"	93"	106"	158"	81"	121"	70"	105"	54"	81"	91"	131"	71"	106"
#10	28"	39"	91"	136"	70"	105"	114"	171"	91"	131"	74"	118"	61"	91"	103"	154"	80"	114"
#11	31"	43"	101"	151"	78"	116"	132"	191"	101"	151"	81"	131"	67"	101"	114"	171"	88"	132"
#14	38"	NP	121"	181"	93"	140"	NP	NP	NP	NP	105"	151"	81"	121"	NP	NP	NP	NP
#18	50"	NP	161"	242"	124"	186"	NP	NP	NP	NP	140"	204"	108"	161"	NP	NP	NP	NP
SEE NOTE 2	LCE	LCS	LTE				LTS				LTE				LTS			

10 SCHEDULE OF EMBEDMENT AND SPLICE LENGTH



TYPICAL TIE ARRANGEMENT

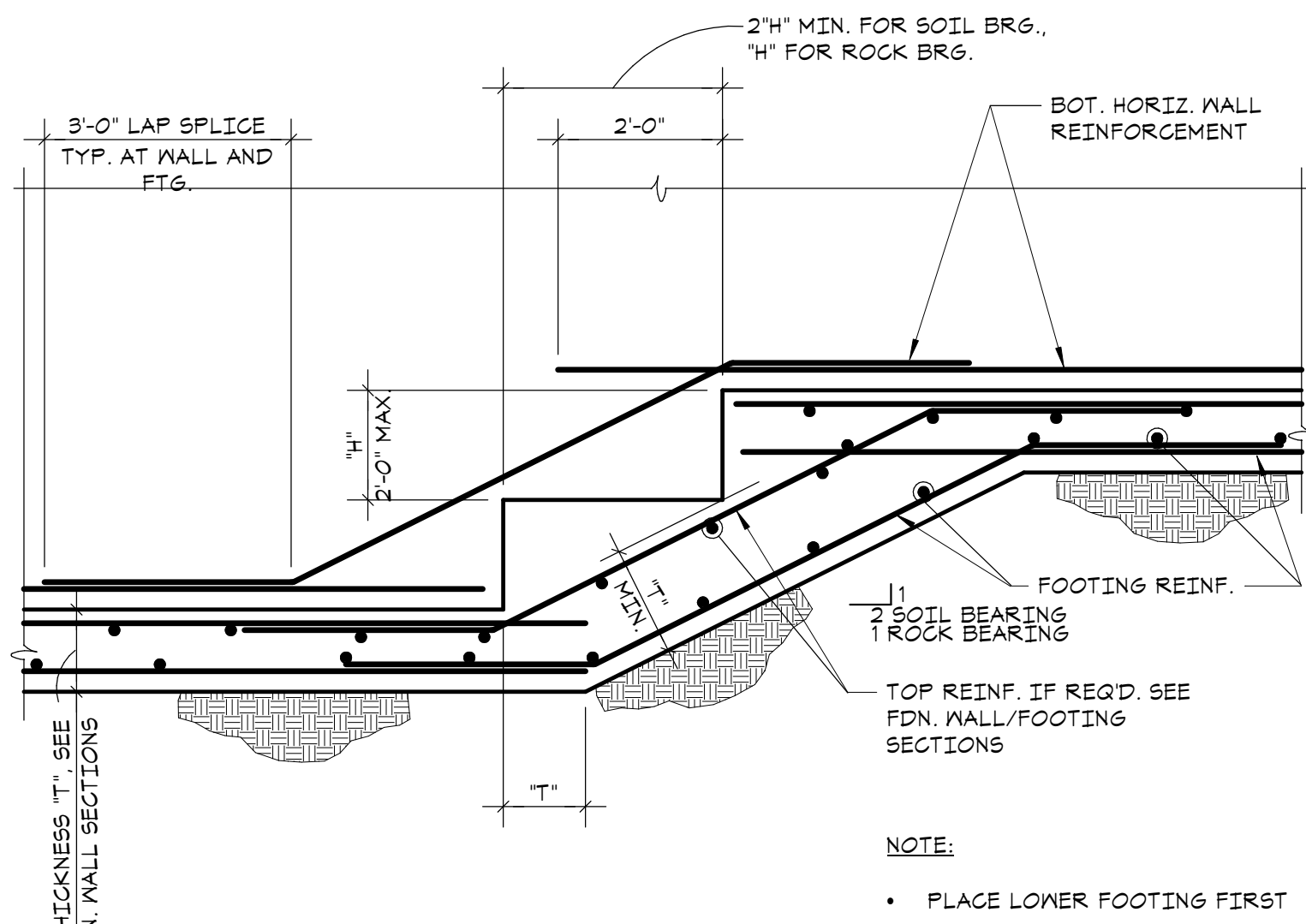
MAXIMUM SPACING OF TIES

VERTICAL BAR SIZE	SIZE AND SPACING OF TIES		
	#3	#4	#5
#5	10"	-	-
#6	12"	-	-
#7	14"	-	-
#8	16"	16"	-
#9	18"	18"	-
#10	18"	20"	-
#11	†	22"	22"
#14	†	24"	21"
#18	†	24"	30"

- NOTES:
- † INDICATES MAX. SPACING NOT TO EXCEED LEAST DIMENSION OF COMPRESSION MEMBER.
 - † INDICATES #3 TIES NOT PERMITTED.

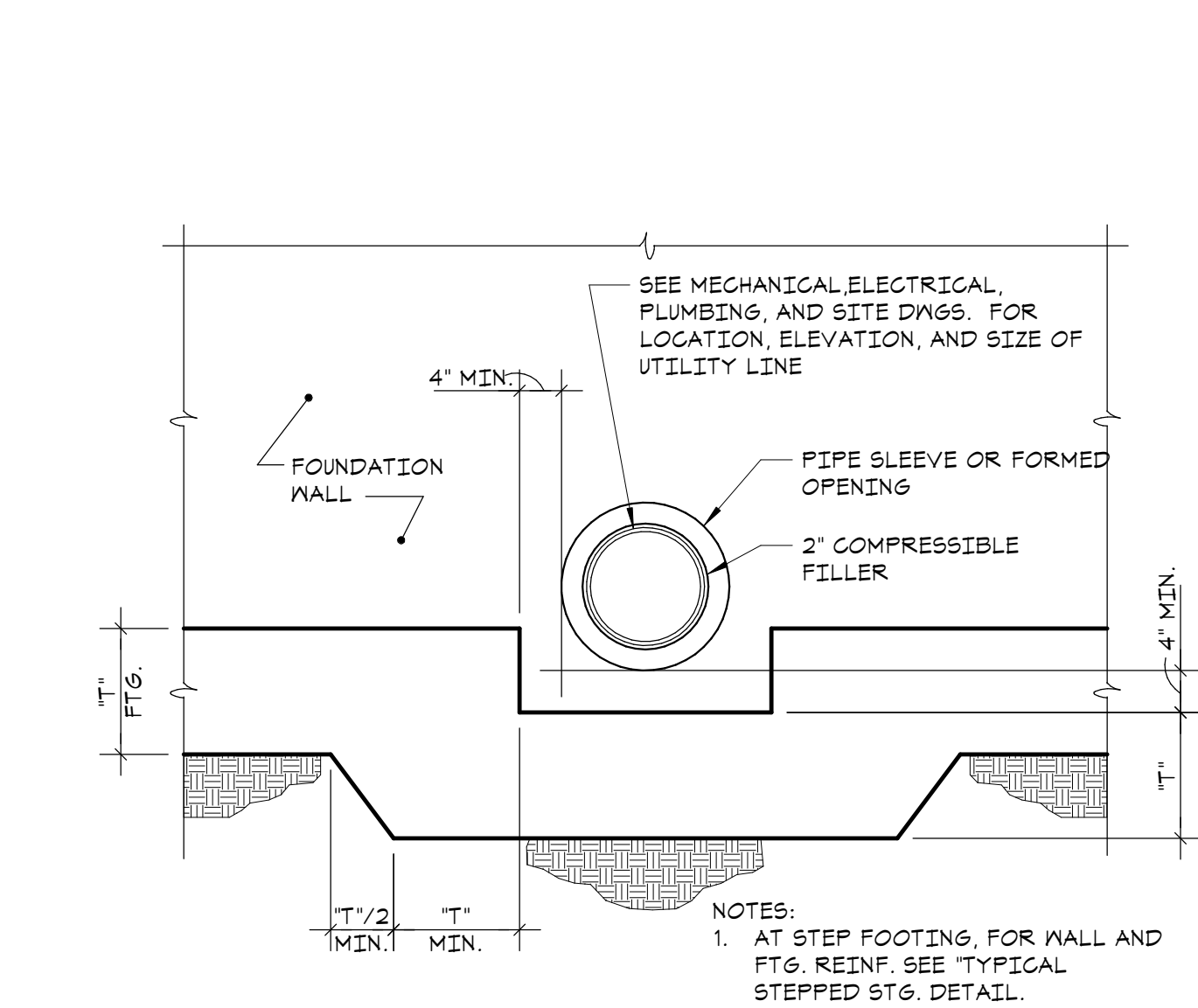
- NOTES:
- THE VERTICAL BARS SHALL BE SPACED AROUND THE PERIMETER SO AS TO ACHIEVE APPROXIMATELY EQUAL SPACING.
 - SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR DOORS.
 - TIES SHALL BE ARRANGED SUCH THAT EVERY CORNER AND ALTERNATE VERTICAL BAR SHALL HAVE LATERAL SUPPORT PROVIDED BY THE CORNER OF A TIE WITH AN INCLUDED ANGLE OF NOT MORE THAN 135 DEGREES AND NO BAR SHALL BE FURTHER THAN 6" CLEAR ON EACH SIDE ALONG THE TIE FROM SUCH A LATERALLY SUPPORTED BAR.
 - TIES SHALL BE LOCATED VERTICALLY NOT MORE THAN ONE-HALF A TIE SPACING ABOVE THE TOP OF FOOTING OR SLAB IN ANY STORY, AND SHALL BE SPACED AS PROVIDED HEREIN TO NOT MORE THAN ONE-HALF A TIE SPACING BELOW THE LOWEST HORIZONTAL REINFORCEMENT IN SLAB OR DROP PANEL ABOVE.
 - WHERE BEAMS OR BRACKETS FRAME FROM FOUR DIRECTIONS INTO A COLUMN, PIER OR BUTTRESS, TERMINATION OF TIES NOT MORE THAN 3" BELOW REINFORCEMENT IN SHALLOWEST OF SUCH BEAMS OR BRACKETS SHALL NOT BE PERMITTED.
 - SEE THE GENERAL NOTES ON DRAWING 91 FOR THE MINIMUM CONCRETE COVER REQUIREMENTS FOR REINFORCEMENT.
 - WHERE PIERS ARE ADJACENT TO FOUNDATION WALLS, PIERS ARE TO BE POURED MONOLITHICALLY WITH WALLS, PIER REINFORCING SHALL EXTEND WITHIN 2" OF OPPOSITE FACE WALL.

