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BID ADDENDUM NO. 4

Date of Addendum:	April 17, 2026
Issued for Bid Date:	March 27, 2026
Client Name:	Public Schools of the Tarrytowns
Project Name:	2024 Capital Bond Project – Phase 2; Washington Irving Intermediate School
SED Project No.:	66-04-01-03-0-002-017
TUFSD Bid Number:	25/26-TUFSD-003
MEMASI Project No.:	101-2403
Contracts:	Contract 1: General Construction (GC) Contract 2: Mechanical Construction (MC) Contract 3: Electrical Construction (EC) Contract 4: Plumbing Construction (PC)

This Bid Addendum forms part of the Contract Documents and modifies the original Issued for Bid Documents dated March 27, 2026. Where provisions of the following supplementary information differ from those of the original Bid Documents, this Addendum shall govern and take precedence.

The Bid Documents are modified and clarified as follows:

1. **GENERAL / CLARIFICATIONS**
   a. **Question:** This is referring to pages A901-A905. The sign schedule on A904 does not match the signage shown by floor on A901-A903. Which tables designate the signage that must be produced for this project? Also are there photos of existing signage, and do we need to match the style of the existing signage?
   Response: Disregard the sign schedule on Sheet A904. This project will provide all new signage. Therefore, matching existing signage is not required.

   b. Question: Kindly advise the wood door veneer. Nothing is called for in 081416.
   Response: Veneer for interior doors shall be Red Oak, with Chocolate CH18 finish (VT Industries designation).

   c. Question: Please advise if Zoomlock max braze free fittings are acceptable.
   Response: No, press fittings are not permitted for refrigerant piping.

   d. **Question: On** drawing MD100, it calls for the contractor to re-insulate the pipe where asbestos has been removed. Since the entire steam system is to be removed is this necessary or are we to provide temporary insulation until the removal of the system? Please advise.
   Response: Re-insulate existing piping to remain or piping that must remain active until removed according to the phasing plan.

   e. **Question:** If the Food Service scope is not entirely under the Plumbing Contract, please provide a clear breakdown and summary of responsibilities for the Food Service work, including which portions are to be carried out by each Prime Contractor. Please confirm if all rough-ins, final connections, and utility provisions (water, waste, gas, etc.) for food service equipment are to be included under the Plumbing Contract, or if any portion is by others.
   Response: Refer to “General Plumbing Notes” on Sheet FS102 and “General Electrical Notes” on Sheet FS103 for a summary of responsibilities for each prime contractor. Food Service

Equipment Contractor is defined as a sub-contractor to the General Contractor, meeting the qualification requirements of Section 114000 Paragraph 1.11.

f. **Question:** Which contractor is responsible for furnishing food service equipment?
Response: The Food Service Equipment Contractor, a sub-contractor to the General Contractor, is responsible for furnishing food service equipment in accordance with Section 114000 and the Food Service drawings.

g. **Question:** Exterior elevations noted MP for what looks like metal panels. Please confirm this is the same as OIP as shown on A613.
Response: Yes, MP noted on exterior elevation is the same as OIP as shown on A613. Delete glazing type MP on 20/A201. Add glazing type 0IP on 20/A201.

h. **Question:** Window elevations on A613 are calling for G-1 glazing which is fire rated. Shouldn't these windows be glazed with IG-1? Or IG-2? Please clarify.
Window type W1 has a panel that is labeled "G-V". Please clarify.
Response: Window elevations on A613 will be glazed with IG-1.
Response: Disregard the label "G-V" from window type W1.

i. **Question:** As per Addendum-3 dr. A502.1 issued showing demo/infill details for existing /1997 addition. Is it detail for louvers showing on dr. AD200 note D92? Please advise
Response: Yes, demo/infill details for existing/1997 addition is for louvers shown on drawing AD200 note D92.

i. **Question:** Borings or a Geotech report could not be found in the documents. Please provide.
Response: Refer to attached Appendix B - Report of Geotechnical Investigation.

j. **Question:** Spec 064023-2 refers to "Prefab Kitchen Casework" 123530. Spec 123530 is missing from Spec Book. Please provide.
Response: Section 123530 "Prefab Kitchen Casework" does not apply to this project.

k. **Question:** Please confirm who is responsible for the chimney liner showing on drawing MD100.
Response: The General Contractor shall be responsible for removing the chimney liner.

l. **Question:** DR. A201/20 South Elevation shows "MP" between windows W3. Please confirm its Interlocking Metal Wall Panel per spec 074213.13. Or provide specs/ detail otherwise
Response: No, Interlocking Metal Wall Panel is not to be used between windows W3 . Refer to Section 085113 for Opaque Insulated Panel "OIP" used between windows W3.

r. **Question:** Section 011100 Milestone Schedule is provided indicating phases. Phase 1 defines specific areas. Specific areas in Phase 2 and 3 are not provided. Can you provide specific locations on floor plan for Phase 2 and 3.
Response: Refer to "Phasing and Logistics" plans provided at the end of Specification Section 015000 Temporary Facilities and Controls.

2. CHANGES TO THE PROJECT MANUAL

a. SECTION 004116-2 BID FORM – MECHANICAL CONSTRUCTION – DELETE Add Alternate ALT 1: Chimney Removal.

b. SECTION 011000 – SUMMARY OF WORK
   i. Paragraph 1.3 General Requirements – All Contracts; ADD “21. All Contractors are reminded to pay special attention to the “Phasing and Logistics” plans provided at the end of specification section 015000 Temporary Facilities and Controls.”
   ii. Paragraph 1.4 General Work Contract (GWC or GC); ADD “11. GWC to provide temporary measures as per Drawing 1/C200. Work to be completed prior to 8/3/26 as per 011100 Milestone Schedule Phase 1-A Site Logistics Summer 2026.”

c. SECTION 012300 – ALTERNATES; Paragraph 3.1A DELETE the words “and Contract 2 – Mechanical Construction.”

d. SECTION 015000 - TEMPORARY FACILITIES AND CONTROLS
   i. DELETE Paragraph 3.30 “See attached Staging and Site Logistics Plan.”
   ii. ADD Paragraph 3.30 “See attached ‘Phasing and Logistics Plans.’”
   iii. DELETE Paragraph 3.31 “See attached ‘Temp Egress Sketches.’”

e. SECTION 055000 - MISCELLANEOUS METALS
   i. Paragraph 2.1A. ADD
      1. Aluminum Extrusions: ASTM B221 (ASTM B221M), Alloy 6063-T6.
   ii. Paragraph 2.6B. Ladders: DELETE “Vertical steel ladders”. ADD “Vertical steel or aluminum ladders”.

f. SECTION 060423 – INTERIOR ARCHITECTURAL WOODWORK; DELETE paragraph 1.3E.

g. SECTION 071326 – SHEET MEMBRANE WATERPOOFING; Paragraph 2.1A, ADD to the list of acceptable manufacturers: GMX Inc.

h. SECTION 102100 – TOILET COMPARTMENTS 2.1A; ADD : 2. Other Acceptable Manufacturer: Scranton Products; Scranton, PA, (704) 426-3354, https://www.scrantonproducts.com/.

i. APPENDIX B – REPORT OF GEOTECHNICAL INVESTIGATION; ADD APPENDIX B – REPORT OF GEOTECHNICAL INVESTIGATION.

3. CHANGES TO THE DRAWINGS

a. SHEET C100 - SITE PREP, DEMO, AND EROSION CONTROL PLAN; DELETE existing sheet C100. ADD Sheet C100, revised 4/17/2026, attached.

b. SHEET C200 – SITE LAYOUT PLAN; REVISE concrete landing at Door EXT-Q in accordance with Sketch SK-C200-1, attached.

c. SHEET C500 – SITE LAYOUT PLAN; DELETE existing sheet C500. ADD Sheet C500, revised 4/17/2026, attached.

d. SHEET AD101-B – DEMOLITION PART– GROUND FLOOR AREA B; DELETE existing sheet AD101-B. ADD Sheet AD101-B, revised 4/17/2026, attached.

e. SHEET A403 – GYMNASIUM INTERIOR ELEVATIONS; ADD acoustic panels in accordance with SHEET A403, revised 4/17/2026, attached.

f. SHEET A409 – KITCHEN AND HELP DESK ENLARGED PLAN & INTERIOR ELEVATIONS; DELETE existing sheet A409. ADD Sheet A409, revised 4/17/2026, attached.

g. SHEET A431 –ENLARGED PLANS & ELEVATIONS – SOUTH ENTRY; DELETE existing sheet A431. ADD Sheet A431, revised 4/17/2026, attached.

h. SHEET A502 – PLAN DETAILS; DELETE existing Details 10/A502 and 20/A502 on sheet A502, dated 03/27/2026; ADD Details 10/A502 and 20/A502 on sheet A502, revised 04/17/2026, attached.