4 TYPICAL COLUMN, PIER AND BUTTRESS TIE DETAILS

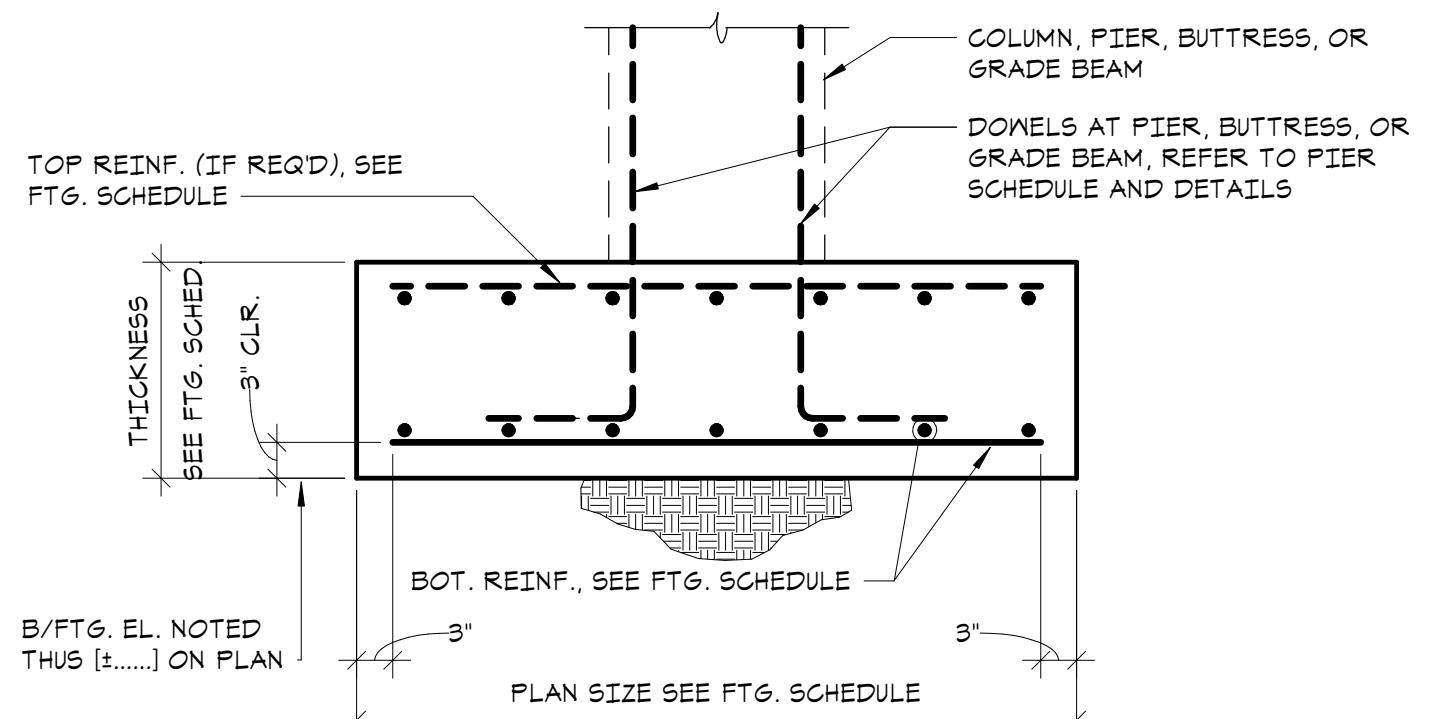


7 TYPICAL STEPPED FOOTING DETAIL

5 4000 PSF FOOTING SCHEDULE AND DETAIL

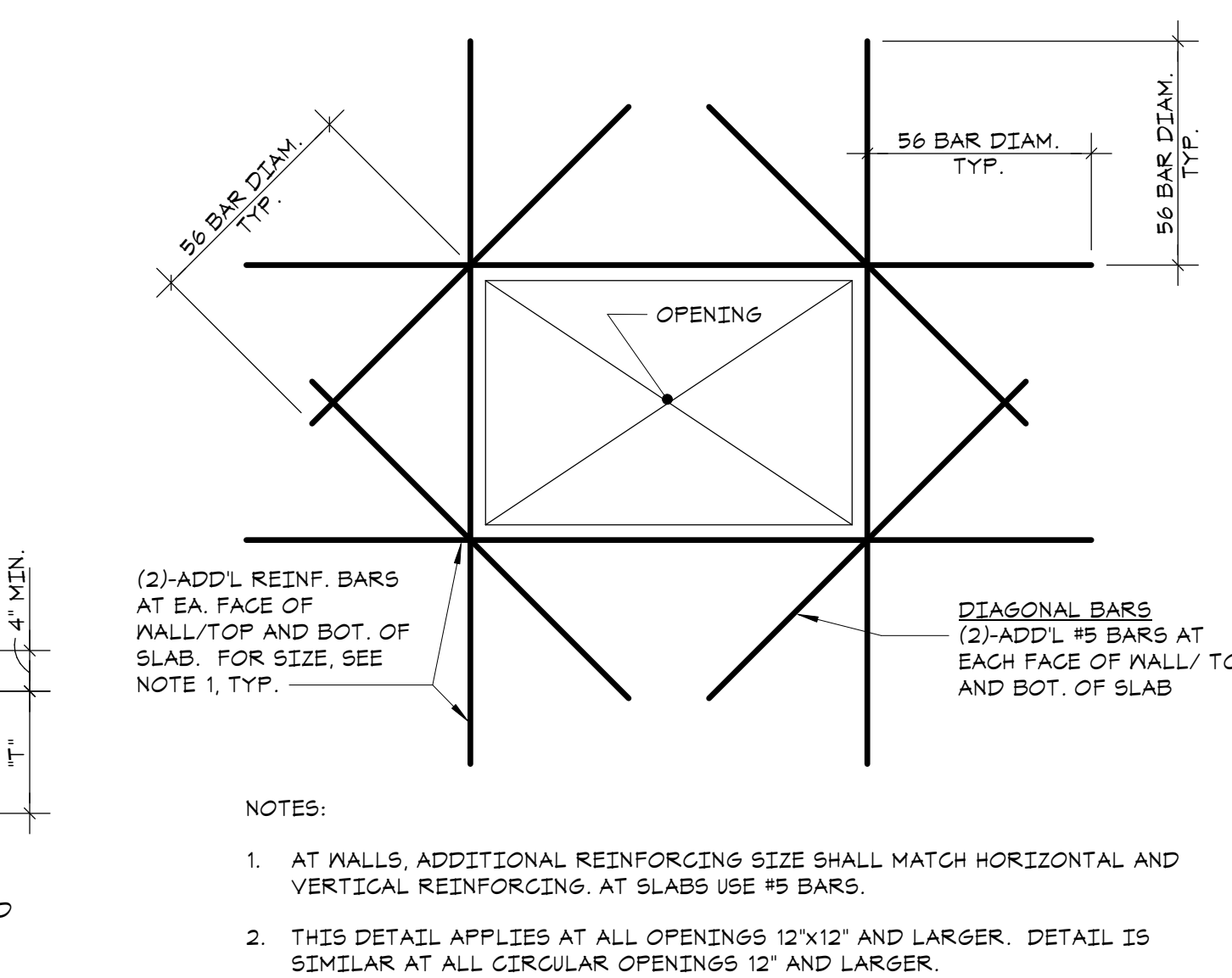


8 TYPICAL STEP FOOTING DETAIL AT WALL PIPE PENETRATION

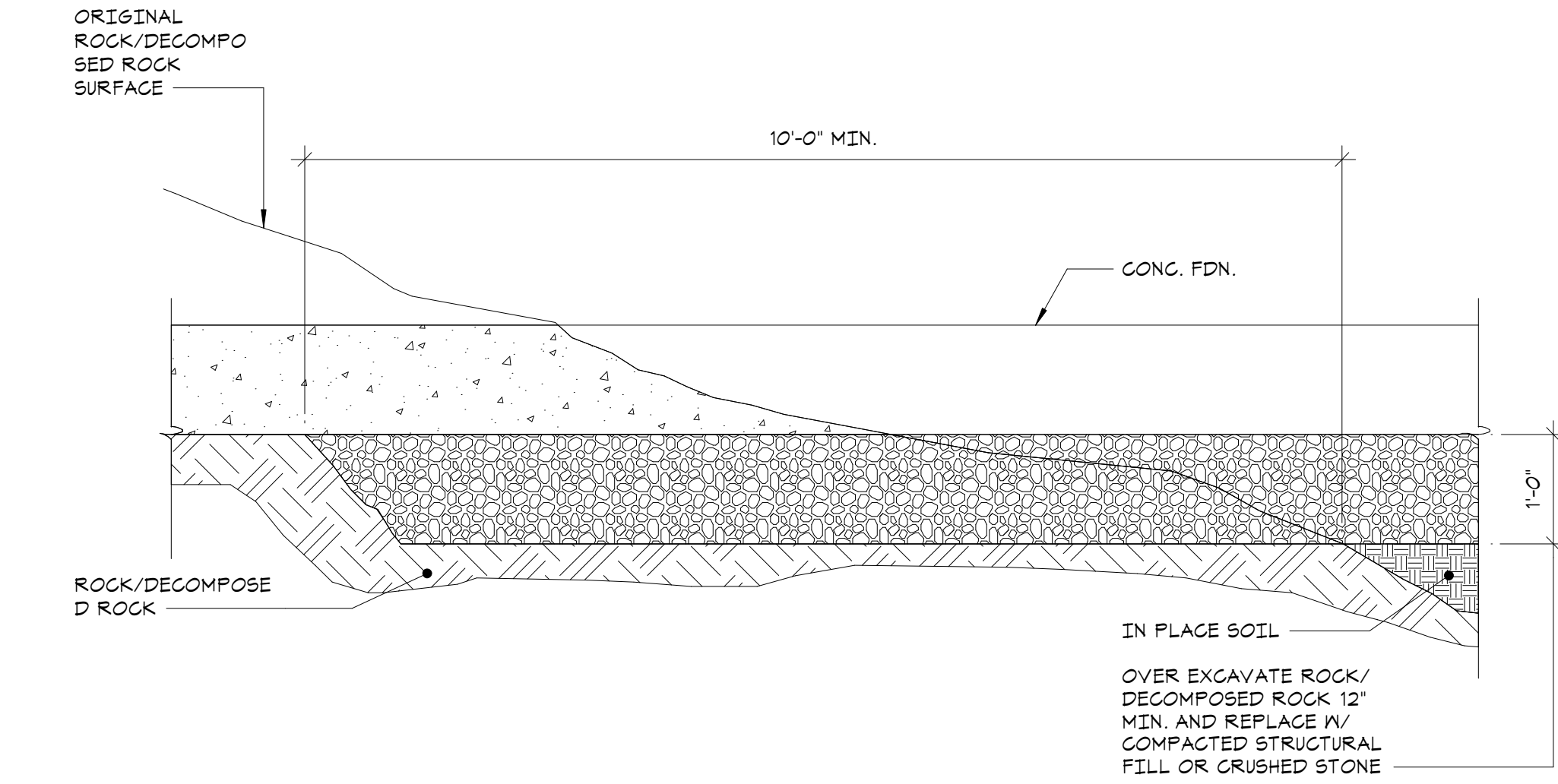


FOOTING SCHEDULE (ALLOWABLE BEARING PRESSURE: 4000 PSF)						
MARK	PLAN SIZE		THICKNESS	BOTTOM REINF.		REMARKS
	N-S	E-W		N-S	E-W	
F-3.0	3'-0"	3'-0"	1'-4"	(4)-#5	(4)-#5	
F-3.5	3'-6"	3'-6"	1'-4"	(4)-#5	(4)-#5	
F-4.0	4'-0"	4'-0"	1'-4"	(5)-#5	(5)-#5	
F-4.5	4'-6"	4'-6"	1'-4"	(6)-#5	(6)-#5	
F-5.0	5'-0"	5'-0"	1'-6"	(7)-#5	(7)-#5	
F-5.5	5'-6"	5'-6"	1'-6"	(7)-#5	(7)-#5	
F-6.0	6'-0"	6'-0"	1'-6"	(8)-#5	(8)-#5	
F-6.5	6'-6"	6'-6"	1'-6"	(9)-#5	(9)-#5	
F-7.0	7'-0"	7'-0"	2'-0"	(7)-#7	(7)-#7	
F-7.5	7'-6"	7'-6"	2'-0"	(7)-#7	(7)-#7	
F-8.0	8'-0"	8'-0"	2'-0"	(7)-#7	(7)-#7	
F-8.5	8'-6"	8'-6"	2'-0"	(8)-#7	(8)-#7	
F-9.0	9'-0"	9'-0"	2'-6"	(10)-#7	(10)-#7	
F-9.5	9'-6"	9'-6"	2'-6"	(11)-#7	(11)-#7	
F-10.0	10'-0"	10'-0"	2'-6"	(11)-#7	(11)-#7	
F-11.0	11'-0"	11'-0"	2'-6"	(12)-#7	(12)-#7	
F-12.0	12'-0"	12'-0"	3'-0"	(16)-#7	(16)-#7	
F-13.0	13'-0"	13'-0"	3'-0"	(17)-#7	(17)-#7	
F-14.0	14'-0"	14'-0"	3'-0"	(12)-#8	(12)-#8	

9 TYPICAL SLAB/WALL PENETRATION REINFORCING DETAIL



11 FDN. SUB-GRADE PREP. FOR ROCK/SOIL TRANSITION DETAIL



11 FDN. SUB-GRADE PREP. FOR ROCK/SOIL TRANSITION DETAIL

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No.	Date	Issue
5	07/12/2023	ISSUE FOR BID - PHASE 1
4	06/09/2023	ISSUE FOR PERMIT - PHASE 1
2	2023-03-31	PHASE 1 - 50% CDS
1	2020-06-01	DESIGN DEVELOPMENT

Sheet Title

FOUNDATION SCHEDULES AND TYPICAL DETAILS

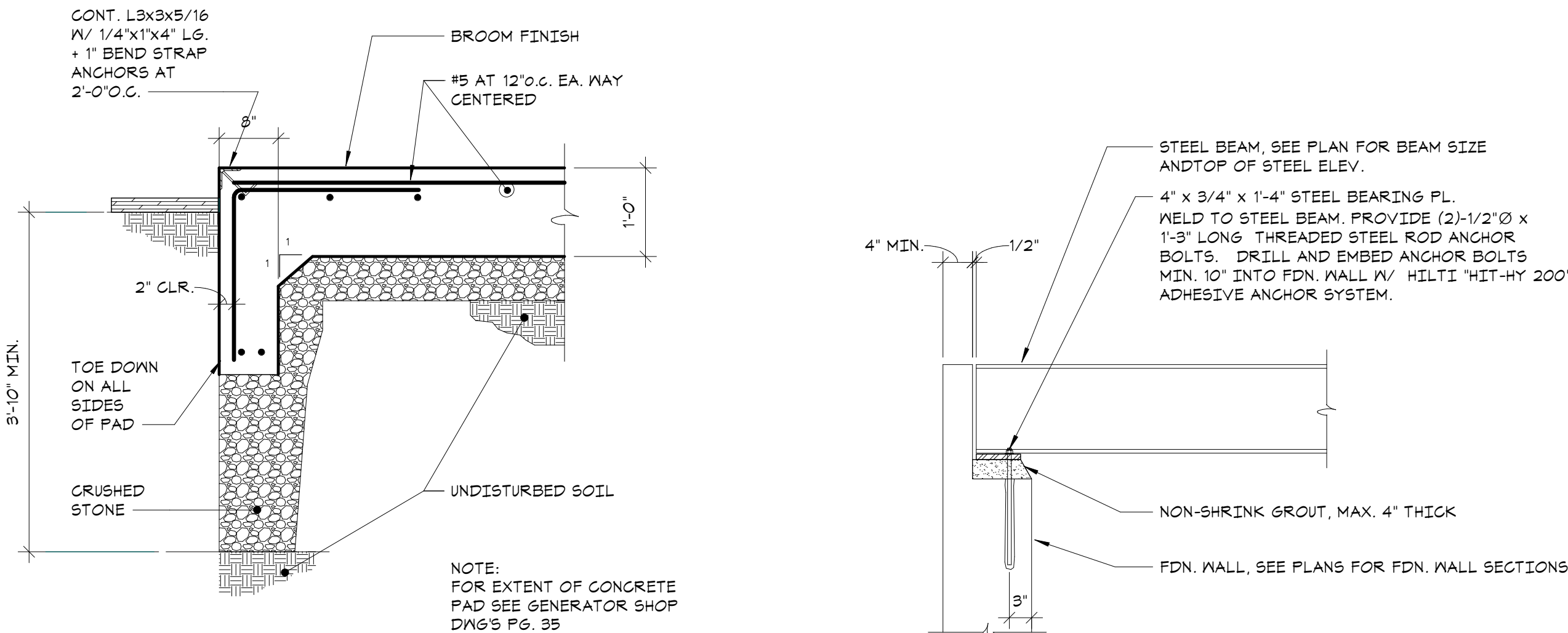
Job No.	20029.00	Date	2020-06-01
Scale	AS NOTED	Drawn / Checked	JM / EM
Sheet Number			

S-201

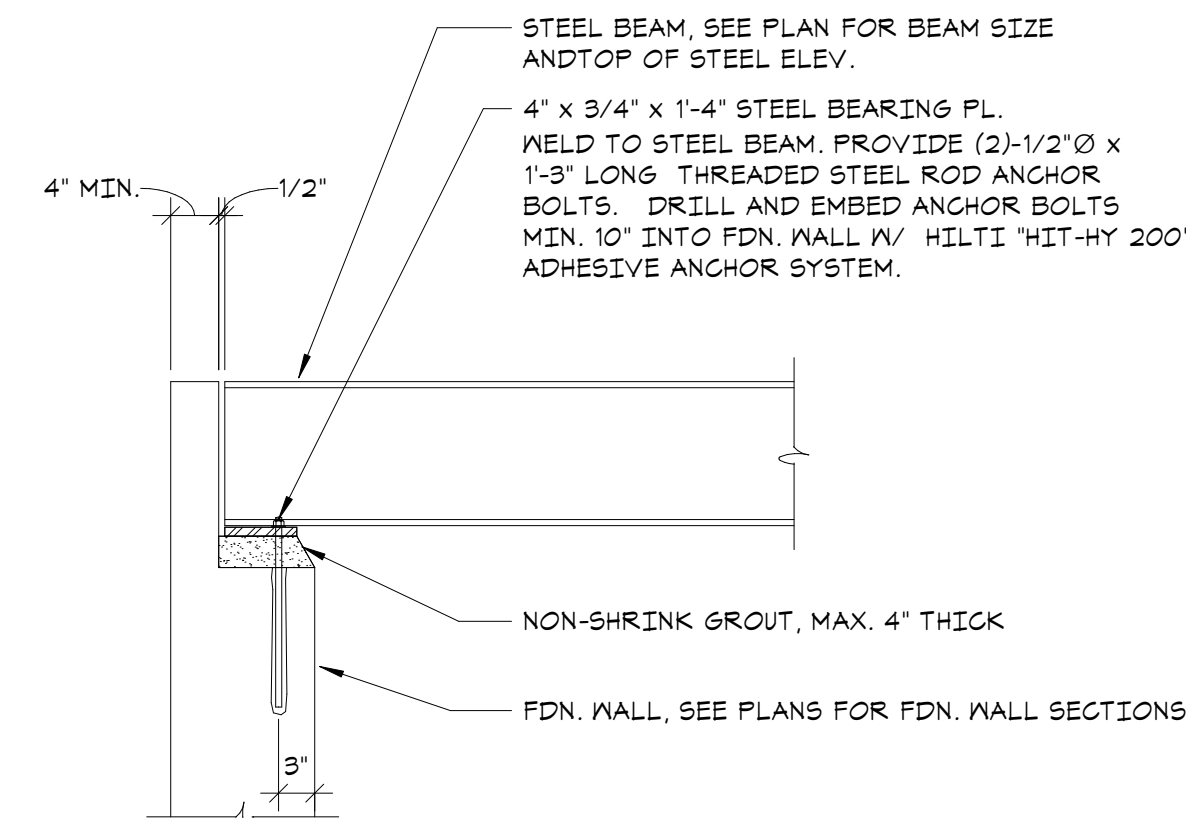
DOWEL SCHEDULE		
MALL DESCRIPTION	VERTICAL REINFORCEMENT	
FUTURE 8" CMU SHEAR WALLS, SHAFT WALLS, AND EXTERIOR WALLS	(A)	#5 AT 32" O.C.
	(B)	(3)-#5
FUTURE 8" CMU PARTITIONS	(A)	#4 AT 48" O.C.
	(B)	(1)-#4
NOTES: 1. PROVIDE REINFORCING DOWELS BETWEEN FOUNDATION AND FUTURE MASONRY WALL EQUAL TO SIZE AND SPACING OF MASONRY VERT. WALL REINFORCING (MAX. SPACING = 4'-0" O.C.). REINFORCING DOWELS SHALL EXTEND UP FROM FOUNDATION WALLS, WITH LENGTHS SUFFICIENT TO PROVIDE FOR LAP LENGTHS OF 48 BAR DIAMETERS.		

KEYNOTES	
(A)	DOWELS FOR TYPICAL VERTICAL REINFORCING
(B)	DOWELS FOR VERT. FULL HEIGHT REINF. AT WALL ENDS, CORNERS, AND EA. SIDE OF CJS

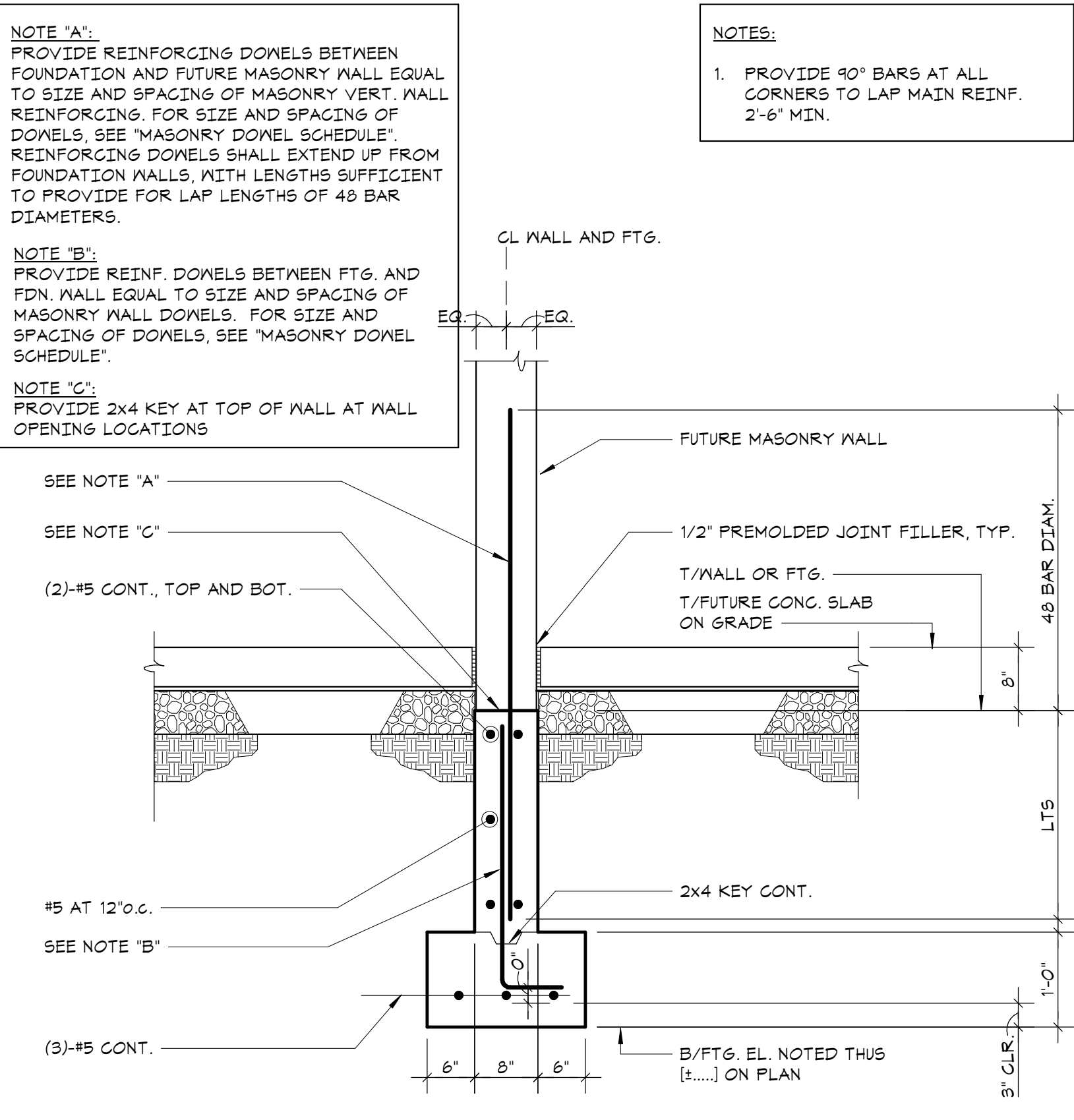
1 MASONRY DOWEL SCHEDULE
NO SCALE



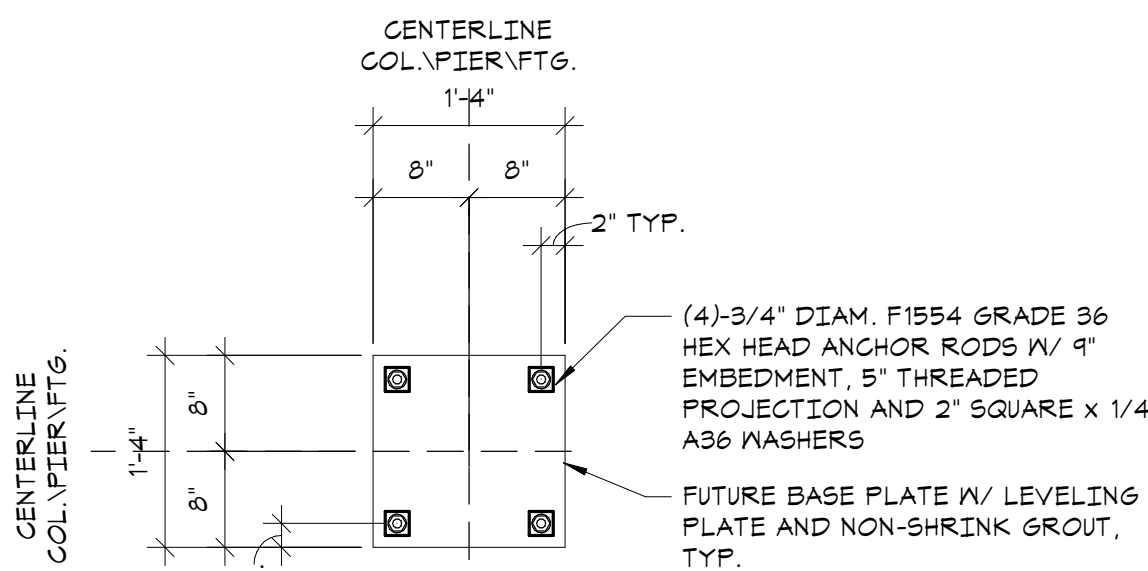
2 EQUIPMENT PAD DETAIL
NO SCALE



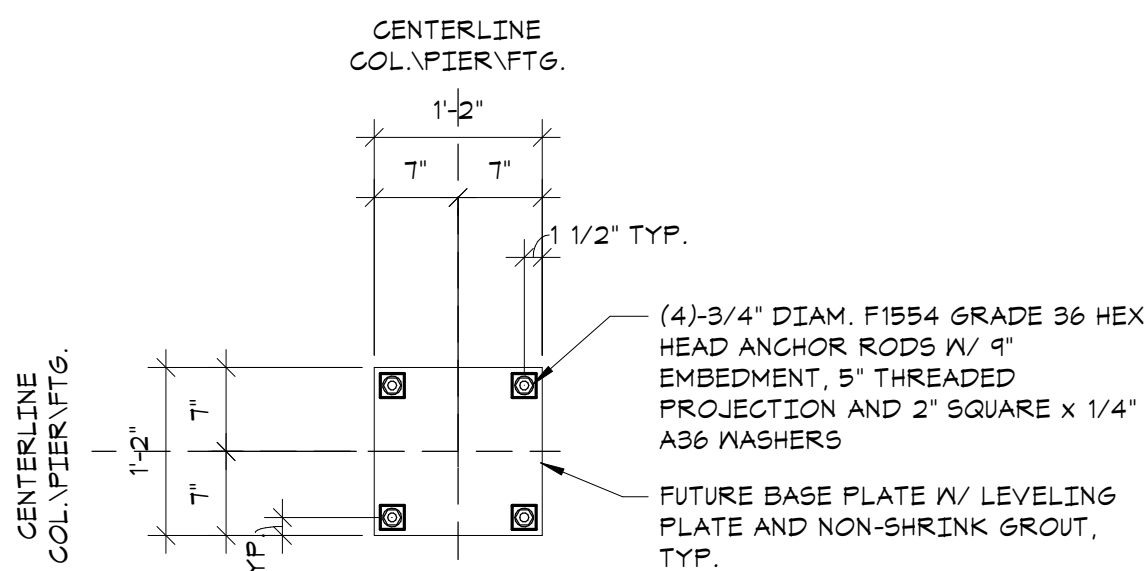
3 TYPICAL BEAM BEARING ON CONG. FDN WALL
NO SCALE



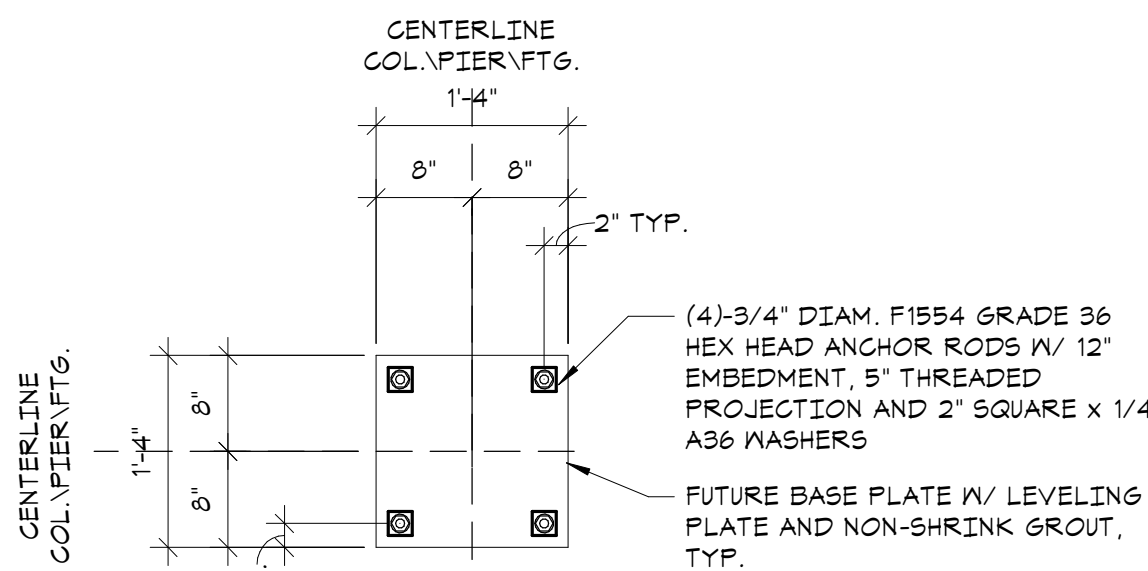
4 TYPICAL FOUNDATION DETAIL FOR FUTURE MASONRY WALLS
NO SCALE



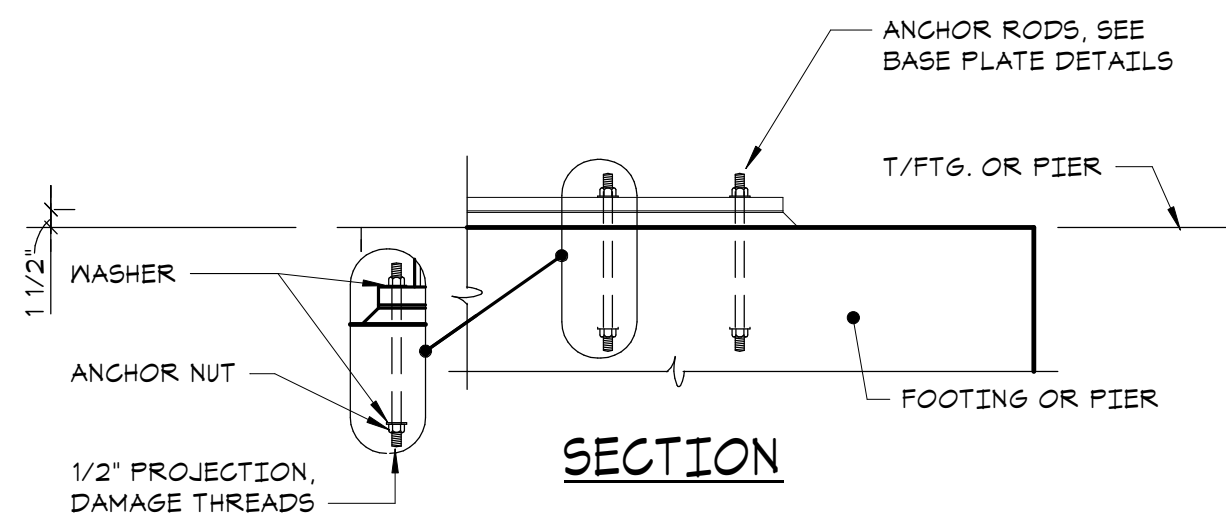
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BASE PLATE DETAIL



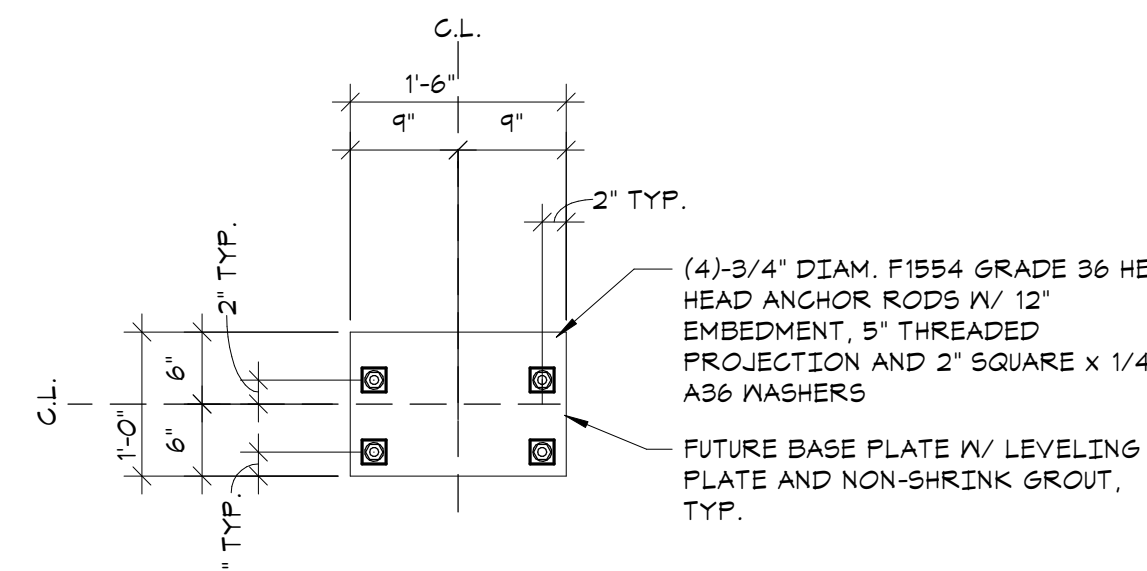
AR-2
BASE PLATE DETAIL



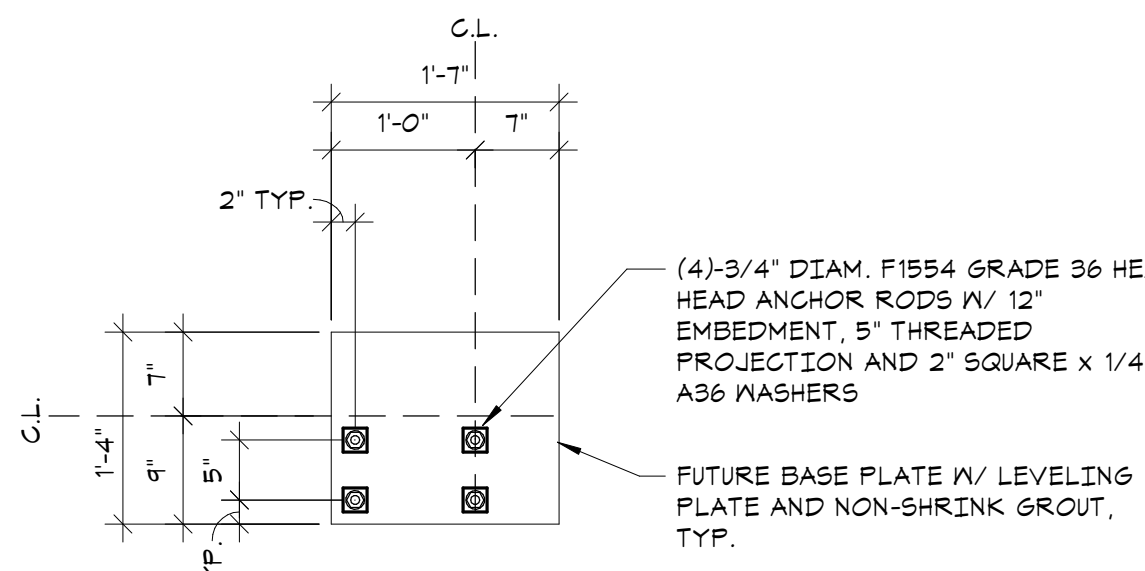
AR-3
BASE PLATE DETAIL



5 TYPICAL BASE PLATE ANCHOR DETAILS
NO SCALE



AR-4
BASE PLATE DETAIL



AR-5
BASE PLATE DETAIL

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5	07/12/2023	ISSUE FOR BID - PHASE 1
4	06/09/2023	ISSUE FOR PERMIT - PHASE 1
No.	Date	Issue

Sheet Title

FOUNDATION SCHEDULES AND TYPICAL DETAILS

Job No.	20029.00	Date	04/20/23
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S-202

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2	2023-03-31	PHASE 1 - 50% CD'S

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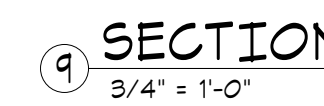
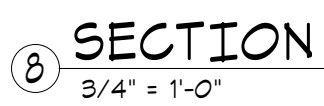
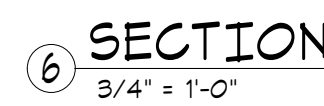
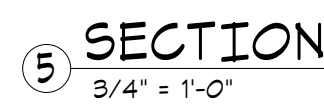
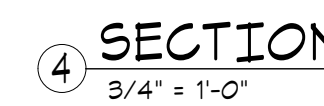
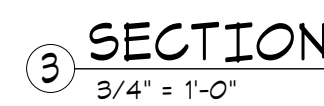
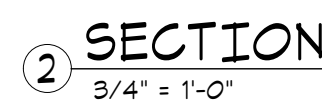
FOUNDATION SECTIONS