i. SHEET A904 – SIGNAGE SCHEDULE; DELETE sheet A904 in its entirety.

j. SHEET S002 – GENERAL NOTES CONTINUED – REVISE existing 'STEEL BEAM LEGEND', with revisions as shown on the attached sketch. Revised 4/17/2026, attached.

k. SHEET S100 – BASEMENT FOUNDATION PLAN – AREA A – REPLACE existing sheet S100, with new Sheet S100, attached, in its entirety. Revised 4/17/2026, attached.

l. SHEET S101 – GROUND FLOOR FRAMING PLAN – AREA A – REPLACE existing sheet S101, with new Sheet S101, attached, in its entirety. Revised 4/17/2026, attached.

m. SHEET S102 – FIRST FLOOR FRAMING PLAN – AREA A – REPLACE existing sheet S102, with new Sheet S102, attached, in its entirety. Revised 4/17/2026, attached.

n. SHEET S103 – SECOND FLOOR FRAMING PLAN – AREA A – REPLACE existing sheet S103, with new Sheet S103, attached, in its entirety. Revised 4/17/2026, attached.

o. SHEET S200 – PARTIAL PLANS AND DETAILS – REVISE existing detail 10/S200 with revisions shown on the attached sketches. Revised 4/17/2026, attached.

p. SHEET S200 – PARTIAL PLANS AND DETAILS – REVISE existing detail 11/S200 with revisions shown on the attached sketches. Revised 4/17/2026, attached.

q. SHEET S200 – PARTIAL PLANS AND DETAILS – ADD detail 12/S200 to existing sheet S200 with revisions shown on the attached sketches. Revised 4/17/2026, attached.

r. SHEET S302 – CONCRETE SECTIONS AND DETAILS – REVISE existing detail 6/S302 with revisions shown on the attached sketches. Revised 4/17/2026, attached.

s. SHEET S501 – STEEL SECTIONS AND DETAILS – ADD detail 7/S501, to existing sheet S501 with revisions shown on the attached sketches. Revised 4/17/2026, attached.

t. SHEET S502 – SECTIONS AND DETAILS – REVISE existing detail 7/S502 with revisions shown on the attached sketches. revised 4/17/2026, attached.

u. SHEET M104-A – MECHANICAL DUCTWORK PART PLAN – SECOND FLOOR – AREA A; DELETE existing sheet M104-A. ADD Sheet M104-A, revised 4/17/2026, attached.

v. SHEET M106-A – MECHANICAL PART PLAN – ROOF – AREA A; DELETE existing sheet M106-A. ADD Sheet M106-A, revised 4/17/2026, attached.

w. SHEET M106-B – MECHANICAL PART PLAN – ROOF – AREA B; DELETE existing sheet M106-B. ADD Sheet M106-A, revised 4/17/2026, attached.

x. SHEET M201 – MECHANICAL PIPING PLAN – BASEMENT; DELETE existing sheet M201. ADD Sheet M201, revised 4/17/2026, attached.

y. SHEET M202-A – MECHANICAL PIPING PART PLAN – GROUND FLOOR AREA A; DELETE existing sheet M202-A. ADD Sheet M202-A, revised 4/17/2026, attached.

z. SHEET M204-A – MECHANICAL PIPING PART PLAN – SECOND FLOOR AREA A; DELETE existing sheet M204-A. ADD Sheet M204-A, revised 4/17/2026, attached.

aa. SHEET M204-B – MECHANICAL PIPING PART PLAN – SECOND FLOOR AREA B; DELETE existing sheet M204-B. ADD Sheet M204-B, revised 4/17/2026, attached.

bb. SHEET M206-A – MECHANICAL PIPING PART PLAN – ROOF AREA A; DELETE existing sheet M206-A. ADD Sheet M206-A, revised 4/17/2026, attached.

cc. SHEET M206-B – MECHANICAL PIPING PART PLAN – ROOF AREA B; DELETE existing sheet M206-B. ADD Sheet M206-B, revised 4/17/2026, attached.

dd. SHEET E101 – ELECTRICAL POWER PLAN – BASEMENT; DELETE existing sheet E101. ADD Sheet E101, revised 4/17/2026, attached.

ee. SHEET E102-A – ELECTRICAL POWER PLAN – GROUND FLOOR - AREA A; DELETE existing sheet E102-A. ADD Sheet E102-A, revised 4/17/2026, attached.

ff. SHEET E102-B – ELECTRICAL POWER PLAN – GROUND FLOOR - AREA B; DELETE existing sheet E102-B. ADD Sheet E102-B, revised 4/17/2026, attached.

gg. SHEET E103-A – ELECTRICAL POWER PLAN – FIRST FLOOR - AREA A; DELETE existing sheet E103-A. ADD Sheet E103-A, revised 4/17/2026, attached.

hh. SHEET E103-B – ELECTRICAL POWER PLAN – FIRST FLOOR - AREA B; DELETE existing sheet E103-B. ADD Sheet E103-B, revised 4/17/2026, attached.

ii. SHEET E103-A – ELECTRICAL POWER PLAN – FIRST FLOOR - AREA A; DELETE existing sheet E103-A. ADD Sheet E103-A, revised 4/17/2026, attached.

jj. SHEET E104-A – ELECTRICAL POWER PLAN – SECOND FLOOR - AREA A; DELETE existing sheet E104-A. ADD Sheet E104-A, revised 4/17/2026, attached.

kk. SHEET E104-B – ELECTRICAL POWER PLAN – SECOND FLOOR - AREA B; DELETE existing sheet E104-B. ADD Sheet E104-B, revised 4/17/2026, attached.

ll. SHEET E105 – ELECTRICAL POWER PLAN – THIRD FLOOR; DELETE existing sheet E105. ADD Sheet E105, revised 4/17/2026, attached.

mm. SHEET E106-A – ELECTRICAL POWER PART PLAN – ROOF – AREA A; DELETE existing sheet E106-A. ADD Sheet E106-A, revised 4/17/2026, attached.

nn. SHEET E601 – ELECTRICAL PANEL SCHEDULE SHEET 1; DELETE existing sheet E601. ADD Sheet E601, revised 4/17/2026, attached.

oo. SHEET E603 – ELECTRICAL PANEL SCHEDULE SHEET 1; DELETE existing sheet E603. ADD Sheet E603, revised 4/17/2026, attached.

pp. SHEET PD102-B –PLUMBING DEMOLITION PART PLAN – GROUND FLOOR – AREA B; DELETE existing sheet PD102-B. ADD Sheet PD102-B, revised 4/17/2026, attached.

qq. SHEET P104-A –PLUMBING PART PLAN – SECOND FLOOR – AREA A; DELETE existing sheet P104-A. ADD Sheet P104-A, revised 4/17/2026, attached.

rr. SHEET P106-A –PLUMBING PART PLAN – ROOF – AREA A; DELETE existing sheet P106-A. ADD Sheet P106-A, revised 4/17/2026, attached.

ss. SHEET P502 –PLUMBING RISER DIAGRAMS; DELETE existing sheet P502. ADD Sheet P502, revised 4/17/2026, attached.

END OF BID ADDENDUM NO. 4

APPENDIX B

REPORT OF GEOTECHNICAL INVESTIGATION

MEMASI

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WARREN, NJ 07059
908.668.7777
whitestoneassoc.com

REPORT OF GEOTECHNICAL INVESTIGATION

WASHINGTON IRVING INTERMEDIATE SCHOOL
PROPOSED ADDITION & SITE IMPROVEMENTS
103 SOUTH BROADWAY
TARRYTOWN, WESTCHESTER COUNTY, NEW YORK

Prepared for:

MEMASI
Two Lyon Place
White Plains, New York 10601

Prepared by:

WHITESTONE ENGINEERING & GEOLOGY NY, PLLC
30 Independence Boulevard
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Mudar Khantamr, PE
Senior Associate

Laurence W. Keller, PE
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Whitestone Project No.: GJ2422051.Y00
October 1, 2024

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SUITE 250
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October 1, 2024

via email

MEMASI
Two Lyon Place
White Plains, New York 10601

Attention: Daryl Mastracci, PE, LEED AP BD+C
Managing Partner

Regarding: REPORT OF GEOTECHNICAL INVESTIGATION
WASHINGTON IRVING INTERMEDIATE SCHOOL
PROPOSED ADDITION & SITE IMPROVEMENTS
103 SOUTH BROADWAY
TARRYTOWN, WESTCHESTER COUNTY, NEW YORK
WHITESTONE PROJECT NO.: GJ2422051.Y00

Dear Mr. Mastracci:

Whitestone Associates Engineering & Geology NY, PLLC (Whitestone) is pleased to submit the attached *Report of Geotechnical Investigation* for the above-referenced project. The attached report presents the results of Whitestone's soils exploration efforts and presents recommendations for design of the proposed structural foundations, floor slabs, pavements, and related earthwork associated with the proposed development.

Whitestone's Geotechnical Division appreciates the opportunity to be of continued service to MEMASI. Please note that Whitestone has the capability to conduct the additional geotechnical engineering services recommended herein. Please contact us with any questions regarding the enclosed report.

Sincerely,

WHITESTONE

Mudar Khantamr, PE
Senior Associate

Laurence W. Keller, PE
Vice President

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Enclosures
Copy: Paul A. Rouis III, P.E., Ryan Biggs Clark Davis
Rasul Ali, Ryan Biggs Clark Davis

REPORT OF GEOTECHNICAL INVESTIGATION

Washington Irving Intermediate School
Proposed Additions & Site Improvements
103 South Broadway
Tarrytown, Westchester County, New York

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Washington Irving Intermediate School
Proposed Additions & Site Improvements
103 South Broadway
Tarrytown, Westchester County, New York

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SECTION 1.0
Summary of Findings

Whitestone has conducted an exploration and evaluation of the subsurface conditions at the subject site located at 103 South Broadway in Tarrytown, Westchester County, New York. The site of the proposed redevelopment is shown on the *Test Location Plan* included as Figure 1.

At the time of Whitestone's site investigation, the subject site consisted of the Washington Irving Intermediate School including a three-story building, athletic field, pavements, landscaping, and utilities.

Based on the June 20, 2024 *Structural Plans* prepared by Ryan Biggs Clark Davis (RBCD), March 30, 2023 *Request for Geotechnical Engineering* prepared by RBCD, undated *Geotechnical Scope of Services* and *Soil Boring Sketch* prepared by RBCD, the proposed redevelopment is anticipated to include constructing an approximately 6,000-square feet (footprint), three-story building addition to the north and west of the existing building, replacing the existing grandstand with a new grandstand and associated site retaining walls, pavements, and new stormwater management (SWM) area.

The subsurface exploration included conducting a reconnaissance of the project site, drilling soil test borings, conducting an in-situ infiltration test, and collecting soil samples for laboratory analyses. The data from this exploration was analyzed by Whitestone in light of the project information provided by Memasi.

A summary of Whitestone's findings is presented in the following:

- **Subsurface Conditions:** The borings were conducted within landscaped and paved areas of the site. Underlying the surface cover, two borings encountered existing fill consisting silty sand with gravel. Where encountered, the existing fill extended to a depth of approximately two feet below ground surface (fbgs). Beneath the surface cover and/or existing fill the borings encountered natural residual soils generally composed of a mixture of sand, silt, clay, and gravel (USCS: SM, ML, and CL). The borings, with the exception of B-5, B-6, and B-8, were terminated within the residual soils at depths ranging from approximately 12 fbgs to 23.5 fbgs. Beneath the residual soils, borings B-5, B-6, and B-8 and were subsequently terminated within weathered rock or bedrock at depths ranging from approximately 15 fbgs to 24 fbgs. Static groundwater was encountered within borings B-1, B-2, and B-4 during drilling operations at depths ranging from approximately 11 fbgs to 14 fbgs. Additionally, a temporary groundwater piezometer was installed at boring B-4 and groundwater was measured to be approximately 12 fbgs on August 26, 2024 (approximately one month following fieldwork). Static and perched/trapped water conditions are expected to fluctuate seasonally and following periods of precipitation.

Recommendations developed upon consideration of these results are summarized below and presented in greater detail in the following report.

- **Foundations:** Whitestone recommends supporting the proposed structures on conventional shallow spread and continuous wall footings designed to bear within the underlying improved natural residual soils and/or on controlled structural fill materials that are properly inspected, placed, and compacted as described herein. The upper natural residual soils were relatively loose/soft and will require improvement via in-place compaction prior to structural support. Areas of overexcavation and recompaction also should be anticipated. Foundations bearing within these materials may be designed using a maximum allowable net bearing pressure of 2,500 pounds per square foot (psf).

- **Floor Slabs and Pavements:** Whitestone anticipates that the improved site soils and/or controlled structural fill will be suitable for support of the proposed floor slabs and pavements provided these materials are properly evaluated, placed, and proofrolled as recommended herein. Any areas that become softened or disturbed as a result of wetting and/or repeated exposure to construction traffic should be removed and replaced with compacted structural fill. The upper 12 inches of all subgrades should be recompacted in-place under the observation of the owner's geotechnical engineer due to the presence of loose/soft materials.

- **Soil Reusability/Moisture Sensitivity:** Whitestone anticipates that the majority of the existing fill and underlying natural materials will be marginally suitable for selective reuse as structural fill and/or backfill below proposed foundations, floor slabs, and pavements provided moisture contents are controlled within two percent of the optimum moisture content. Reuse of the existing fill will be contingent on careful inspection in the field by the owner's geotechnical engineer by visual observation and/or test pit excavations during construction as recommended herein. The reuse of the fine-grained site materials (USCS: ML and CL) and granular materials with more than 12 percent fines (USCS: SM) typically requires extended periods of warm and dry weather conditions. Soil reuse will typically require mixing with a more granular material and/or drying. The on-site soils will become increasingly difficult to reuse and compact where wetted beyond the optimum moisture content. Immediate re-use of on-site soil should not be expected. Alternatively, imported fill materials may be used to attain the desired grades and expedite earthwork operations during wet weather periods. The contractor should cover stockpiled soils, seal subgrades, and provide proper surface drainage during forecasted wet weather. The stripped topsoil and asphalt should not be used as fill or backfill.

More detailed design criteria and construction recommendations for proposed foundations, slabs, pavements, and earthwork are discussed in the following report.

SECTION 2.0
Introduction

2.1 AUTHORIZATION

Daryl Mastracci, PE, LEED AP BD+C with MEMASI issued authorization to Whitestone to conduct the geotechnical investigation at this site relevant to the proposed addition and site improvements located at 103 South Broadway in Tarrytown, Westchester County, New York. The geotechnical investigation was conducted in general accordance with Whitestone's revised proposal dated June 26, 2024.

2.2 PURPOSE

The purpose of this subsurface exploration and analysis was to:

- ascertain the various soil profile components at test locations;
- estimate the engineering characteristics of the proposed foundation bearing and subgrade materials;
- provide geotechnical criteria for use by the design engineers in preparing the foundations, floor slab, and pavements;
- provide recommendations for required earthwork and subgrade preparation;
- record groundwater and bedrock levels, where encountered, at the time of the investigation and discuss the potential impact on the proposed construction; and
- recommend additional investigation and/or analysis (if warranted).

2.3 SCOPE

The scope of the exploration and analysis included the subsurface exploration, field testing and sampling, laboratory analyses, and a geotechnical engineering analysis and evaluation of the subsurface materials. This *Report of Geotechnical Investigation* is limited to addressing the site conditions related to the physical support of the proposed construction.

2.3.1 Field Exploration

The field exploration of the project site was conducted by means of eight test borings (identified as B-1 through B-8) conducted with track-mounted drill rig using hollow stem augers and split-spoon sampling

techniques to depths ranging from approximately 12 fbgs to 24 fbgs. Additionally, one in-situ infiltration test (identified as I-1) was conducted. The tests were backfilled with excavated soils generated from the investigation and surficially restored with asphaltic concrete cold patch, where necessary. The locations of the subsurface tests are shown on the accompanying *Test Location Plan* included as Figure 1.

The subsurface tests were conducted in the presence of Whitestone personnel who conducted field tests, recorded visual classifications, and collected samples of the various strata encountered. The test locations were located in the field using normal taping procedures and estimated right angles. These locations are presumed to be accurate within a few feet.

The borings and Standard Penetration Tests (SPTs) were conducted in general accordance with ASTM International (ASTM) designation D 1586. The Standard Penetration Resistance value (N) can be used as an indicator of the consistency of fine-grained soils and the relative density of coarse-grained soils. The N-value for various soil types can be correlated with the engineering behavior of earthworks and foundations. Rock was sampled using a NQ-sized diamond bit. The rock core description, recovery, Rock Quality Designation (RQD), and other pertinent information were recorded on the boring logs and are included in Appendix A on the *Records of Subsurface Exploration*. The RQD values reflect the quality and fracture spacing of the rock and are calculated by summing all unbroken samples that are four inches or longer divided by the total length of the run. The percentage of core recovery and RQD values provide an understanding of the physical and engineering properties of the rock.

Groundwater level observations, where encountered, were recorded during and immediately following the completion of the testing operations within the borings. Additionally, a temporary groundwater monitoring piezometer was installed at boring B-4. Seasonal variations, temperature effects, and recent rainfall conditions may influence the levels of the groundwater, and the observed levels will depend on the permeability of the soils. Groundwater elevations derived from sources other than seasonally observed groundwater monitoring wells may not be representative of true groundwater levels.