Job No. 20029.00	Date 2020-06-01
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Sheet Number	
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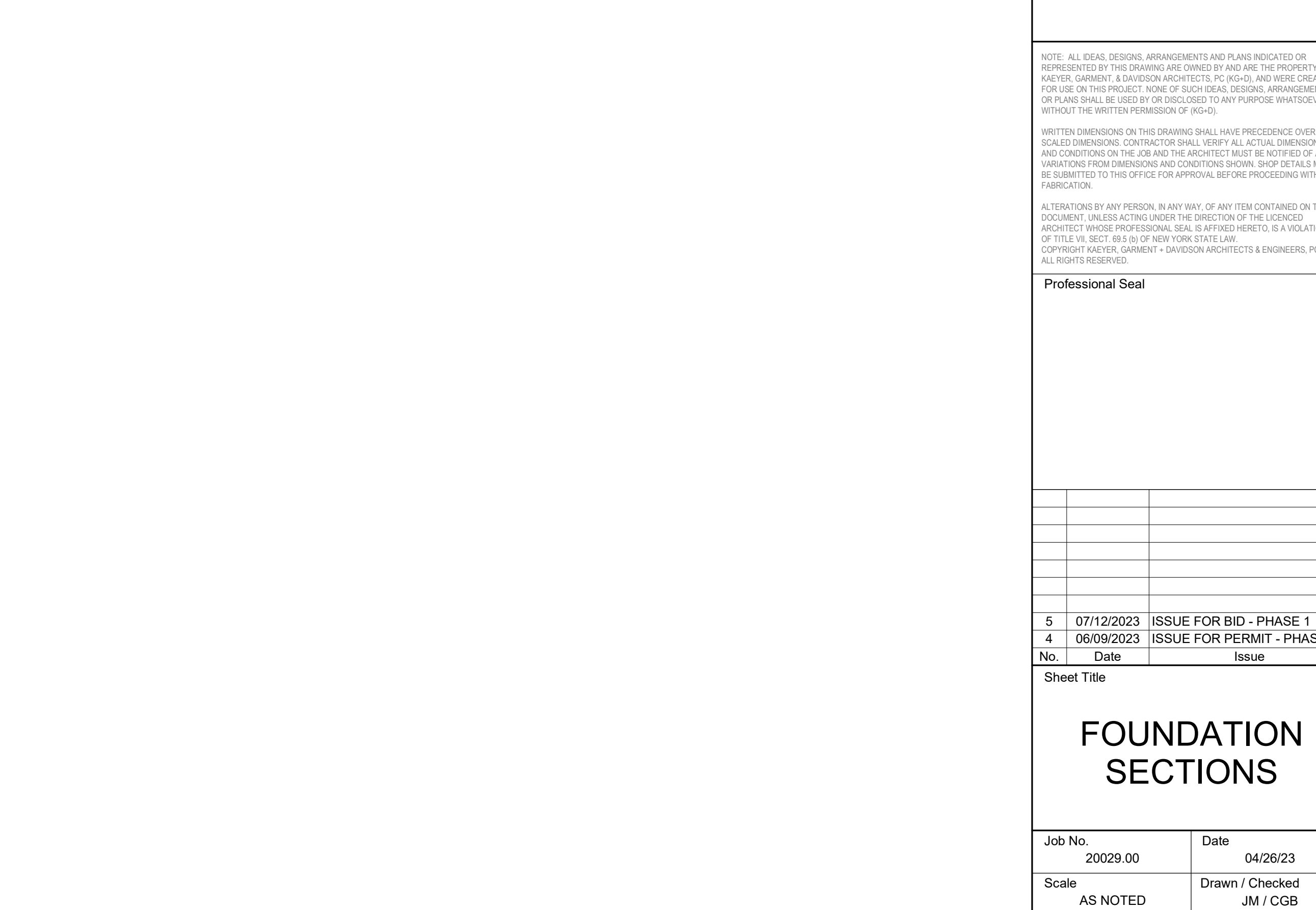
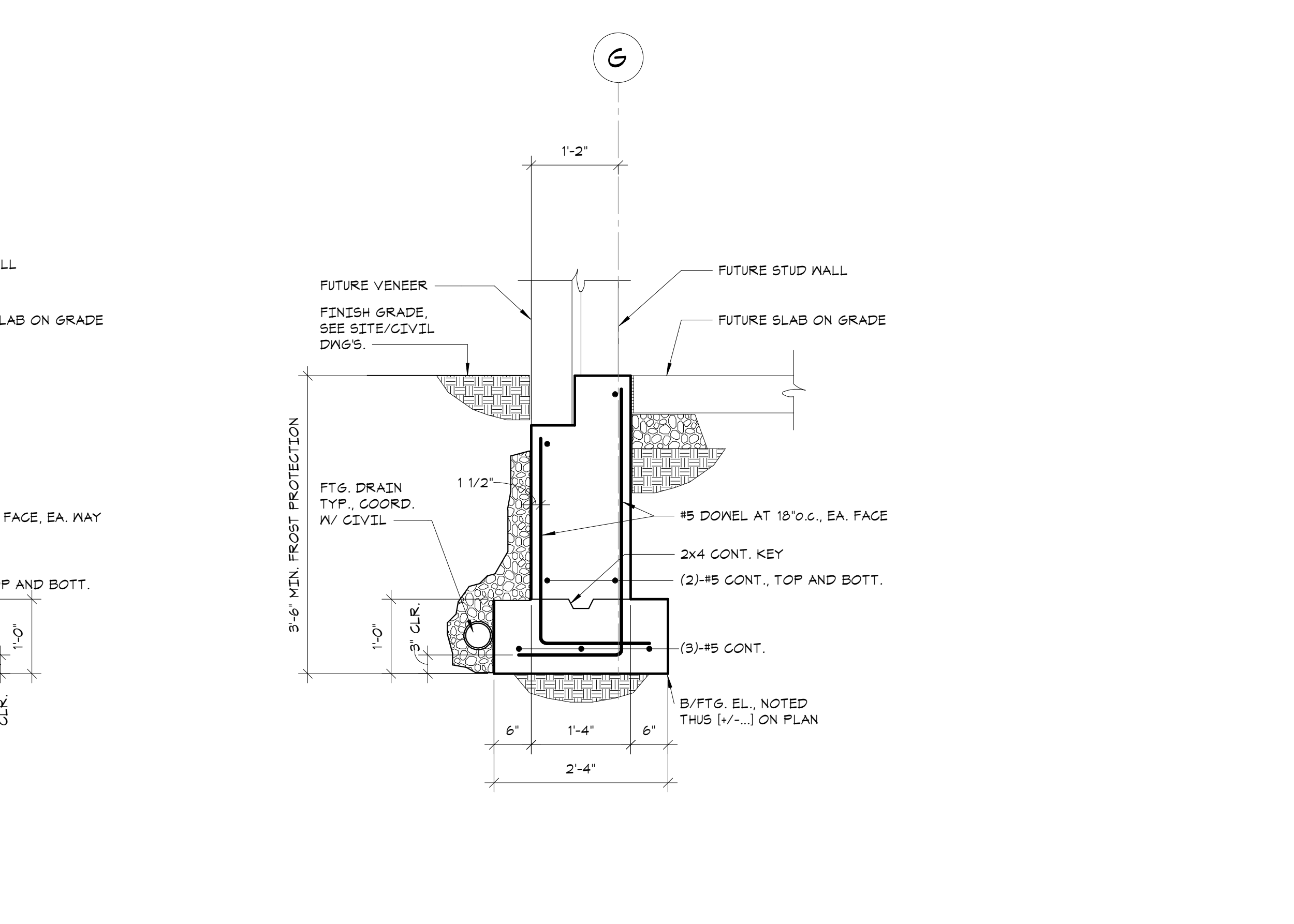
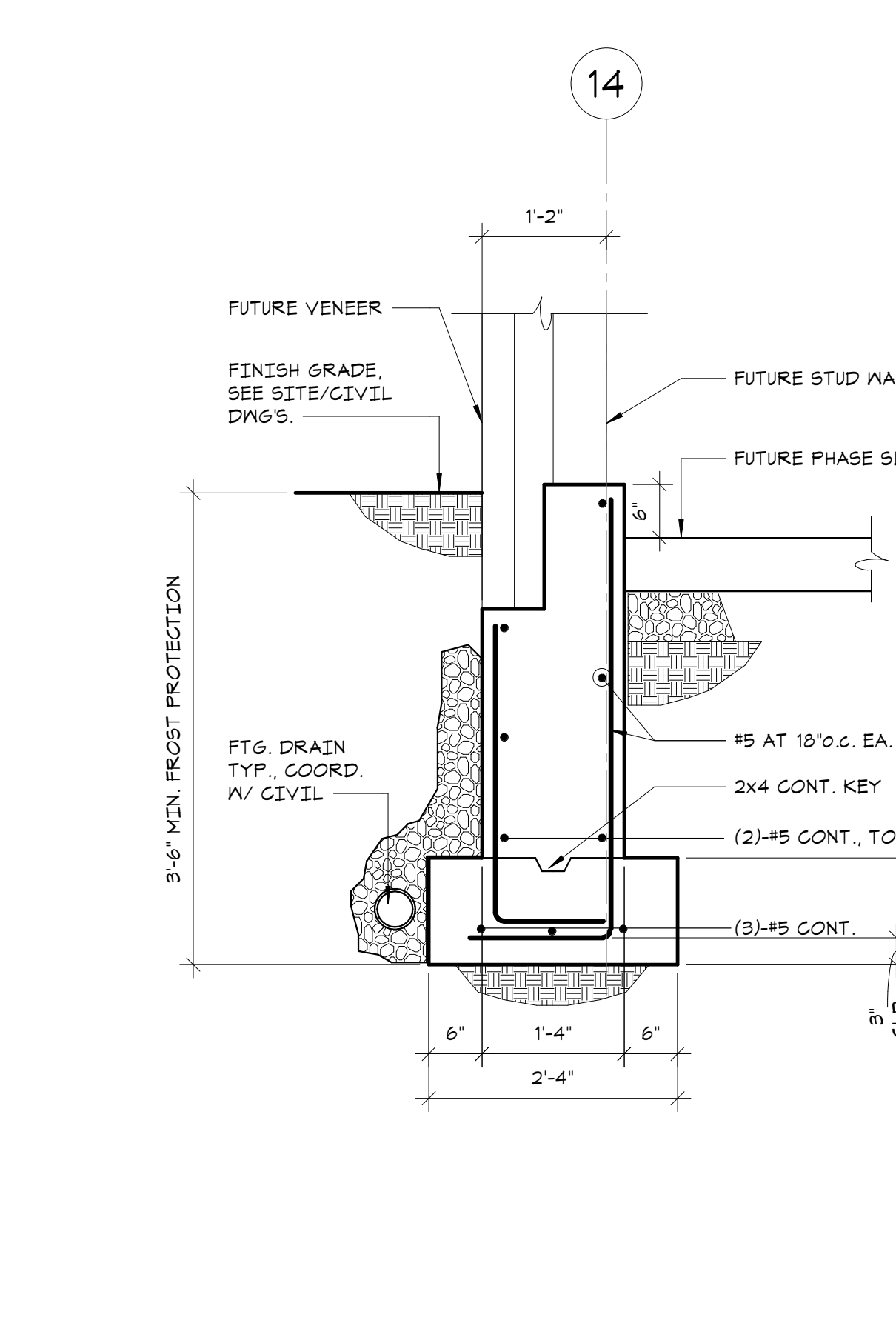
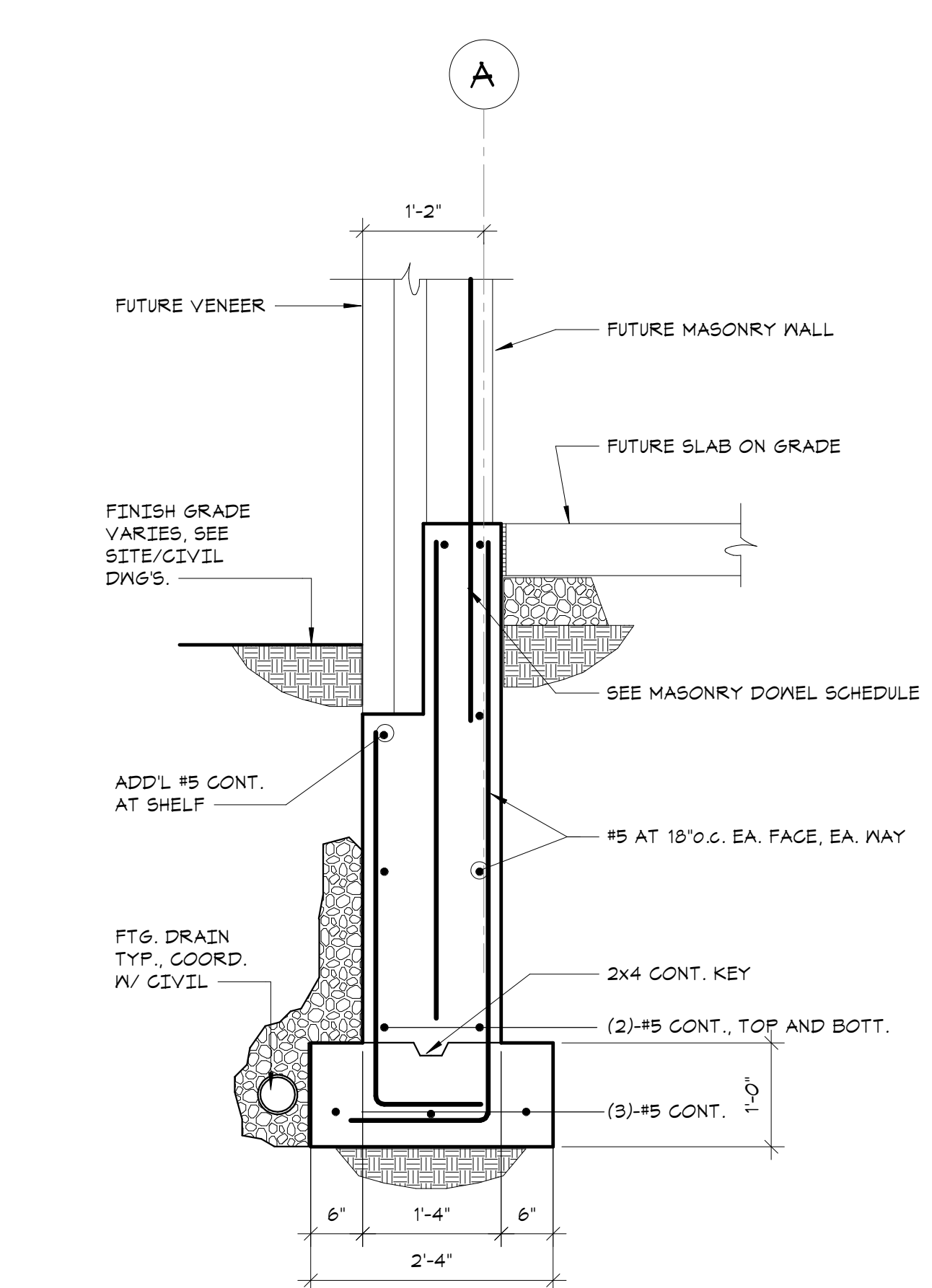
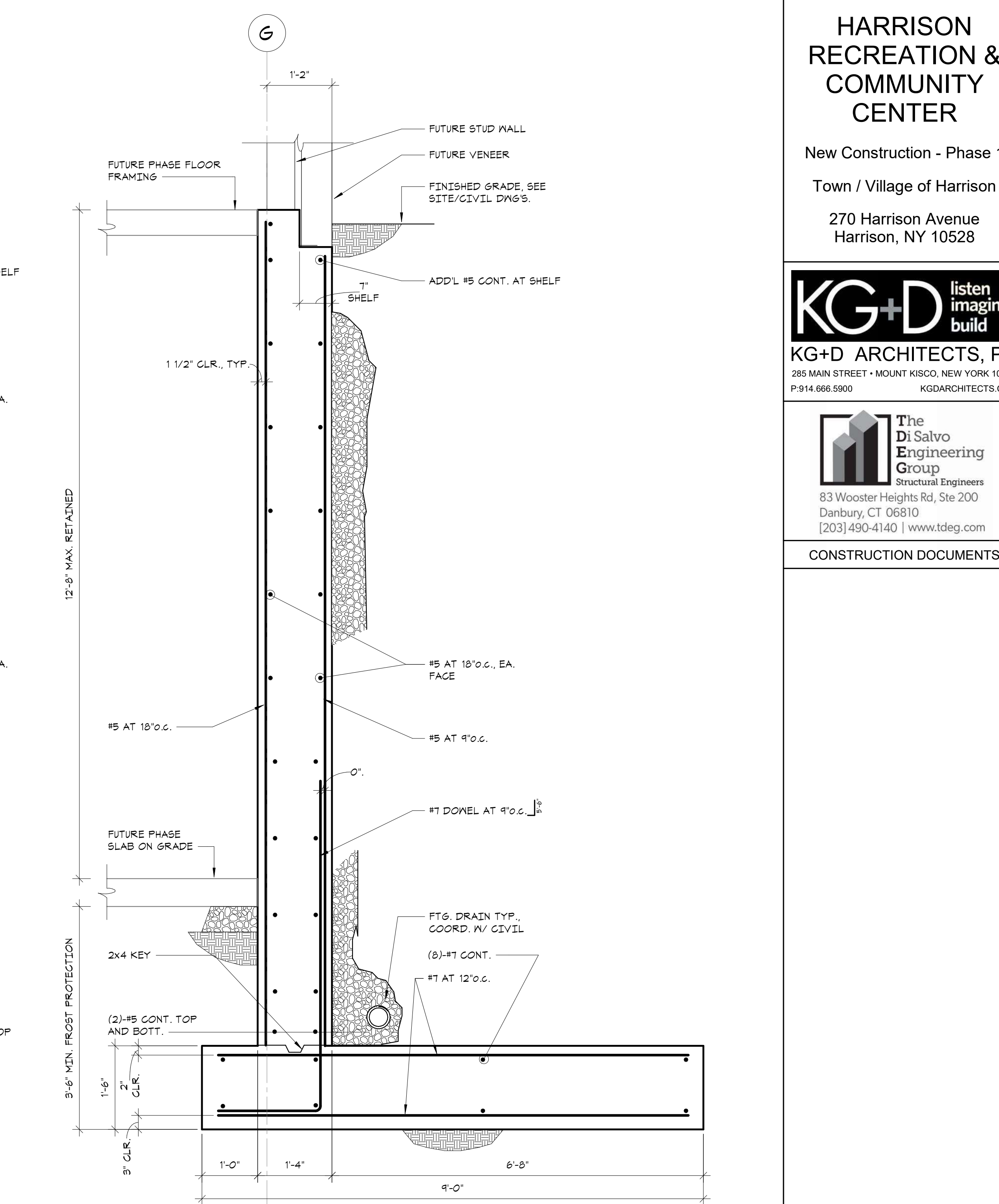
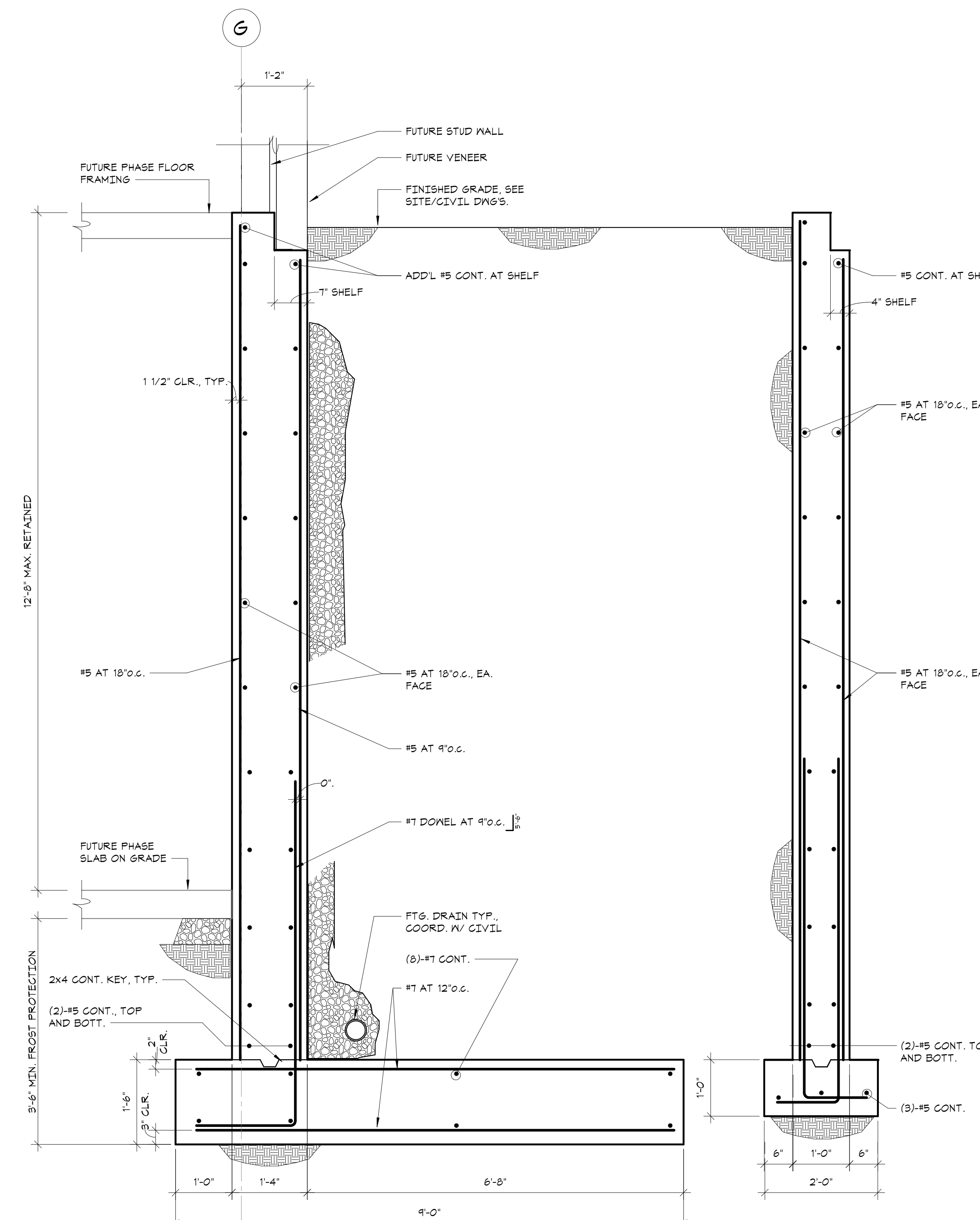
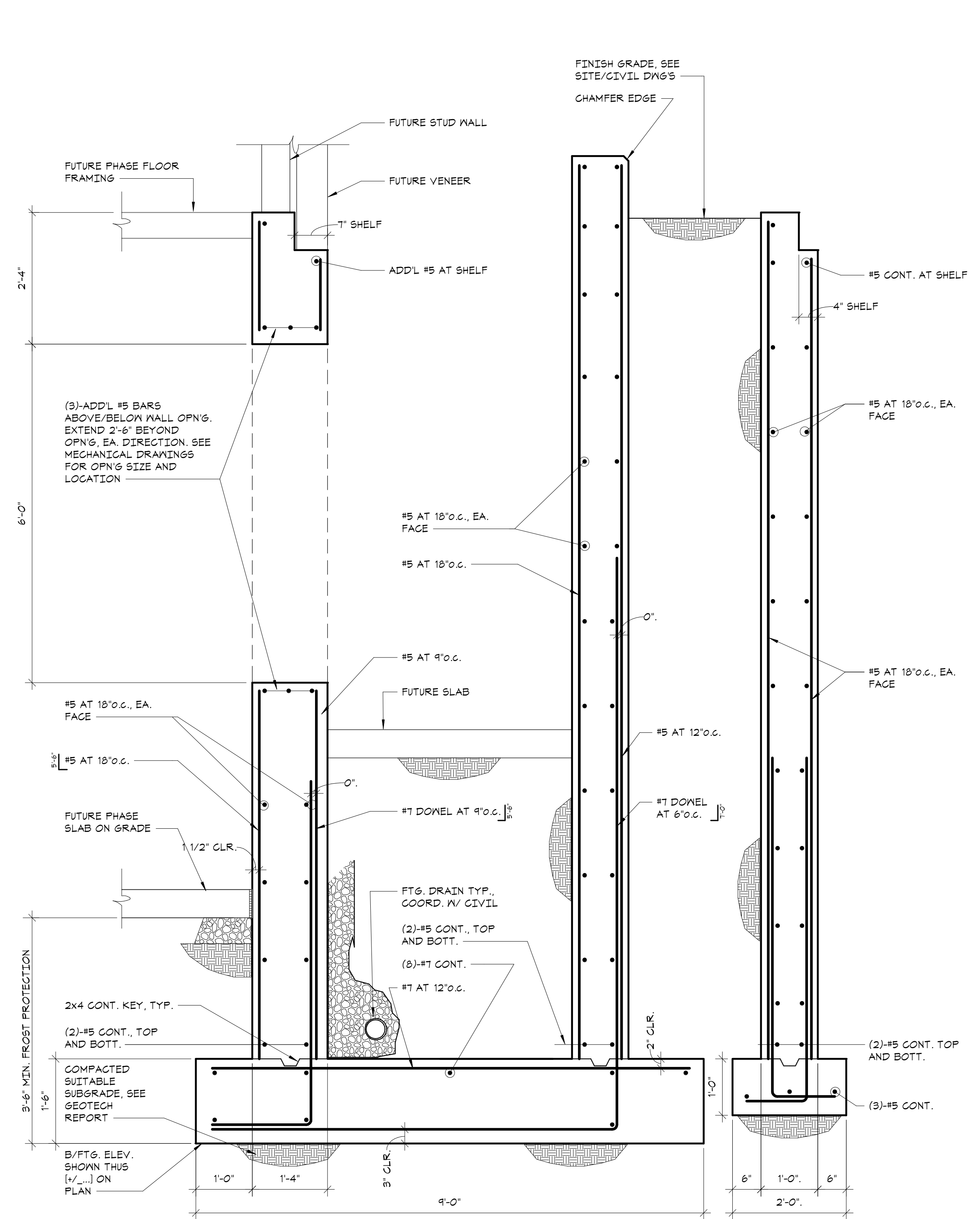
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FOUNDATION
SECTIONS

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3 07/12/2023 ISSUE FOR BID - PHASE 1