2.3.2 Laboratory Program

Representative samples of selected strata encountered were subjected to a laboratory program that included Atterberg limits determination (ASTM D-4318), moisture content determinations (ASTM D-2216), and washed gradation analyses (ASTM D-422) in order to conduct supplementary engineering soil classifications in general accordance with ASTM D-2487. The soil strata tested were classified by the Unified Soil Classification System (USCS) and results of the laboratory testing are summarized in the following table. Quantitative test results are provided in Appendix B.

PHYSICAL/TEXTURAL ANALYSES SUMMARY							
Source of Sample	**Sample Number**	**Depth (fbgs)**	**Passing No. 200 Sieve (%)**	**Natural Moisture Content (%)**	**Liquid Limit (%)**	**Plastic Index (%)**	**USCS Classification**
B-1	S-4	6.0 - 8.0	65.7	21.3	25	1	ML
B-3	S-2	2.0 - 4.0	63.5	9.9	NP	NP	ML
B-4	S-3	4.0 - 6.0	63.4	11.2	NP	NP	ML
B-7	S-3	4.0 - 6.0	71.5	17.8	23	2	ML

NP = Non-Plastic

The engineering classifications are useful when considered in conjunction with the additional site data to estimate properties of the soil types encountered and to predict the soil's behavior under construction and service loads.

2.3.3 Infiltration Testing

An in-situ infiltration test (I-1) was conducted at the anticipated level of infiltration within the proposed SWM area. Infiltration testing was conducted using the falling head test method. The test resulted in an infiltration rate of 0.0 inches per hour. Infiltration test results are provided in Appendix C.

SECTION 3.0
Site Description

3.1 LOCATION AND DESCRIPTION

The subject site is located at 103 South Broadway in Tarrytown, Westchester County, New York. The site is bound to the north by Franklin Street, to the east by Broadway, to the south by residential properties, and to the west by Miller Avenue and residential properties. The site of the proposed construction is shown on the *Test Location Plan* included as Figure 1.

3.2 EXISTING CONDITIONS

Surface Cover/Development: At the time of Whitestone's site investigation, the subject site consisted of the Washington Irving Intermediate School including a three-story building, athletic field, pavements, landscaping, and utilities.

Topography: A topographic survey was not available at the time of this report. Based on visual observations, the proposed redevelopment areas have estimated grade changes on the order of approximately three feet to five feet.

Utilities: At the time of Whitestone's subsurface field investigation, the subject site was serviced by public and private utilities including underground natural gas, electric, communications, water, and stormwater sewer lines. Other utilities were not observed at the subject site by Whitestone but may be present. The utility information contained in this report is presented for general discussion only and is not intended for construction purposes.

Site Drainage: Surface run-off for the site generally follows existing topography draining towards inlets located within paved portions of the site. The termini of the inlets are unknown.

3.3 SITE GEOLOGY

The subject site is located within the Manhattan Prong of the New England Uplands Physiographic Province. The area is underlain by the Fordham Gneiss formation which includes gneiss and amphibolite members. The materials overlying the Gneiss bedrock consist of partly weathered rock and residual soil formed by the in place weathering of the parent rock. The overburden materials at the site also include manmade fill associated with previous and present development of the site.

3.4 PROPOSED CONSTRUCTION

Based on the *Structural Plans*, *Request for Geotechnical Engineering*, and *Geotechnical Scope of Services* and *Soil Boring Sketch* prepared by RBCD, the proposed redevelopment is anticipated to include constructing an approximately 6,000-square feet (footprint), three-story building addition to the north and west of the existing building, replacing the existing grandstand with a new grandstand and associated site retaining walls, pavements, and new SWM area. The proposed building addition is not anticipated to include a below-grade level.

Detailed grading information was not provided at this time of this report. Whitestone assumes the proposed building addition finished floor elevation (FFE) will match the existing building FFE resulting in cuts/fills on the order of approximately one foot to three feet. Whitestone assumes the proposed retaining wall will have a maximum height of up to five feet.

The anticipated maximum loads are expected to be less than the following:

- column loads - 225 kips;
- wall loads - 3.0 kips/linear foot; and
- floor slabs - 125 pounds per square foot.

The above-referenced structural loads were assumed based upon Whitestone's previous experience with similar facilities and should be confirmed by the structural engineer. The scope of Whitestone's investigation and the professional advice contained in this report were generated based on the project details noted herein. Any revisions or additions to the design details enumerated in this report should be brought to the attention of Whitestone for additional evaluation as warranted.

SECTION 4.0
Subsurface Conditions

Details of the subsurface materials encountered are presented on the *Records of Subsurface Exploration* presented in Appendix A of this report. The subsurface soil conditions encountered in the borings consisted of the following generalized strata in order of increasing depth.

4.1 SUBSURFACE SOIL CONDITIONS

Surface Cover: The borings were conducted within landscaped and paved areas of the site. The borings conducted within existing landscaped areas encountered approximately five inches to 12 inches of topsoil at the surface. The borings conducted within existing paved areas encountered approximately three inches of asphaltic concrete pavement at the surface. Apparent subbase materials were not encountered.

Existing Fill: Underlying the surface cover, borings B-4 and B-5 encountered existing fill consisting silty sand with gravel. Where encountered, the existing fill extended to a depth of approximately two fbgs. SPT N-values recorded within the existing fill were 19 blows per foot (bpf) and 48 bpf.

Residual Deposits: Beneath the surface cover and/or existing fill the borings encountered natural residual deposits generally composed of a mixture of sand, silt, clay, and gravel (USCS: SM, ML, and CL). The borings, with the exception of B-5, B-6, and B-8, were terminated within the residual soils at depths ranging from approximately 12 fbgs to 23.5 fbgs. SPT N-values within coarse-grained portions of this stratum ranged between four blows per foot (bpf) and refusal (refusal defined as more than 50 blows per six inches of split spoon sampler advancement), generally indicating loose to very dense relative density and averaging approximately 29 bpf. Pocket penetrometer tests conducted within fine-grained portions of this stratum resulted in unconfined compressive strengths ranging between approximately 0.5 ton per square foot (tsf) and 1.5 tsf, generally indicating medium stiff to stiff consistency.

Weathered Rock: Beneath the residual soils, borings B-5, B-6, and B-8 encountered weathered rock at depths ranging from approximately 10 fbgs and 13 fbgs. Borings B-5 and B-8 were terminated within the weathered rock at depths of approximately 15 fbgs and 15.3 fbgs, respectively. Within B-6, the weathered rock extended to a depth of approximately 19 fbgs. SPT N-values within this stratum consistently were in the refusal range.

Bedrock: Beneath weathered rock materials, boring B-6 encountered auger refusal on top of apparent bedrock at a depth of approximately 19 fbgs. The bedrock was sampled with rock coring techniques and consisted of gneiss. The rock core recovery was approximately 100 percent and the RQD value was approximately 33 percent.

4.2 GROUNDWATER

Static groundwater was encountered within borings B-1, B-2, and B-4 during drilling operations at depths ranging from approximately 11 fbgs to 14 fbgs. Additionally, a temporary groundwater piezometer was installed at boring B-4 and groundwater was measured to be approximately 12 fbgs on August 26, 2024 (approximately one month following fieldwork). Static and perched/trapped water conditions are expected to fluctuate seasonally and following periods of precipitation.

SECTION 5.0
Conclusions and Recommendations

5.1 GENERAL

Whitestone recommends supporting the proposed structures on conventional shallow spread and continuous wall footings designed to bear within the underlying improved natural residual soils and/or on controlled structural fill materials that are properly inspected, placed, and compacted as described herein. The upper natural residual soils were relatively loose/soft and will require improvement via in-place compaction prior to structural support. Areas of overexcavation and recompaction also should be anticipated.

Whitestone anticipates that following supplemental evaluation and compaction of the upper site soils, the proposed floor slabs and pavements may be supported on improved and approved existing fill, underlying natural materials, and/or controlled structural fill provided these materials are prepared in accordance with the recommendations herein including areas of overexcavation and replacement, recompaction, and/or mechanical stabilization anticipated, due to the presence of variable existing fill and loose/soft natural soils, and as evidenced by the variable SPT-N values.

The majority of the site soils are fine-grained and are especially moisture sensitive. Therefore, immediate reuse may not be practical. Construction schedules and budgets should account for earthwork contingencies, such as moisture control and importing materials to raise grades, restore overexcavations, and backfill utility trenches when construction must occur following wet weather or on an expedited basis.

5.2 SITE PREPARATION AND EARTHWORK

Surface Cover Stripping and Demolition: Prior to stripping operations, all utilities should be identified and secured. The existing grandstand and pavements to be demolished and stripped should be removed from within and at least five feet beyond the limits of areas requiring structural fill. Existing structural elements, such as foundation walls, or any concrete foundations, walls or slabs should be removed entirely from below proposed foundations and their zones of influence (as determined by lines extending at least one foot laterally beyond footing edges for each vertical foot of depth) and excavated to at least two feet below proposed construction subgrade levels elsewhere. Foundations and slabs may remain in place below these depths below proposed ground-supported slabs, pavements and landscaped areas, provided interference with future construction is avoided, however, any existing slab to remain should be thoroughly broken such that maximum particle size is 12 inches to allow vertical drainage of water. The demolition contractor should be required to conduct all earthwork in accordance with the recommendations in this report including backfilling any excavation, utility, etc. with structural fill. All fill or backfill placed in structural areas during any demolition operations should be placed as structural fill in accordance with Section 5.2, 5.3, and 5.11 of this report.

Surface Preparation/Proofrolling: Prior to placing any fill or subbase materials to raise or restore grades to the desired subgrade elevations, the existing exposed soils should be compacted to a firm surface with several passes in two perpendicular directions of a minimum 10-ton roller. The roller should be operated in the static mode or a kneading "sheepsfoot" roller should be used if silt and/or clay soils are encountered at subgrade elevations. The surface then should be proofrolled with a loaded tandem axle truck in the presence of the geotechnical engineer to help identify soft or loose pockets which may require removal and replacement or further investigation. Proofrolling should be conducted after a suitable period of dry weather to avoid degrading an otherwise stable subgrade. Any fill or backfill should be placed and compacted in accordance with Section 5.3.

Weather Performance Criteria: Because the majority of the site soils are highly moisture sensitive and will soften when exposed to water, every effort must be made to maintain drainage of surface water runoff away from construction areas by grading and limiting the exposure of excavations and prepared subgrades to rainfall. Accordingly, excavation and fill placement procedures should be conducted during favorable weather conditions. Overexcavation of saturated soils and replacement with controlled structural fill per Section 5.3 of this report may be required prior to resuming work on disturbed subgrade soils.

Subgrade Protection and Inspection: The majority of the site soils are composed of fine-grained materials that are highly moisture sensitive. Every effort should be made to minimize disturbance of the on-site materials by construction traffic and surface runoff. The on-site soils will deteriorate when subjected to repeated wetting and construction traffic and likely will require extensive drying or overexcavation and replacement. Construction schedules and budgets should account for contingencies, such as importing materials to raise grades or restore overexcavations when construction must occur following wet weather or on an expedited basis. However, if properly protected and maintained during warm, dry weather as recommended herein, the site soils will provide adequate support for the proposed construction. The site contractors should employ necessary means and methods to protect the subgrade including, but not limited to the following:

- leaving the existing pavement in place as long as practical to protect the subgrade from freeze-thaw cycles and exposure to inclement weather;
- sealing exposed subgrade soils on a daily basis with a smooth drum roller operated in static mode;
- regrading the site as needed to maintain positive drainage away from construction areas;
- removing wet surficial soils and ruts immediately; and
- limiting exposure to construction traffic especially following inclement weather and subgrade thawing.

5.3 STRUCTURAL FILL AND BACKFILL

Imported Fill Material: Any imported material placed as structural fill or backfill to raise elevations or restore design grades should consist of clean, relatively well graded sand or gravel with a maximum particle size of three inches and five percent to 20 percent of material finer than a #200 sieve. Alternatively, inorganic soil types including silty and clayey sands and gravels with higher percentage of fine material and silts and clays with a liquid limit less than 40 and a plasticity index less than 20 may be considered subject to the owner's approval, provided that the required moisture content and compaction controls are met. The material should be free of clay lumps, organics and deleterious material.

On-Site Material/Moisture Sensitivity: Whitestone anticipates that the majority of the existing fill and underlying natural materials will be marginally suitable for selective reuse as structural fill and/or backfill where free of deleterious materials and moisture contents are controlled within two percent of the optimum moisture content. Reuse of the existing fill will be contingent on careful inspection in the field by the owner's geotechnical engineer by visual observation and/or test pit excavations during construction as recommended herein. The reuse of fine-grained soils (USCS: ML and CL) and granular materials with greater than 12 percent fines (USCS: SM) typically requires warm and dry weather conditions. Soil reuse will typically require mixing with a more granular material and/or drying. The on-site soils will become increasingly difficult to reuse and compact where wetted beyond the optimum moisture content. Immediate re-use of on-site soil should not be expected.

Cobble- and boulder-sized weathered rock/bedrock materials or similarly sized materials greater than three inches in diameter will need to be separated from on-site soils to be placed as structural fill or backfill. Cobble-sized materials between three inches to six inches may be crushed or individually placed in structural fill or backfill layers deeper than two feet below proposed foundation and pavement subgraded levels. Care must be taken to individually seat any large particles and to compact soil around large particles with hand operated equipment to minimize risk of void formation. Boulder-sized greater than 12 inches in diameter need to be crushed prior to replacement as structural fill materials. Materials greater than three inches in size should be placed a minimum of three feet from utilities.

Materials that become exceedingly wet likely will require discing and aerating which may not be practical during wet seasons. Alternatively, imported fill materials may be used to attain the desired grades and expedite earthwork operations. The stripped asphalt and topsoil should not be used as fill or backfill.

Submerged Fill: Where necessary, up to two feet of an open-graded, crushed, three-quarter inch stone may be placed in the wet to provide a working mat, expedite dewatering efforts and enable subsequent placement of structural fill or backfill in the dry. Prior to placing submerged fill materials, free water and disturbed materials should be removed to the extent recommended by the geotechnical engineer. A separation geotextile, such as Mirafi 140N or equivalent, should be placed at the base and sides of the overexcavation to separate the stone from underlying and adjacent soils. The fabric also should be placed on top of the stone prior to subsequent fill placement if fill soils with a substantial amount of fines are to be used to restore grade.

Demolition Material: Demolition material, free of environmental restrictions, may be used as fill material provided the material is properly segregated and processed as recommended herein. Concrete masonry materials should be crushed to a well graded blend with a maximum size of three inches in diameter. Stripped asphaltic materials and deleterious building materials such as wood, insulation, metal shingles etc. should not be used as general structural fill material. Milled or reclaimed asphalt pavement (RAP) may be re-used as granular base or stabilizing materials provided that the RAP particle size meets New York State Department of Transportation (NYSDOT) standard specifications for granular base and no more than 50 percent of the pavement granular base contains RAP.

Compaction and Placement Requirements: All fill and backfill should be placed in maximum eight-inch loose lifts and compacted to 95 percent of the maximum dry density within two percent of the optimum moisture content as determined by ASTM D 1557 (Modified Proctor) unless otherwise recommended in subsequent sections of this report. Whitestone recommends using a vibratory drum roller to compact the on-site soils or a small hand-held vibratory compactor within excavations.

Structural Fill Testing: A sample of the imported fill material and on-site materials to be re-used should be submitted to the geotechnical engineer for analysis and approval prior to use. The placement of all fill and backfill should be monitored by a qualified engineering technician to ensure that the specified material and lift thicknesses are properly installed. A sufficient number of in-place density tests (methods ASTM D 6938 or ASTM D 1556) should be conducted on each lift to ensure that the specified compaction is achieved throughout the height of the fill or backfill.

5.4 GROUNDWATER CONTROL

Static groundwater was encountered at depths ranging from approximately 11 fbgs to 14 fbgs. Based on anticipated final grades, static groundwater levels are anticipated to be deeper than proposed site excavations. However, trapped/perched water may be expected to be encountered within the existing fill, at the existing fill natural soil interface, and/or within finer-grained layers of the natural site soils, especially following precipitation events. As such, construction phase dewatering of trapped/perched water through the use of gravity fed sump pumps should be anticipated during excavation activities for this site.

Because the subsurface soils will soften when exposed to water, every effort must be made to maintain drainage of surface water runoff away from construction areas by grading and limiting the exposure of excavations to rainfall. Overexcavation of saturated soils and replacement with controlled structural fill and/or one foot to two feet of open graded gravel (such as 3/4 inch clean crushed stone) may be required prior to resuming work on disturbed subgrade soils.

5.5 FOUNDATIONS

Shallow Foundation Design Criteria: Whitestone recommends supporting the proposed structures on conventional spread and continuous wall footings designed to bear within the underlying improved natural soils or controlled structural fill provided these materials are properly evaluated, placed, and compacted in accordance with Sections 5.2, 5.3, and 5.11 of this report. The upper natural residual soils were relatively loose/soft and will require improvement via in-place compaction prior to structural support. Areas of overexcavation and recompaction, particularly within the proposed building addition footprint, also should be anticipated. Foundations bearing within the improved natural residual soils and/or controlled structural fill materials may be designed using a maximum allowable net bearing pressure of 2,500 psf.

Reuse of the existing fill for foundation support will be contingent upon supplemental evaluation, as described in Section 5.11. All footing bottoms should be improved by in-trench compaction in the presence of the geotechnical engineer. Regardless of loading conditions, proposed foundations should be sized no less than minimum dimensions of 24 inches for continuous wall footings and 36 inches for isolated column footings.