No. Date Issue

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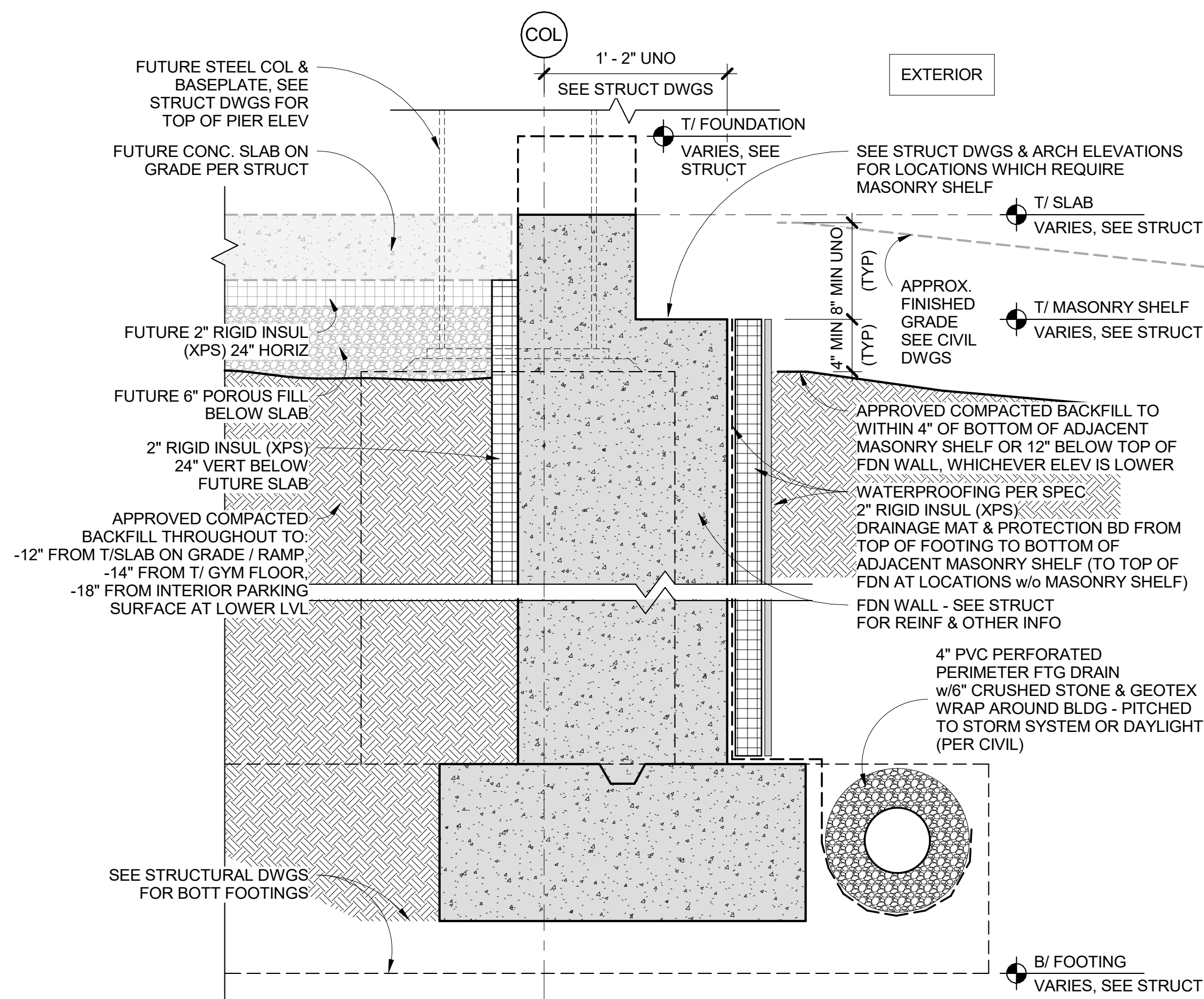
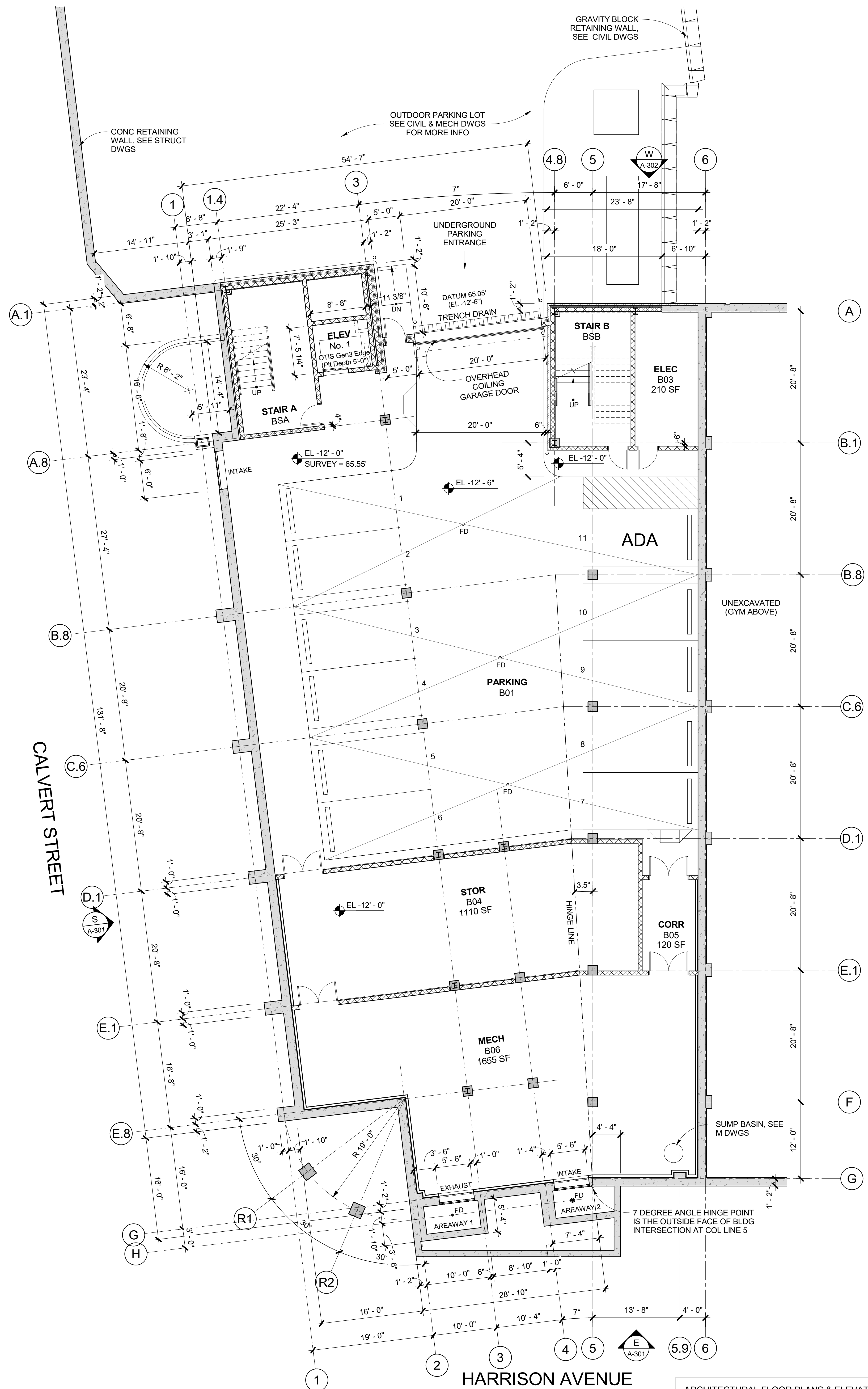
LOWER LEVEL
PLAN &
FOUNDATION
DETAIL

Job No. 2020-1005 Date 07/12/2023

Scale AS NOTED Drawn / Checked

Sheet Number

A-200



THIS DETAIL IS INTENDED TO SHOW TYPICAL COMPONENTS REQUIRED AT PERIMETER FOUNDATION WALLS. GC SHALL PROVIDE FOUNDATION DRAINAGE, WATERPROOFING, THERMAL CONTROL, PROTECTION BOARD & BACKFILL TO THE EXTENT AS SHOWN & NOTED. ELEVATIONS SHOWN VARY GREATLY THROUGHOUT THE FOUNDATIONS. SEE STRUCTURAL DRAWINGS FOR ACTUAL FOUNDATION WALL CONFIGURATIONS.

1 TYPICAL FOUNDATION SECTION DETAIL
1 1/2" = 1'-0"

L LOWER LEVEL & PARKING PLAN
1/8" = 1'-0"

ARCHITECTURAL FLOOR PLANS & ELEVATIONS ARE INTENDED TO DEMONSTRATE DESIGN INTENT OF SUBSEQUENT PHASES & ARE INCLUDED WITH PHASE 1 DOCUMENTS SOLELY FOR CONVENIENCE OF REFERENCE.

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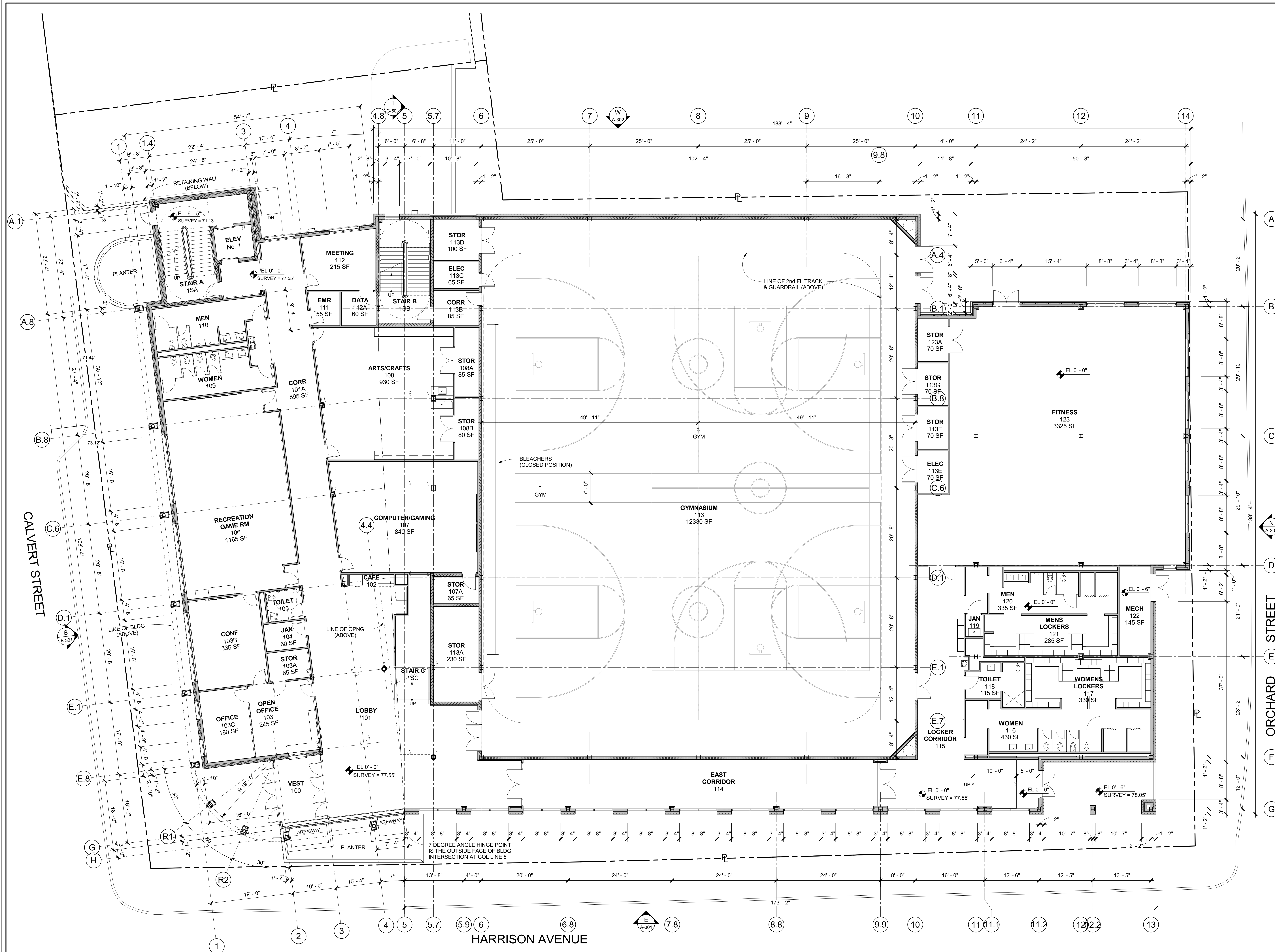
3	07/12/2023	ISSUE FOR BID - PHASE 1
No.	Date	Issue

Sheet Title

1st FLOOR PLAN
For Reference Only

Job No. 2020-1005	Date 07/12/2023
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Sheet Number	

A-201



1 1st FLOOR PLAN
1/8" = 1'-0"

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ARE INTENDED TO DEMONSTRATE DESIGN
INTENT OF SUBSEQUENT PHASES & ARE
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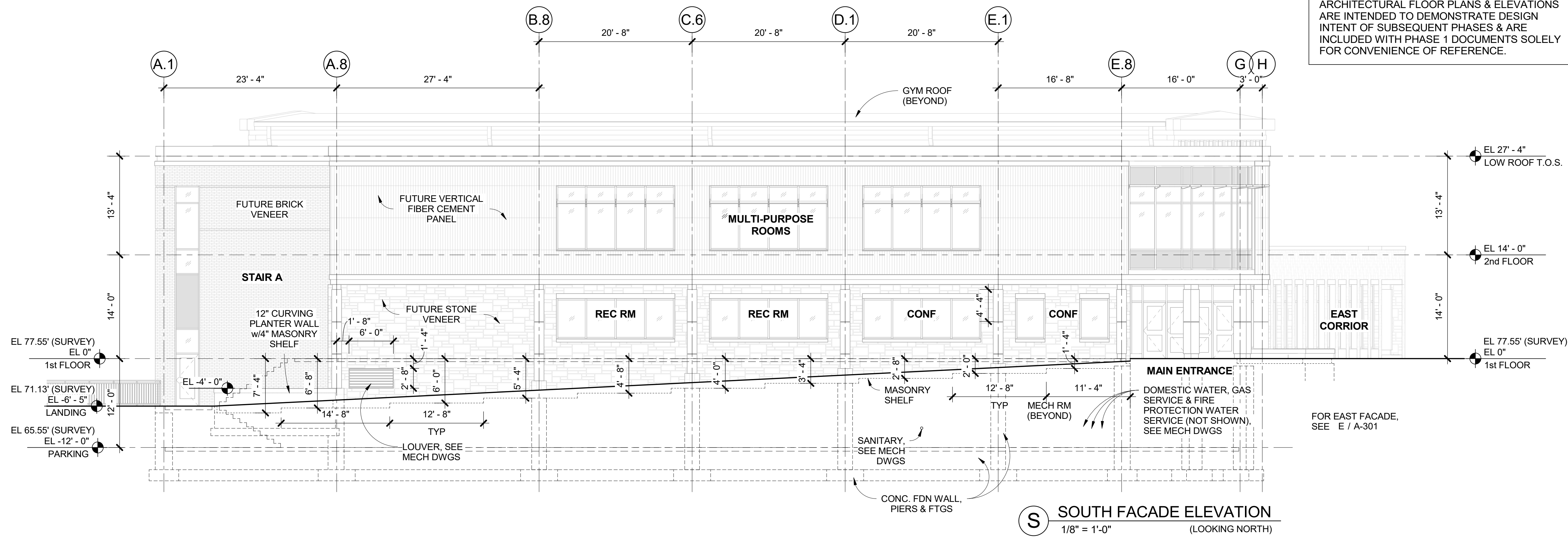
EXTERIOR
ELEVATIONS - For
Reference Only

Job No. 2020-1005 Date 07/12/2023

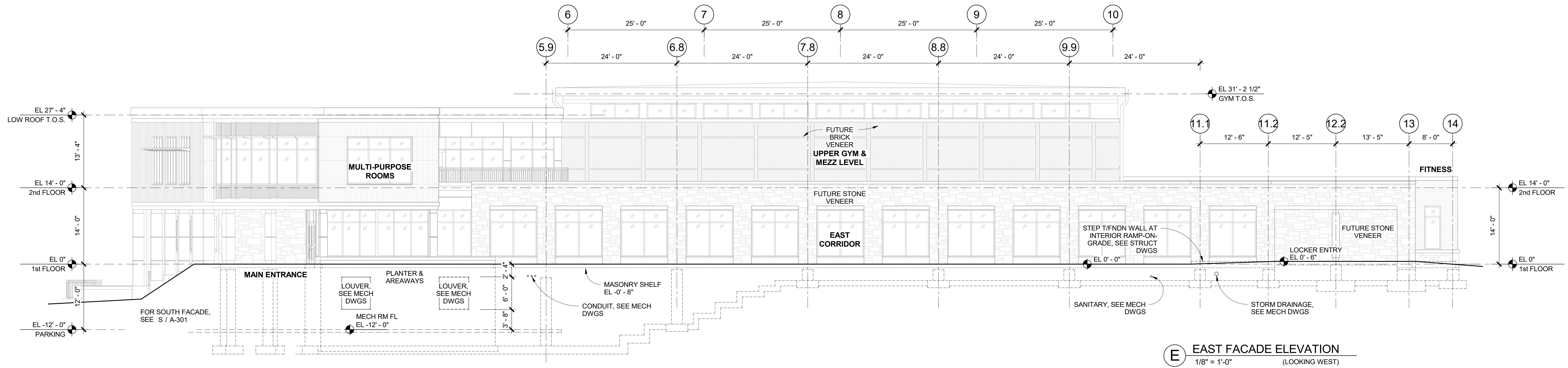
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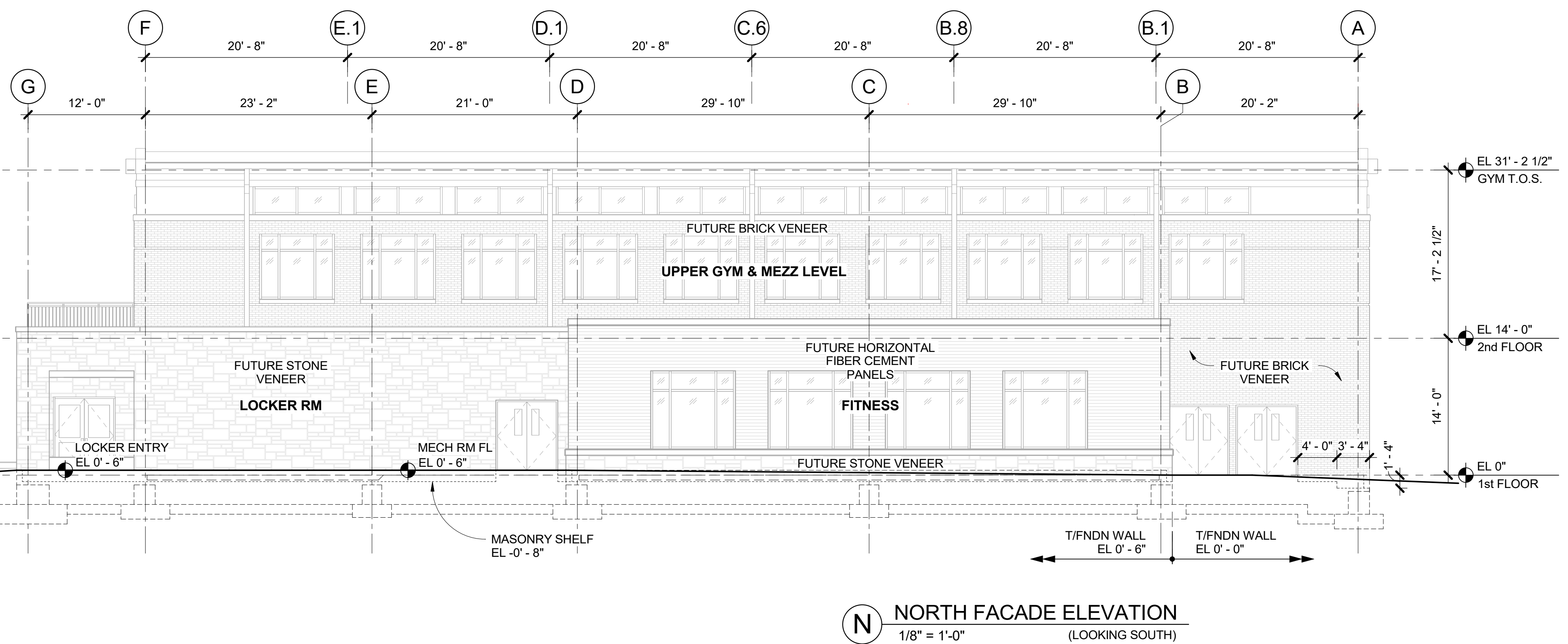
A-301



S SOUTH FACADE ELEVATION
1/8" = 1'-0"
(LOOKING NORTH)



E EAST FACADE ELEVATION
1/8" = 1'-0"
(LOOKING WEST)



N NORTH FACADE ELEVATION
1/8" = 1'-0"
(LOOKING SOUTH)