Footings subject to overturning moments should be designed such that the maximum toe pressure due to the combined effect of vertical loads and overturning moment does not exceed the recommended maximum allowable net bearing pressure. In addition, positive contact pressure should be maintained throughout the base of the footings such that no uplift or tension exists between the base of the footings and the supporting soil. Uplift loads should be resisted by the weight of the concrete. Side friction should be neglected when proportioning the footings such that lateral resistance should be provided by friction resistance at the base of the footings. A coefficient of friction against sliding of 0.35 is recommended for use in the design of the foundations bearing within the existing site soils or imported structural fill soils.

Foundation Inspection/Overexcavation Criteria: Whitestone recommends that the suitability of the bearing soils along and below the foundation bottoms be verified by a geotechnical engineer prior to placing concrete. In the event that areas of unsuitable materials are encountered, such as loose/soft soils, overexcavation and replacement of the materials will be necessary to provide a suitable footing subgrade. Any overexcavation to be restored with structural fill will need to extend at least one foot laterally beyond footing edges for each vertical foot of overexcavation. Lateral overexcavation may be reduced if grade is restored with lean concrete. The bottom of overexcavations should be compacted with vibrating plates or plate tampers (“jumping jacks”) to compact locally disturbed materials.

Settlement: Whitestone estimates post construction settlements of proposed building foundations to be less than one inch if the recommendations outlined in this report are properly implemented. Differential settlement of building foundations should be less than one-half inch.

Frost Coverage: Footings subject to frost action should be placed at least 42 inches below adjacent exterior grades or the depth required by local building codes to provide protection from frost penetration. Interior footings not subject to frost action may be placed at a minimum depth of 18 inches below the slab subgrade.

5.6 FLOOR SLABS

Following supplemental evaluation of the existing fill and surficial compaction of the upper site soils to densify any loose areas, Whitestone anticipates that improved and approved existing fill, the underlying improved natural site soils, and/or controlled structural fill will be suitable for support of the proposed floor slabs provided these materials are properly evaluated, compacted, and proofrolled in accordance with Sections 5.2, 5.3, and 5.11 of this report. Areas of overexcavation and replacement or recompaction of loose/soft existing fill and natural soils should be anticipated. Any areas that become softened or disturbed as a result of wetting and/or repeated exposure to construction traffic should be removed and replaced with compacted structural fill. The properly prepared on-site soils are expected to yield a minimum subgrade modulus (k) of 150 psi/in.

A minimum four-inch layer of coarse aggregate, such as AASHTO #57 stone, dense graded aggregate, or equal, should be installed below ground-supported floor slabs to provide a capillary break. An impervious membrane also should be provided as a moisture vapor barrier beneath all floor slabs.

5.7 PAVEMENT DESIGN CRITERIA

General: Following supplemental evaluation of the existing fill and surficial compaction of the upper site soils to densify any loose areas, Whitestone anticipates that the improved and approved existing fill, underlying improved natural soils, and/or compacted structural fill and/or backfill placed to raise or restore design elevations are expected to be suitable for support of the proposed pavements provided these materials are properly evaluated, compacted, and proofrolled in accordance with Sections 5.2, 5.3, and 5.11 of this report during favorable weather conditions. Localized overexcavation and replacement of existing fill and loose/soft natural soils should be anticipated.

Alternatively, subgrade stabilization with a geogrid, such as Tensar TX-160 or equal, should be anticipated to limit overexcavation. A separation geosynthetic, such as a US 200 woven geotextile or equal, should also be provided where subgrade soils are fine-grained. Where existing fill and loose/soft natural site soils remain below proposed subgrades, increased maintenance, possibly including crack sealing, patching or more frequent re-paving, may be necessary. If the risk of increased maintenance is not acceptable, more extensive subgrade preparation recommendations can be developed. The following pavement section recommendations are based on the assumption that such an increased risk is acceptable. Whitestone would be pleased to prepare alternative recommendations for the more substantial subgrade improvements.

Design Criteria: A California Bearing Ratio value of five has been assigned to the properly prepared subgrade soils for pavement design purposes based on laboratory test results and climatic factors. This value was correlated with pertinent soil support values and assumed traffic loads to prepare flexible and rigid pavement designs per the AASHTO *Guide for the Design of Pavement Structures*.

Design traffic loads were assumed based on typical volumes for similar facilities and correlated with 18-kip equivalent single axle loads (ESAL) for a 20-year life. An estimated maximum load of 25,000 ESAL was used for all pavement areas assuming the pavement primarily will accommodate both automobile and limited heavier truck traffic. Actual pavement loads should be less than this value.

Pavement Sections: The recommended flexible pavement section is presented below:

FLEXIBLE PAVEMENT SECTION		
Layer	**Material**	**Standard Duty Thickness (Inches)**
Asphalt Surface	NYSDOT Type 7 or 7F Top	1.5
Asphalt Base	NYSDOT Type 3 Binder	2.5
Granular Subbase	NYSDOT Type 2 Subbase	6.0

A rigid concrete pavement should be used to provide suitable support at areas of high traffic or severe turns (such as at ingress/egress areas). The recommended rigid pavement is presented below in tabular format:

RIGID PAVEMENT SECTIONS		
Layer	**Material**	**Standard Duty Thickness (Inches)**
Surface	4,000 psi air-entrained concrete	5.0[1]
Base	NYSDOT Type 2 Subbase	6.0

Note[1]: The outer edges of concrete pavements are susceptible to damage as trucks move from rigid pavement to adjacent flexible pavement. Therefore, the thickness at the outer two feet of the rigid concrete pavement should be 12 inches.

Additional Design Considerations: The pavement section thickness designs presented in this report are based on the design parameters detailed herein and are contingent on proper construction, inspection, and maintenance. Additional thickness may be required by local code. The designs are contingent on achieving the minimum soil support value in the field. To accomplish this requirement, all subgrade soil and supporting fill or backfill must be placed, compacted, and evaluated in accordance with Sections 5.2, 5.3, and 5.11 of this report.

The performance of the pavement also will depend on the quality of materials and workmanship. Whitestone recommends that NYSDOT standards for materials, workmanship, and maintenance be applied to this site. Project specifications should include verifying that the installed asphaltic concrete material

composition is within tolerance for the specified materials and that the percentage of air voids of the installed pavement is within specified ranges for the respective materials. All rigid concrete pavements should be suitably air-entrained, jointed, and reinforced.

5.8 RETAINING WALLS/LATERAL EARTH PRESSURES

General: The proposed redevelopment will include new retaining walls. While the design and investigation of the retaining structures are beyond Whitestone's current scope of work, Whitestone would be pleased to assist with the calculation of preliminary lateral earth pressures based on the soil parameters presented herein during the structural design phase when final grading and wall geometries are available.

Lateral Earth Pressure: Temporary retaining structures and permanent below-grade walls may be required to resist lateral earth pressures. Proposed below-grade walls must be capable of withstanding active and at-rest earth pressures. Below-grade walls free to rotate generally can be designed to resist active earth pressures. Below-grade walls corners and restrained walls need to be designed to resist at-rest earth pressures. Such structures should be properly designed by the Owner's engineer. The following soil parameters apply to the encountered subsurface strata and may be used for design of the proposed temporary and permanent retaining structures.

LATERAL EARTH PRESSURE PARAMETERS		
Parameter	**On-Site Soils**	**Imported Granular Backfill**
Moist Density (γ_{moist})	140 pcf	140 pcf
Internal Friction Angle (φ)	28°	30°
Active Earth Pressure Coefficient (K_a)	0.36	0.33
Passive Earth Pressure Coefficient (K_p)	2.77	3.00
At-Rest Earth Pressure Coefficient (K_o)	0.53	0.50

Lateral earth pressure will depend on the backfill slope angle and the wall batter angle. A sloped backfill will add surcharge load and affect the angle of the resultant force. The effect of other surcharges will also need to be included in earth pressure calculations, including the loads imposed by adjacent structures and traffic. The effects of proposed sloped backfill surface grades, and proposed slopes beyond the toe of the retaining structure, if applicable, must be considered when calculating resultant forces to be resisted by the retaining structure. A coefficient of friction of 0.35 against sliding can be used for concrete on the existing site soils. Below-grade wall footings should be designed such that the combined effect of vertical and horizontal resultants and overturning moment does not exceed the maximum soil bearing capacity provided in Section 5.5.

Backfill Criteria: Whitestone recommends that granular soils be used to backfill behind the proposed below-grade walls. The granular backfill materials should consist of clean, relatively well graded sand or gravel with a maximum particle size of three inches and five percent to 15 percent of material finer than a #200 sieve. The material should be free of clay lumps, organics, and deleterious material. The site soils and weathered rock/bedrock are not anticipated to be satisfactory for below-grade wall backfill unless approved by the wall designer. Accordingly, imported granular soils will likely be required. A maximum density of 140 pcf should not be exceeded to avoid creating excessive lateral pressure on the walls during compaction operations.

Whitestone recommends that backfill directly behind any walls be compacted with light, hand-held compactors. Heavy compactors and grading equipment should not be allowed to operate within a zone of influence measured at a 45-degree angle from the base of the walls during backfilling to avoid developing excessive temporary or long-term lateral soil pressures.

Wall Drainage: Positive gravity drainage of the backfill should be provided at the base of the below-grade walls by a series of perforated pipes surrounded by at least 18 inches of clean crushed stone that discharges into a stormwater sewer or daylights to appropriate site surface drainage. Whitestone recommends that a two-foot wide zone of clean crushed stone or washed sand, separated from the backfill by a filter fabric, be constructed adjacent to the back of the wall. This zone should prevent the buildup of hydrostatic pressures and pressures from freezing moisture in the backfill above the groundwater level. The vertical drain should be tied into the gravity drainage system (perforated pipe) installed at the base of the wall. Alternatively, below-grade walls may include weep holes instead of a drain tied to the site drainage system. Where wall drainage is not provided, the wall should be designed to withstand full hydrostatic pressure.

Whitestone should be notified if any other retaining structures or design considerations requiring lateral earth pressure estimations are proposed. Specific recommendations for temporary retaining structures are beyond Whitestone's scope of work.

5.9 SEISMIC AND LIQUEFACTION CONSIDERATIONS

The soils encountered during this investigation are most consistent with a Site Class C defined by the *New York State International Building Code (2020).* Based on the seismic zone and soil profile, liquefaction considerations are not expected to have a substantial impact on design.

5.10 EXCAVATIONS

The soils encountered during this investigation within anticipated excavation depths are consistent with Type C Soil Conditions as defined by 29 CFR Part 1926 (OSHA) which require a maximum unbraced excavation angle of 1.5:1 (horizontal:vertical). Actual conditions encountered during construction should be evaluated by a competent person (as defined by OSHA) to ensure that safe excavation methods and/or shoring and bracing requirements are implemented.

5.11 SUPPLEMENTAL POST INVESTIGATION SERVICES

Construction Phase Evaluation of Existing Fill: Based on the conditions disclosed by the subsurface tests, Whitestone anticipates that the majority of the existing fill encountered throughout the site is anticipated to be suitable for floor slab and pavement support following supplemental evaluation and compaction improvement with some anticipated overexcavation, due to the possible variability within existing fill, and with increased risk of future maintenance within proposed pavement areas where marginal unimproved existing fill remains. Whitestone also anticipates that the majority of the existing fill will be marginally suitable for selective reuse as structural fill where free of deleterious debris and implementation of moisture control operations are utilized. Reuse of the existing fill will be contingent on careful inspection in the field by the owner's geotechnical engineer by visual observation and/or test pit excavations during construction as recommended herein. Due to the inherent variability that exists within existing fill, Whitestone recommends confirming further the condition of the existing fill for floor slab and pavement support and/or reuse as structural fill by means of supplemental evaluation either prior to or during the early stages of construction, as discussed further herein, to identify areas requiring removal and possible uncontrolled conditions or deleterious materials not disclosed by the subsurface tests conducted during this exploration.

Demolition and Construction Inspection and Monitoring: The owner's geotechnical engineer should conduct inspection, testing, and consultation during construction as described in previous sections of this report. Monitoring and testing should also be conducted to verify that any encountered underground structures are properly backfilled, the existing surface cover materials are properly removed, and suitable materials used for controlled fill are properly placed and compacted over suitable subgrade soils. The improvement/overexcavation of loose/soft site soils (where required) and proofrolling of all subgrades prior to foundation, floor slab, and pavement support should be witnessed and documented by the owner's geotechnical engineer.

SECTION 6.0
General Comments

Supplemental recommendations may be required upon finalization of construction plans or if significant changes are made in the characteristics or location of the proposed structures. Soil bearing conditions should be checked at the appropriate time for consistency with those conditions encountered during Whitestone's geotechnical investigation.

The possibility exists that conditions between borings may differ from those at specific test locations, and conditions may not be as anticipated by the designers or contractors. In addition, the construction process may alter soil and rock conditions. Therefore, experienced geotechnical personnel should observe and document the construction procedures used and the conditions encountered.

The recommendations presented herein should be utilized by a qualified engineer in preparing the project plans and specifications. The engineer should consider these recommendations as minimum physical standards which may be superseded by local and regional building codes and structural considerations. These recommendations are prepared for the sole use of MEMASI for the specific project detailed and should not be used by any third party. These recommendations are relevant to the design phase and should not be substituted for construction specifications.

Whitestone assumes that a qualified contractor will be employed to conduct the construction work, and that the contractor will be required to exercise care to ensure all excavations are conducted in accordance with applicable regulations and good practice. Particular attention should be paid to avoiding damaging or undermining adjacent properties and maintaining slope stability.

Whitestone recommends that the services of the geotechnical engineer be engaged to test and evaluate the soils in the footing excavations prior to concreting in order to determine that the soils will support the bearing capacities. Monitoring and testing also should be conducted to verify that suitable materials are used for controlled fills and that they are properly placed and compacted over suitable subgrade soils.

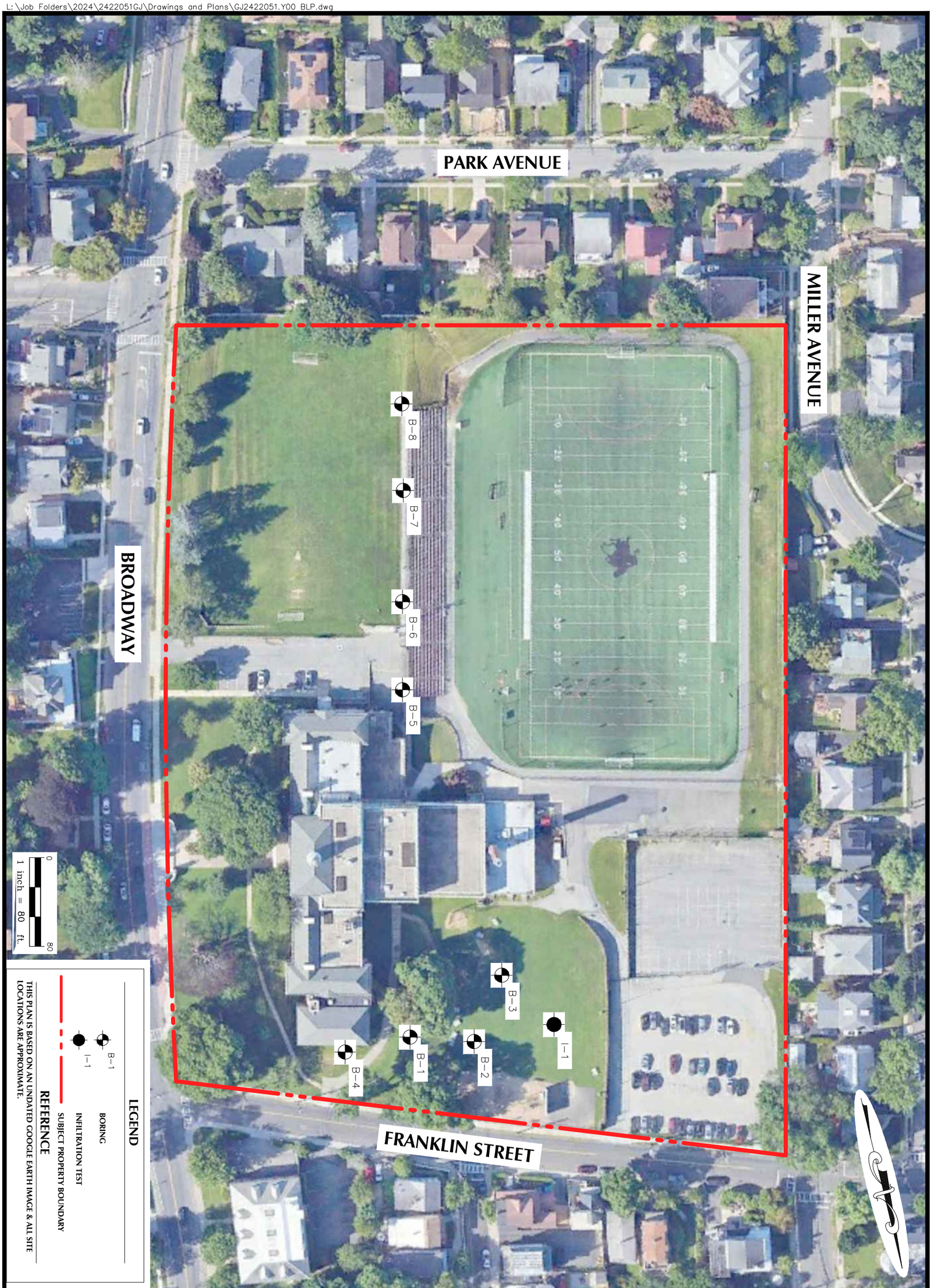
The exploration and analysis of the foundation conditions reported herein are considered sufficient in detail and scope to form a reasonable basis for the foundation design. The recommendations submitted for the proposed construction are based on the available soil information and the preliminary design details furnished by MEMASI. If deviations from the noted subsurface conditions are encountered during construction, they should be brought to the attention of the geotechnical engineer.