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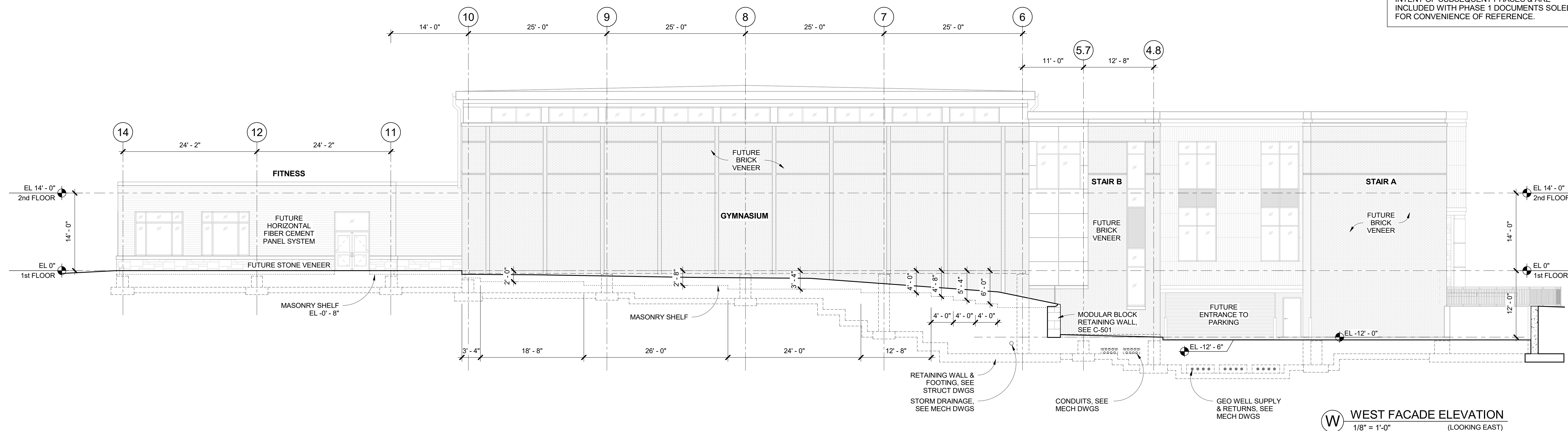
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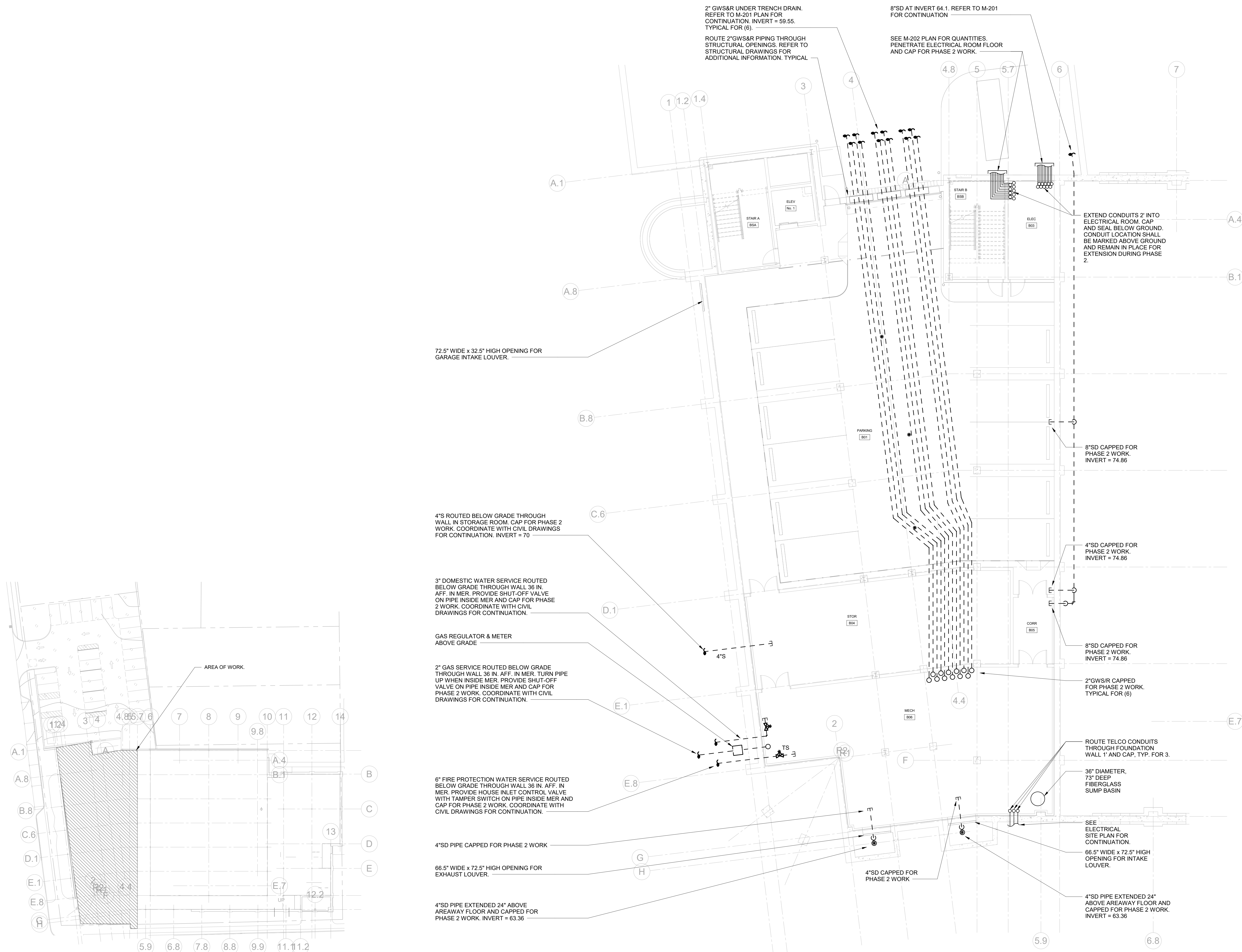
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No. Date Issue

Sheet Title

PARKING FLOOR
PLAN PHASE 1

Job No. NKGD0207.00 Date 06/29/2023
Scale AS NOTED Drawn / Checked HM / RG
Sheet Number VB / ML / DS

M-101



2
SCALE: 1/32" = 1'-0"



1
PARKING FLOOR PLAN PHASE 1
SCALE: 1/8" = 1'-0"

NOTES:
1.) ALL ELECTRICAL CONDUITS ARE ROUTED UNDERGROUND. UON.
2.) ALL ELECTRICAL CONDUITS SHALL BE PROVIDED WITH DRAGLINES.

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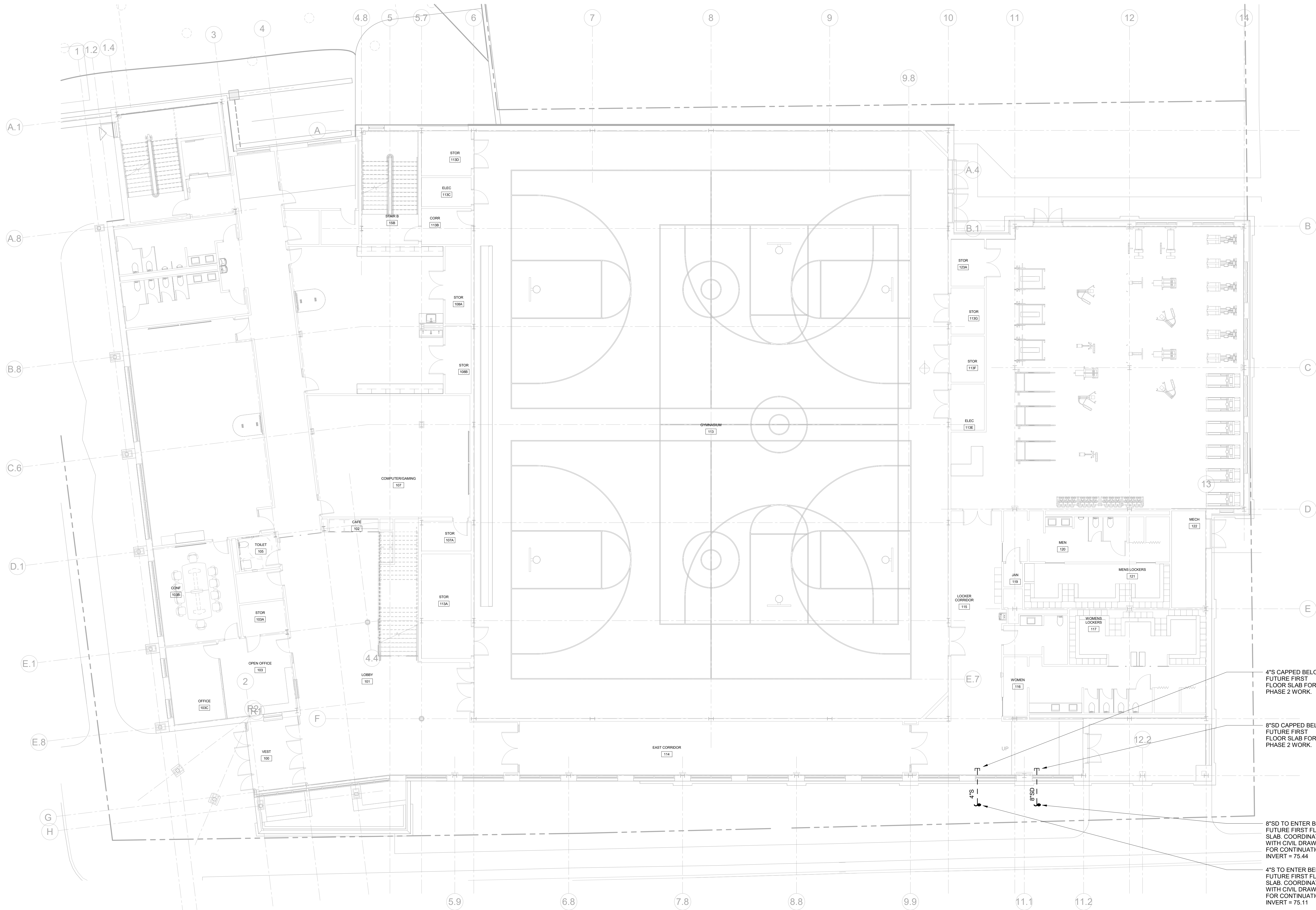
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No.	Date	Issue
3	07/12/2023	ISSUE FOR BID - PHASE 1
Sheet Title		

FIRST FLOOR
PLAN PHASE 1

Job No.	Date
KG0207.00	06/29/2023
Scale	Drawn / Checked
AS NOTED	HJM / RGS
Sheet Number	VB / ML / DS

M-102



1 FIRST FLOOR PLAN PHASE 1
SCALE: 1/8" = 1'-0"

- NOTES:
1.) ALL ELECTRICAL CONDUITS ARE ROUTED UNDERGROUND, UON.
2.) ALL ELECTRICAL CONDUITS SHALL BE PROVIDED WITH DRAGLINES.

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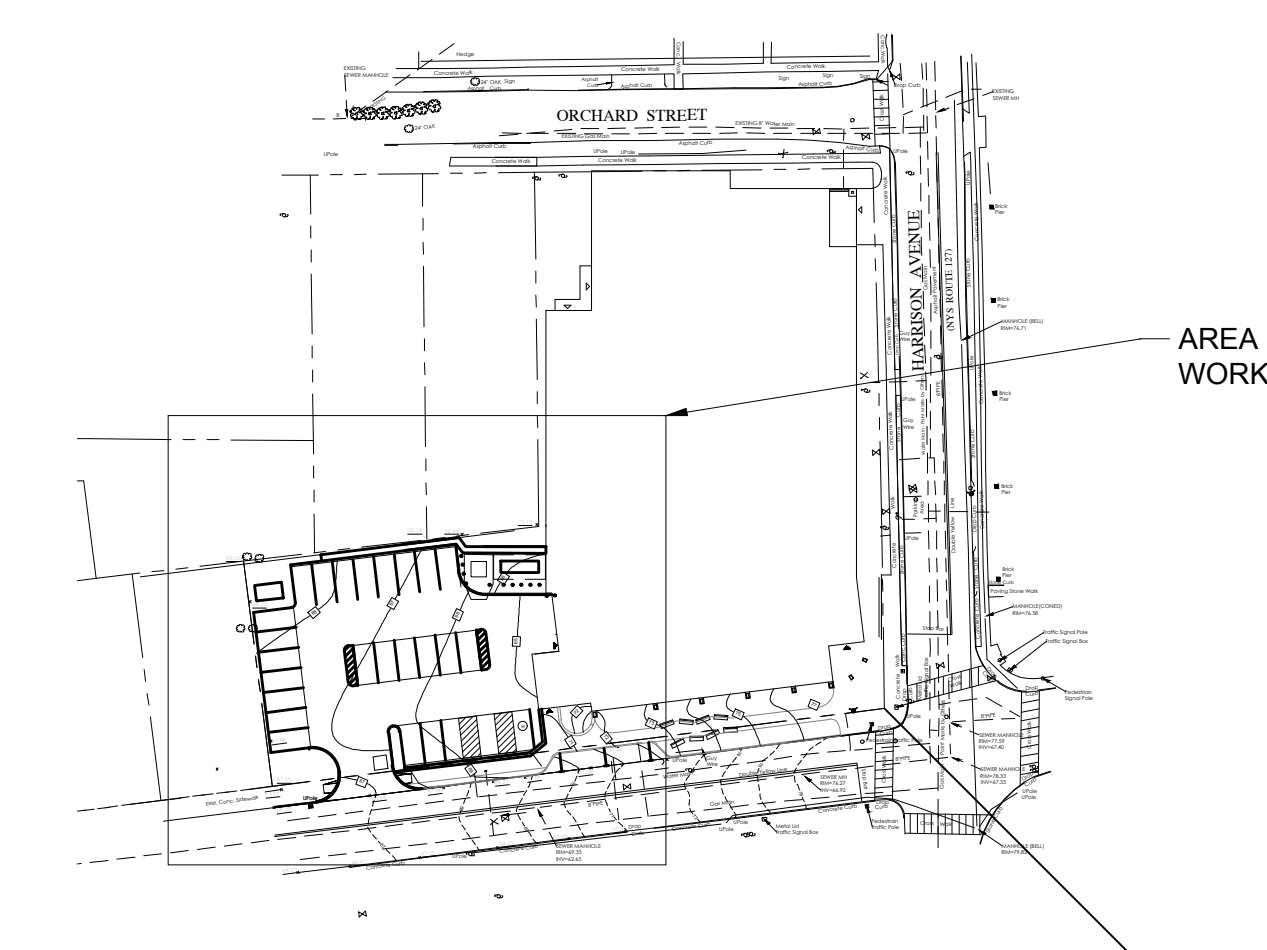
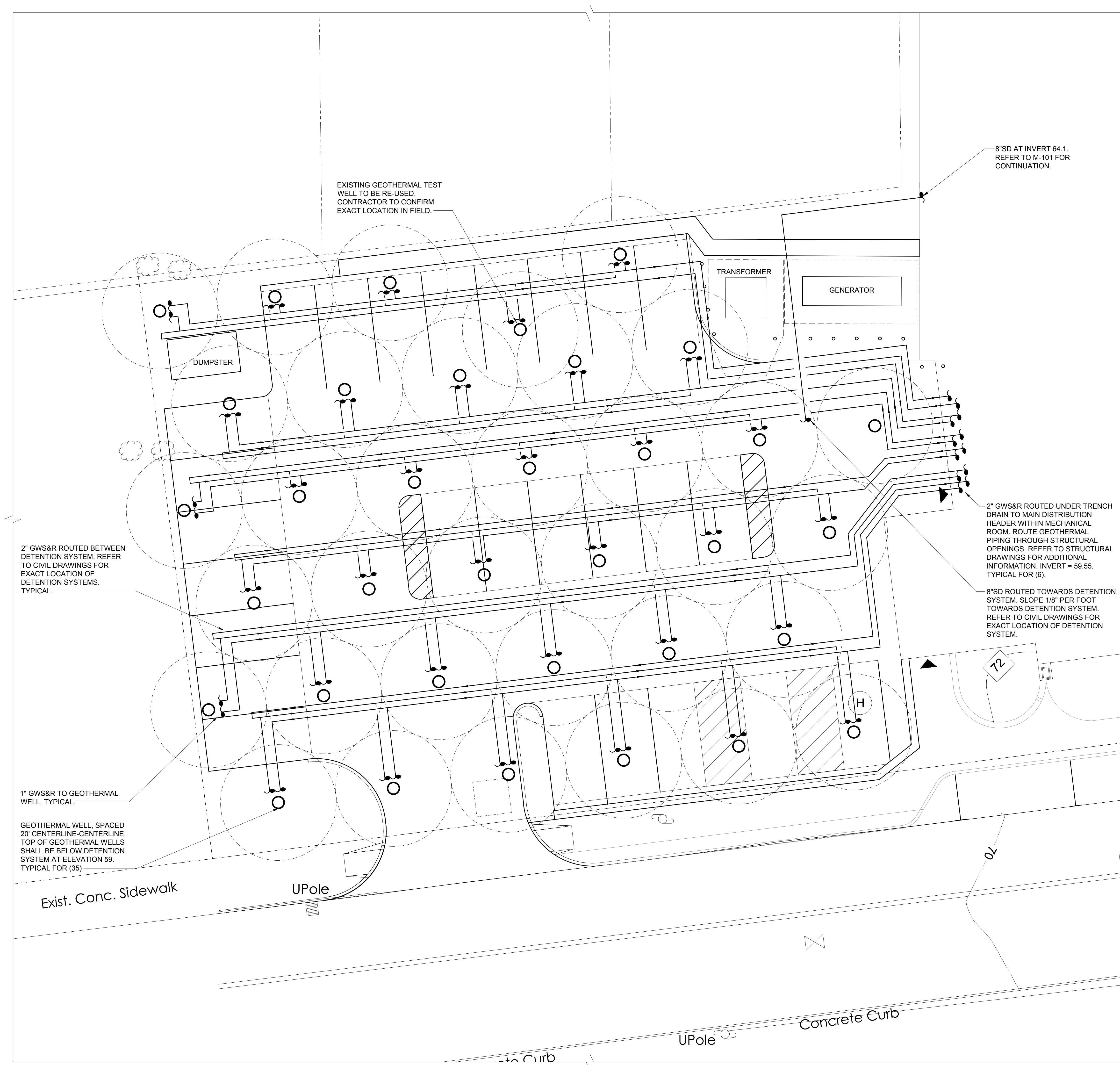
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3	07/12/2023	ISSUE FOR BID - PHASE 1
No.	Date	Issue

MECHANICAL
SITE PLAN

Job No. NKGD0207.00	Date 06/29/2023
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M-201

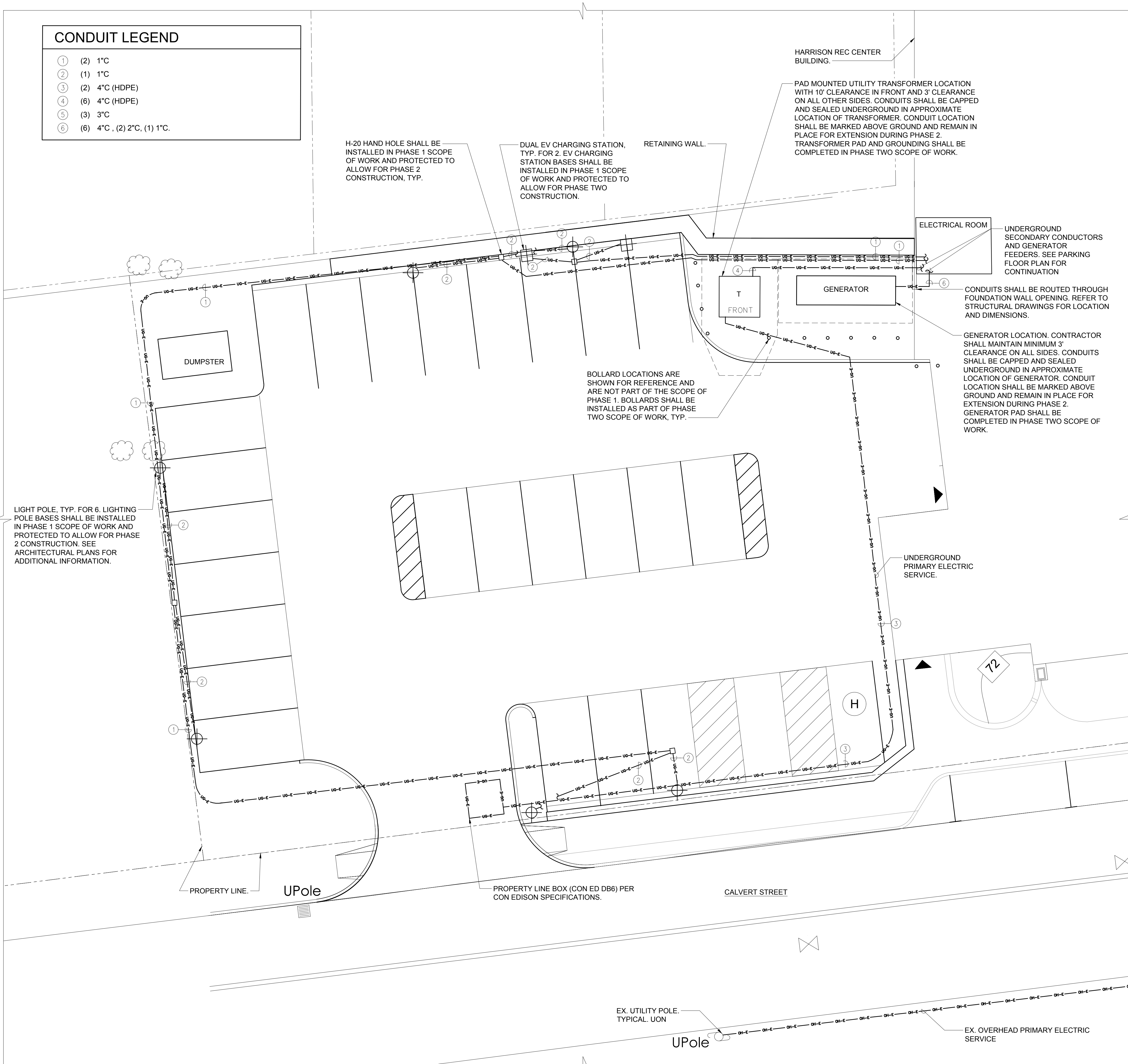


 2 MECHANICAL SITE PLAN
SCALE: 1/8" = 1'-0"

- NOTES:
1.) ALL GEOTHERMAL PIPING SHALL BE ROUTED BETWEEN DETENTION SYSTEM.
2.) ALL GEOTHERMAL WELLS SHALL BE UNDERNEATH DETENTION SYSTEM. TOP OF ALL GEOTHERMAL WELLS SHALL BE AT ELEVATION 59.

 1 **KEY PLAN**
SCALE: NONE

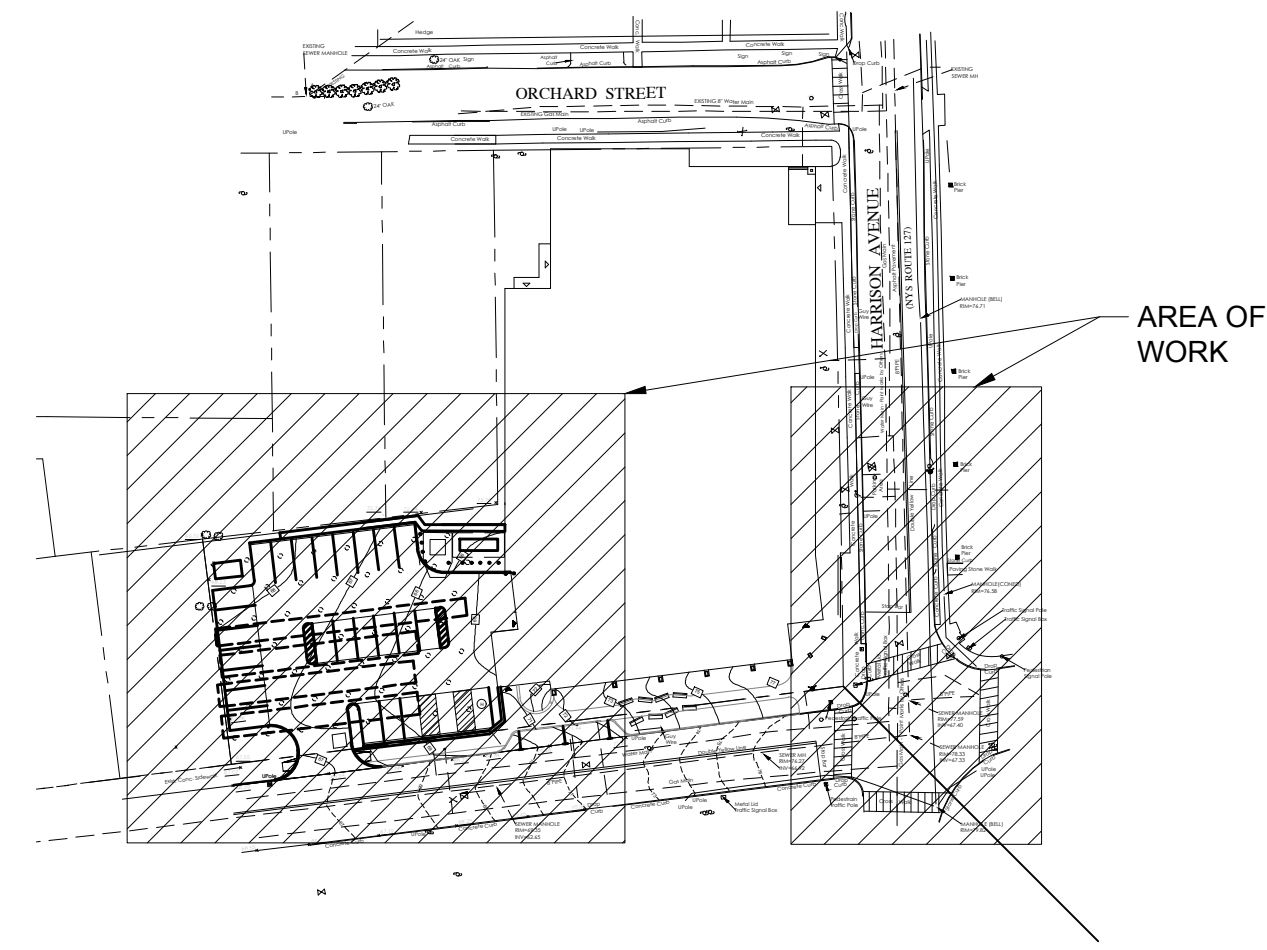
CONDUIT LEGEND	
①	(2) 1"C
②	(1) 1"C
③	(2) 4"C (HDPE)
④	(6) 4"C (HDPE)
⑤	(3) 3"C
⑥	(6) 4"C, (2) 2"C, (1) 1"C.



3 ELECTRICAL SITE PLAN
SCALE: 1/8" = 1'-0"

- NOTES:
1.) ALL ELECTRICAL CONDUITS ARE ROUTED UNDERGROUND, UON.
2.) ALL ELECTRICAL CONDUITS SHALL BE PROVIDED WITH DRAGLINES.
3.) ALL CONDUITS SHALL BE SCHEDULE 40 PVC, UON.

2 ELECTRICAL SITE PLAN
SCALE: 1/8" = 1'-0"



1 KEY PLAN
SCALE: NONE

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ELECTRICAL SITE PLAN

Job No. NKGD0207.00 Date 06/29/2023

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M-202

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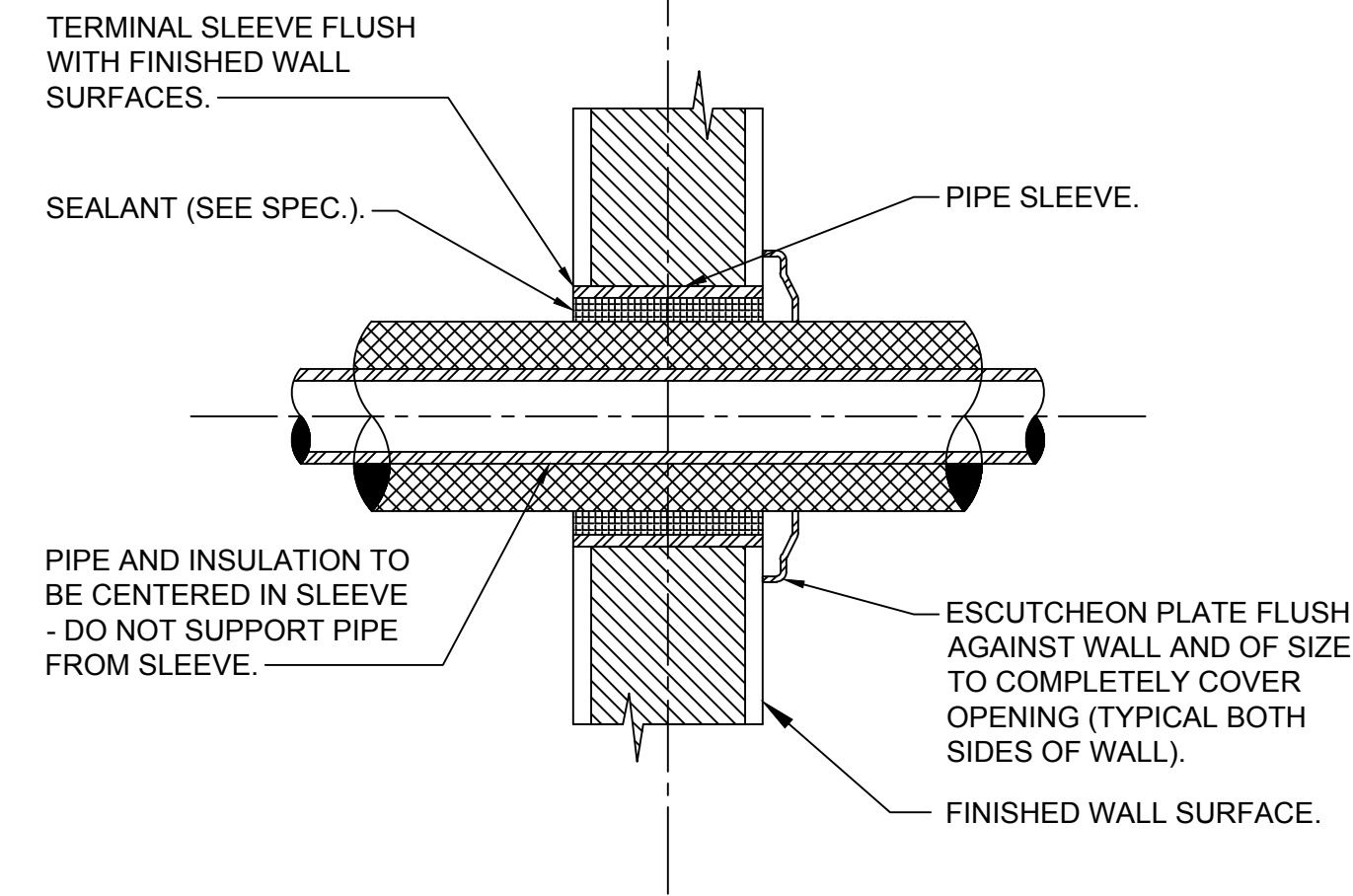
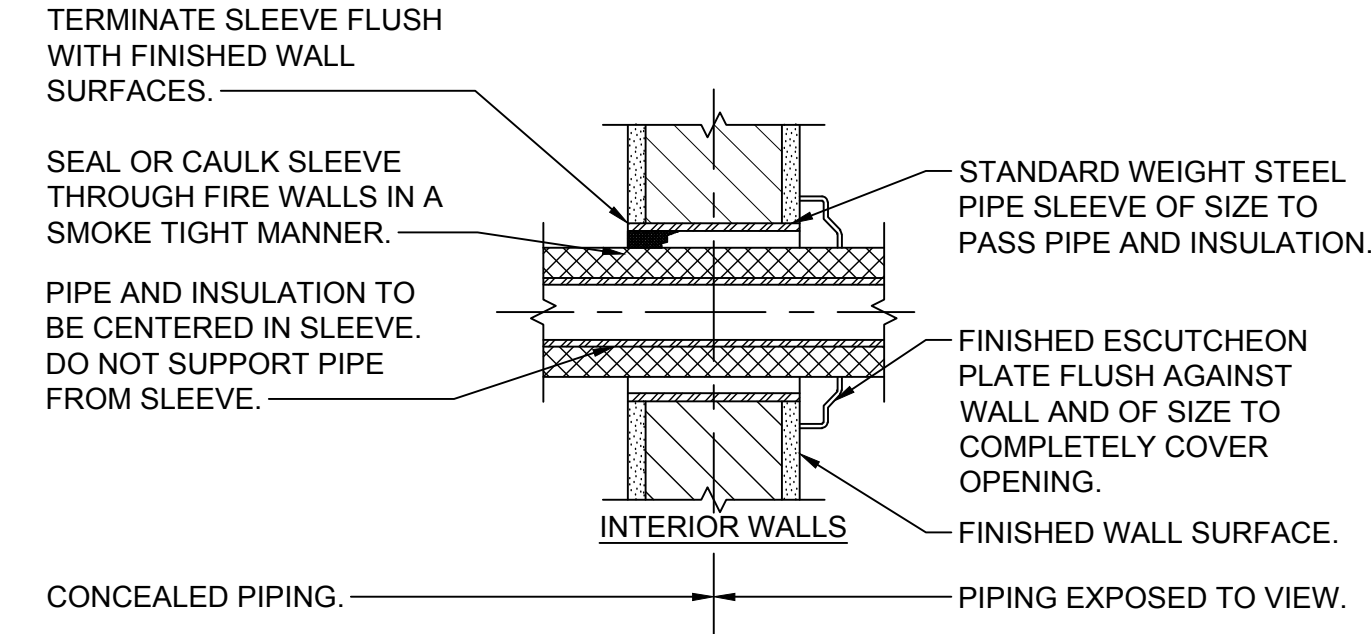
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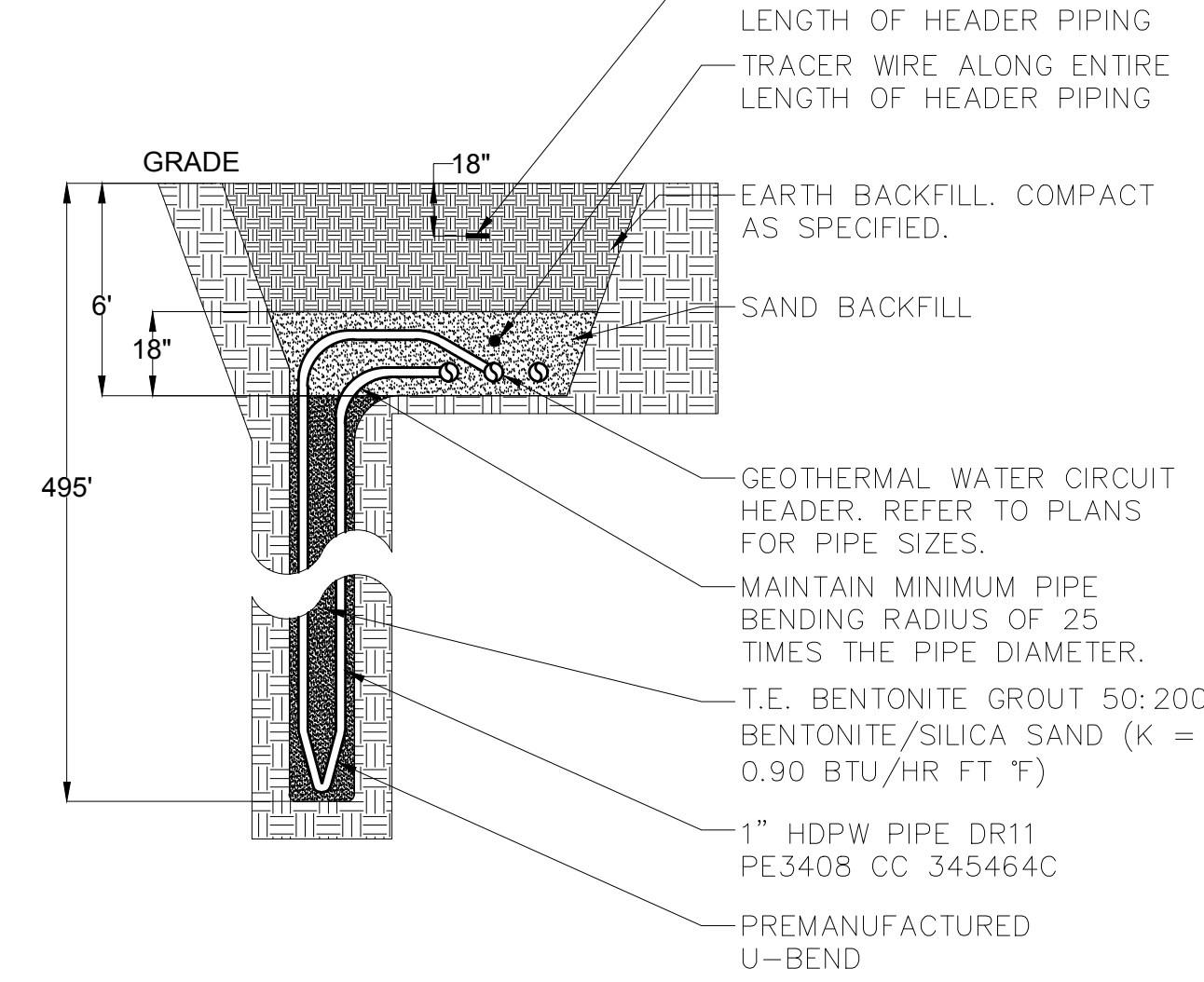
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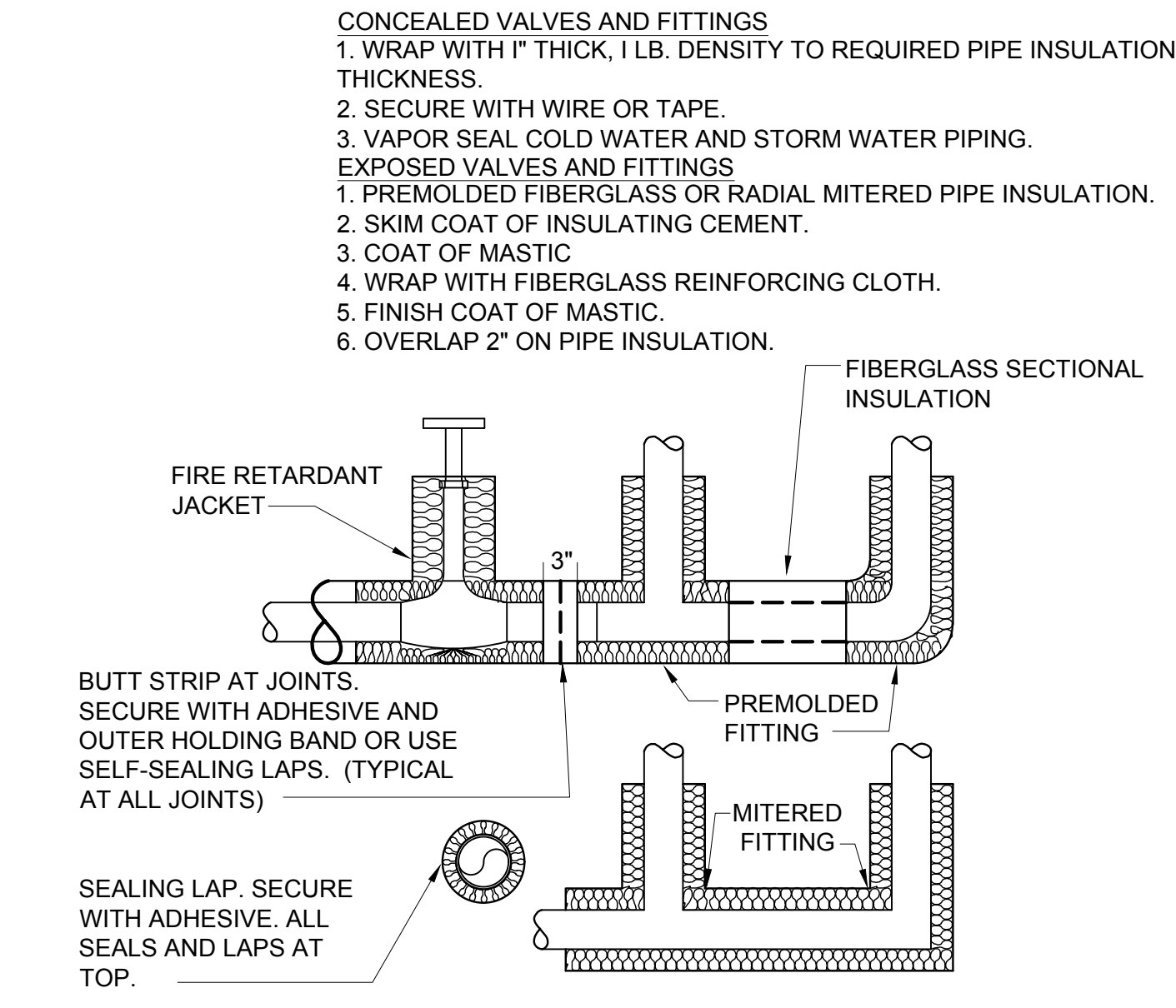
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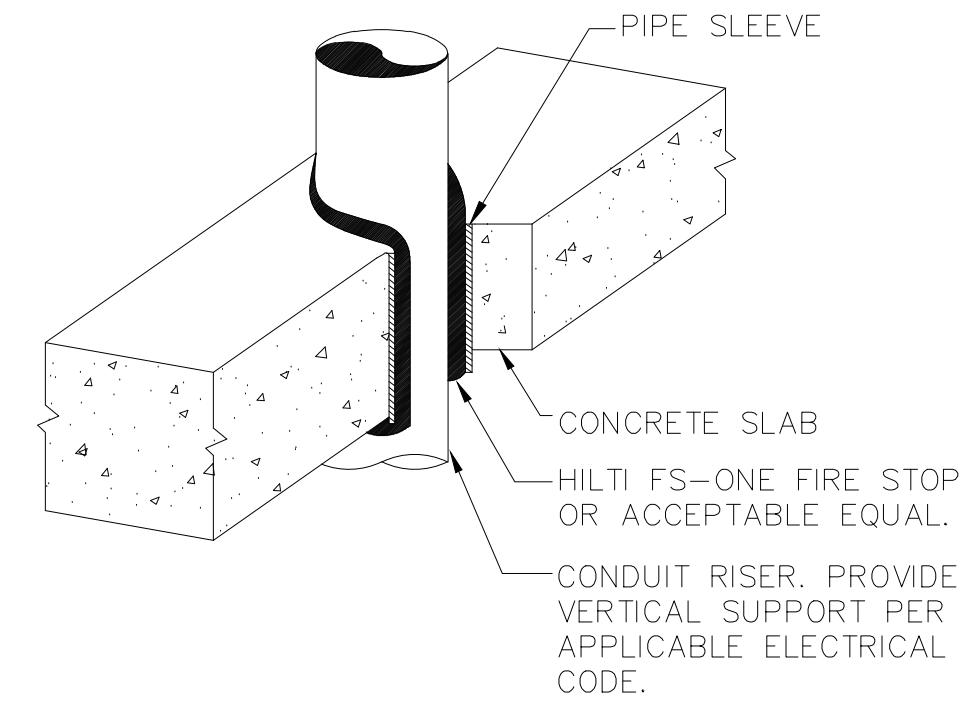
3 PIPE WALL SLEEVE DETAIL
SCALE: NONE



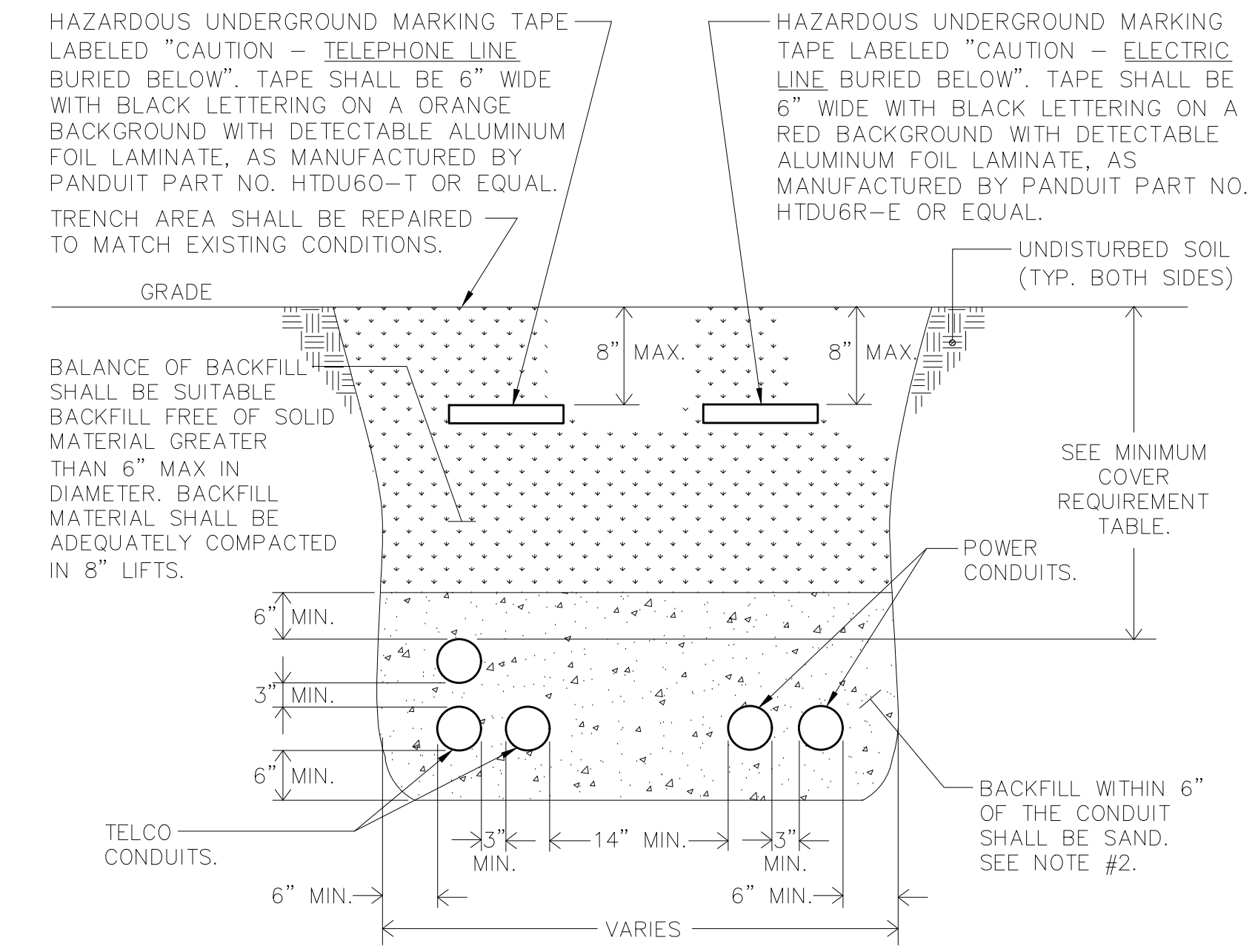
5 GEOTHERMAL WELL DETAIL
SCALE: NONE



8 PIPE INSULATION DETAIL
SCALE: NONE



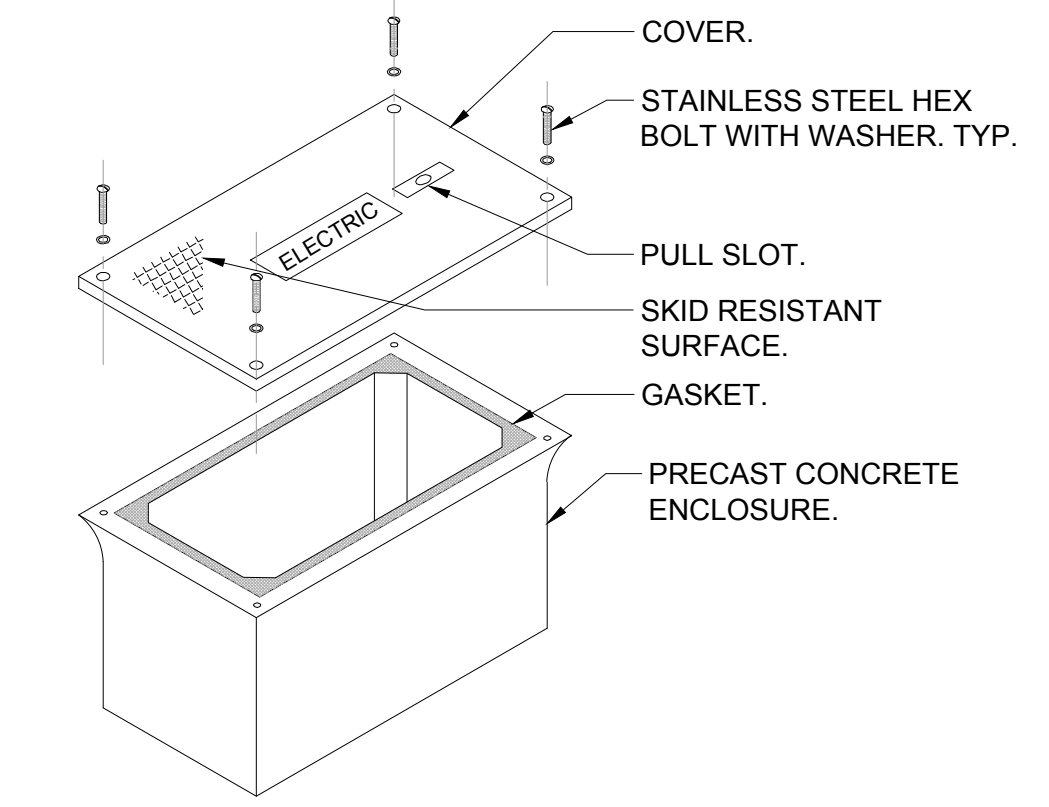
7 TYPICAL VERTICAL CONDUIT PENETRATION DETAIL
SCALE: NONE



MINIMUM COVER REQUIREMENT TABLE	
LOCATION	NONMETALLIC RACEWAYS LISTED FOR DIRECT BURIAL WITHOUT CONCRETE ENCASEMENT OR OTHER APPROVED RACEWAYS
ALL LOCATION NOT SPECIFIED BELOW.	18"
IN TRENCH BELOW 2-IN. THICK CONCRETE OR EQUIVALENT.	12"
UNDER MINIMUM OF 4-IN. THICK CONCRETE EXTERIOR SLAB WITH NO VEHICULAR TRAFFIC AND THE SLAB EXTENDING NOT LESS THAN 6 IN. BEYOND THE UNDERGROUND INSTALLATION.	4" SEE NOTE #2.
UNDER STREETS, HIGHWAYS, ROADS, ALLEYS, DRIVEWAYS, AND PARKING LOTS.	24"

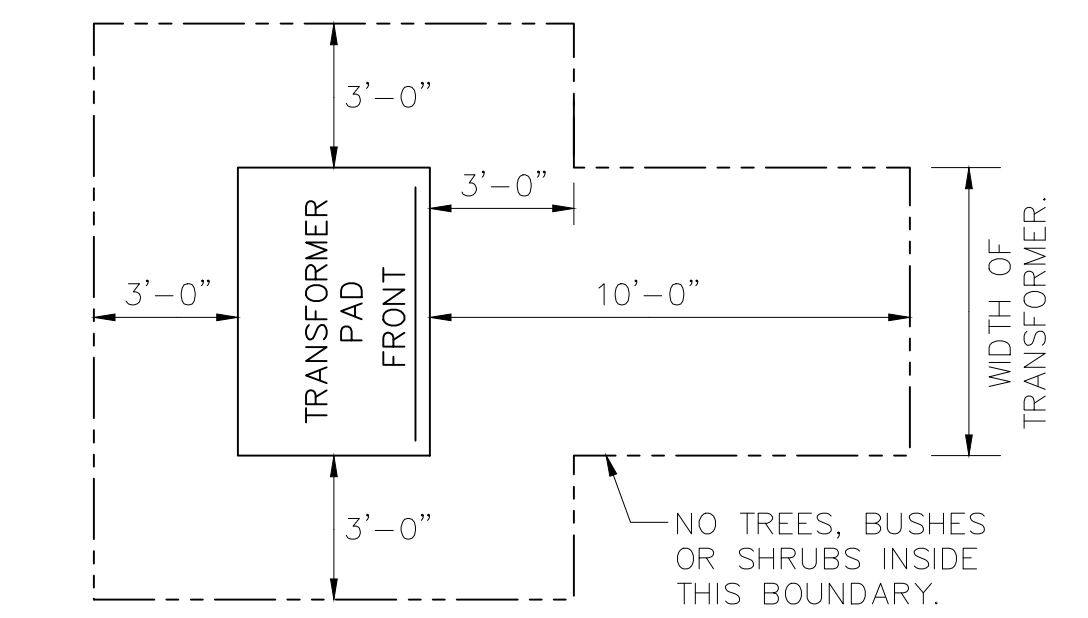
NOTES:
1.) DETAIL SHOWN FOR INFORMATION PURPOSES. SAME CONCEPT SHALL ALSO APPLY FOR SINGLE CONDUITS.
2.) SAND MAY BE OMITTED FOR INSTALLATIONS WHERE COVER REQUIREMENTS ARE 6" OR LESS.

4 TRENCHING DETAIL FOR CONDUIT
SCALE: NONE



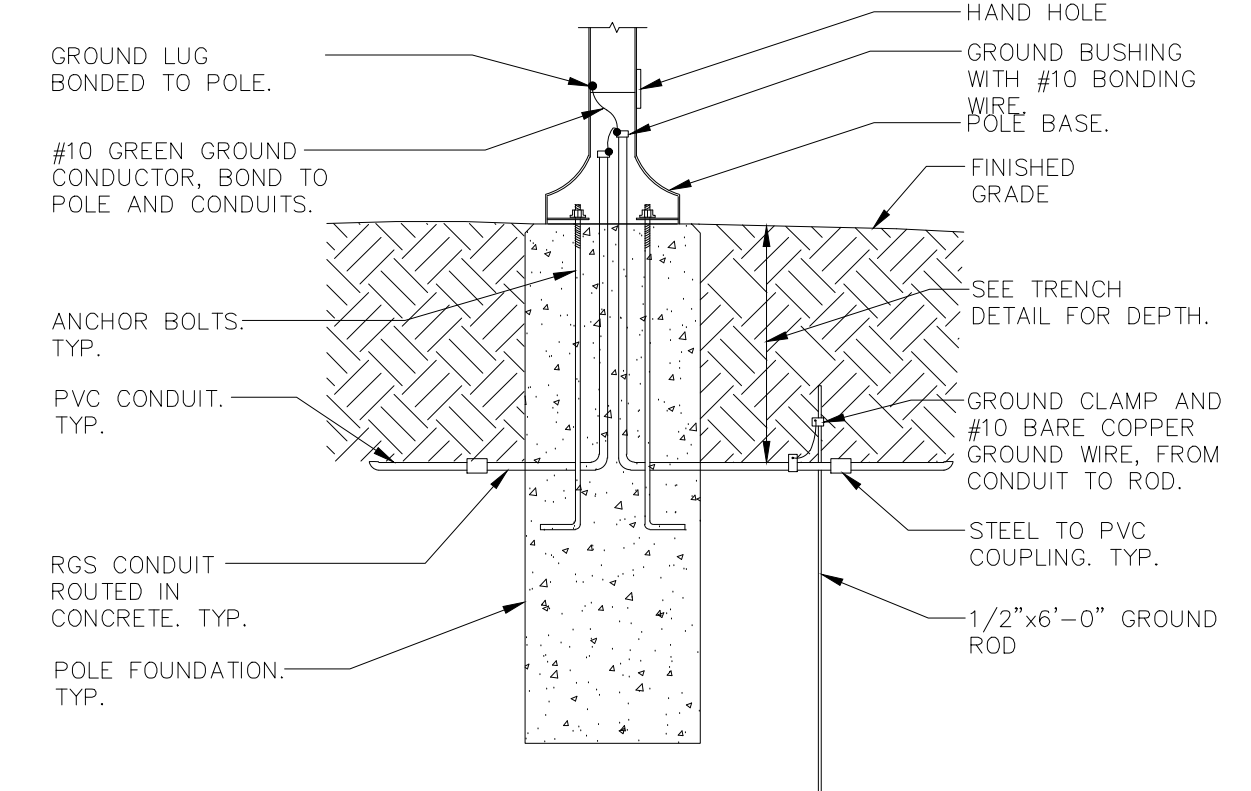
NOTES:
1.) HAND HOLE SHALL BE 12"L x 18"W x 30"H WITH OPEN BOTTOM AND RATED FOR VEHICULAR TRAFFIC AS MANUFACTURED BY HUBBELL-QUAZITE, OR EQUAL. BOX SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
2.) PROVIDE 6" OF GRAVEL BELOW BOX.
3.) COVER SHALL BE IMPRINTED WITH THE APPROPRIATE DESCRIPTION OF BOX CONTENTS (I.E. ELECTRIC, TELEPHONE, DATA, CABLE TV, ETC.).

2 HAND HOLE DETAIL
SCALE: NONE



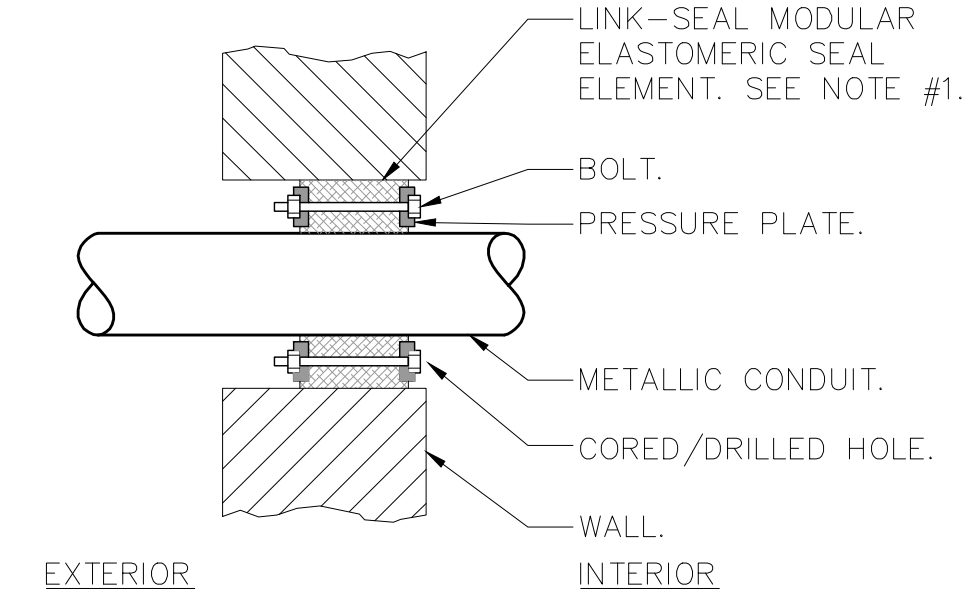
TRANSFORMER CLEARANCE NOTES:
1.) AREA IN FRONT OF DOOR TO BE CLEAR OF ALL ROCKS, STUMPS AND OTHER OBSTRUCTIONS SO THAT THE OPERATOR HAS A SAFE WORKING AREA.
2.) UTILITY COMPANY SHALL HAVE THE RIGHT TO CUT BACK GROWING BUSHES TO WITHIN STATED CLEARANCES.

1 TRANSFORMER PAD
SCALE: NONE



NOTE:
1.) SEE STRUCTURAL DRAWINGS FOR FOUNDATION DETAIL.

9 LIGHT POST ELECTRICAL
INSTALLATION DETAIL
SCALE: NONE



NOTES:
1.) SEAL ASSEMBLY BASED ON MODEL "C" LINK-SEAL MODULAR SEAL, WITH EPDM SEAL ELEMENT, REINFORCED NYLON POLYMER PRESSURE PLATES, STEEL WITH 2-PART ZINC DICHROMATE & CORROSION INHIBITING COATING NUTS AND BOLTS AND WITH A OPERATING TEMPERATURE RANGE OF -40°F TO +250°F.
2.) PROVIDE AND INSTALL TWO SEALS WHEN PENETRATED WALL THICKNESS IS GREATER THAN 12".
3.) PROVIDE SCHEDULE 80 WALL SLEEVE FOR NEW WALL CONSTRUCTION PER MANUFACTURER'S REQUIREMENTS.

6 TYPICAL EXTERIOR MASONRY WALL
BELOW GRADE CONDUIT
PENETRATION DETAIL
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DETAILS

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