The geotechnical engineer warrants that the findings, recommendations, specifications, or professional advice contained herein have been promulgated after being prepared in accordance with generally accepted professional engineering practice in the fields of foundation engineering, soil mechanics, and engineering geology. No other warranties are implied or expressed

WHITESTONE

FIGURE 1
Test Location Plan

L:\Job Folders\2024\2422051GJ\Drawings and Plans\GJ2422051.Y00 BLP.dwg



PROJECT #: GJ2422051.Y00
DESIGNED BY: GR
DATE: 9/30/24
SCALE: 1" = 80'
PROJ. MGR.: MK
FIGURE: 1

DRAWING TITLE:
TEST LOCATION PLAN

CLIENT:
MEMASI

PROJECT:
WASHINGTON IRVING INTERMEDIATE SCHOOL
PROPOSED ADDITION & SITE IMPROVEMENTS
103 SOUTH BROADWAY
TARRYTOWN, WESTCHESTER COUNTY, NY

WHITESTONE
30 INDEPENDENCE BOULEVARD, SUITE 250, WARREN, NJ 07059
908.668.7777 WHITESTONEASSOC.COM

WHITESTONE

APPENDIX A
Records of Subsurface Exploration

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-1

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/31/2024		**Water Depth (feet bgs) | Elevation (feet)**		**Cave-In Depth (feet bgs) | Elevation (feet)**
Termination Depth:	12.0 feet bgs	**Date Completed:**	7/31/2024				
Proposed Location:	Addition	**Logged By:**	CS	**During:**	11.0 | ---		
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:**	NE | ---	**At Completion:**	8.0 | ---
		Equipment:	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
						0.5	TOPSOIL		6" Topsoil	
0 - 2	S-1		2 - 4 - 4 - 3	12	8		RESIDUAL		Brown Silty Sand with Gravel, Moist, Loose (SM)	
2 - 4	S-2		3 - 6 - 4 - 4	12	10				As Above, Medium Dense (SM)	
						4.0				
4 - 6	S-3		3 - 4 - 3 - 4	18	7	5.0			Brown Sandy Silt, Moist, Stiff (ML)	Qu = 1.0 tsf
6 - 8	S-4		4 - 4 - 3 - 3	18	7				As Above, Medium Stiff (ML)	Qu = 0.75 tsf
8 - 10	S-5		3 - 2 - 2 - 3	10	4				As Above (ML)	Qu = 0.75 tsf
						10.0				
10 - 11.9	S-6		1 - 2 - 2 - 50/5"	10	4				As Above, Wet (ML)	
						12.0				
									Boring Log B-1 Terminated at a Depth of 12.0 Feet Below Ground Surface Due to Auger Refusal	
						15.0				
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-2

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/31/2024		**Water Depth | Elevation (feet bgs) | (feet)**		**Cave-In Depth | Elevation (feet bgs) | (feet)**
Termination Depth:	23.5 feet bgs	**Date Completed:**	7/31/2024				
Proposed Location:	Addition	**Logged By:**	CS	**During:**	12.0 | ---		
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:**	12.0 | ---	**At Completion:**	23.0 | ---
		Equipment:	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
						0.4	TOPSOIL		5" Topsoil	
0 - 2	S-1		9 - 8 - 7 - 7	14	15		RESIDUAL		Brown Silty Sand with Gravel, Moist to Dry, Medium Dense (SM)	
2 - 4	S-2		5 - 5 - 7 - 6	12	13				As Above (SM)	
4 - 6	S-3		4 - 4 - 12 - 10	12	16	5.0			As Above (SM)	
6 - 8	S-4		3 - 3 - 4 - 3	16	7	6.0			Brown Lean Clay, Moist, Stiff (CL)	Qu = 0.5 tsf
8 - 10	S-5		8 - 5 - 4 - 5	16	9				As Above (CL)	Qu = 0.5 tsf
10 - 12	S-6		20 - 48 - 24 - 22	18	72	10.0			Brown Silty Sand with Gravel, Moist, Very Dense (SM)	
12 - 14	S-7		12 - 12 - 10 - 12	14	22				As Above , Wet, Medium Dense (SM)	
14 - 16	S-8		7 - 6 - 8 - 8	20	14	14.0			Brown Lean Clay, Wet, Stiff (CL)	Qu = 1.5 tsf
16 - 18	S-9		6 - 22 - 21 - 24	14	43	15.0			As Above, with Gravel (CL)	
18 - 20	S-10		25 - 32 - 22 - 21	18	54	18.0			Brown Silty Sand, Wet, Very Dense (SM)	
20 - 22	S-11		15 - 32 - 21 - 42	14	53	20.0			As Above (SM)	
22 - 23.3	S-12		28 - 48 - 50/4"	14	50/4"	23.5			As Above (SM)	
						25.0			Boring Log B-2 Terminated at a Depth of 23.5 Feet Below Ground Surface Due to Auger Refusal	

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-3

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/31/2024		**Water Depth | Elevation (feet bgs) | (feet)**		**Cave-In Depth | Elevation (feet bgs) | (feet)**
Termination Depth:	16.5 feet bgs	**Date Completed:**	7/31/2024				
Proposed Location:	Addition	**Logged By:**	CS	**During:**	NE | ---		
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:**	NE | ---	**At Completion:**	13.0 | ---
		Equipment:	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
						0.7	TOPSOIL		6" Topsoil	
0 - 2	S-1		4 - 6 - 7 - 10	12	13		RESIDUAL		Brown Sandy Silt, Dry, Medium Dense (ML)	
2 - 4	S-2		2 - 2 - 3 - 2	8	5				As Above, Moist, Loose (ML)	
4 - 6	S-3		3 - 2 - 3 - 2	8	5	5.0			As Above (ML)	
6 - 8	S-4		2 - 16 - 22 - 20	16	38				As Above, with Gravel, Dense (ML)	
8 - 10	S-5		12 - 20 - 25 - 22	10	45				As Above (ML)	
						10.0				
10 - 12	S-6		20 - 18 - 25 - 17	12	43				As Above (ML)	
12 - 14	S-7		8 - 16 - 18 - 21	14	34				As Above (ML)	
14 - 16	S-8		15 - 20 - 27 - 20	14	47	15.0			As Above, Whitish-Brown, Moist (ML)	
16 - 16.3	S-9		50/3"	NR	50/3"	16.5			No Recovery, Assumed As Above (ML)	
									Boring Log B-3 Terminated at a Depth of 16.5 Feet Below Ground Surface Due to Auger Refusal	
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-4

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

				Water Depth | Elevation (feet bgs) | (feet)	Cave-In Depth | Elevation (feet bgs) | (feet)
Surface Elevation:	± NS feet	**Date Started:**	7/30/2024		
Termination Depth:	17.0 feet bgs	**Date Completed:**	7/30/2024		
Proposed Location:	Addition	**Logged By:**	CS	**During:** NE | ---	
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:** 14.0 | ---	**At Completion:** 14.0 | ---
		Equipment:	ATV	**24 Hours:** --- | ---	**24 Hours:** --- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
						0.5	TOPSOIL		6" Topsoil	
0 - 2	S-1		5 - 39 - 9 - 4	6	48		FILL		Brown Silty Sand with Gravel, Moist (FILL)	
						2.0				
2 - 4	S-2		8 - 9 - 8 - 6	10	17		RESIDUAL		Brown Sandy Silt, Moist, Medium Dense (ML)	
4 - 6	S-3		4 - 2 - 2 - 2	14	4	5.0			As Above, Loose (ML)	
6 - 8	S-4		2 - 2 - 2 - 2	10	4				As Above (ML)	
						8.0				
8 - 10	S-5		2 - 2 - 2 - 2	16	4				Brown Lean Clay, Moist, Stiff (CL)	Qu = 0.5 tsf
						10.0				
10 - 12	S-6		2 - 2 - 2 - 2	18	4				As Above (CL)	
						13.0				
13 - 15	S-7		7 - 19 - 31 - 23	16	50				Brownish-White Silty Sand with Gravel, Very Moist, Very Dense (SM)	
						15.0				
15 - 17	S-8		22 - 28 - 31 - 50/5"	12	59				As Above, Brown (SM)	
						17.0				
									Boring Log B-4 Terminated at a Depth of 17.0 Feet Below Ground Surface Due to Auger Refusal	
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-5

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

				Water Depth | Elevation (feet bgs) | (feet)	Cave-In Depth | Elevation (feet bgs) | (feet)
Surface Elevation:	± NS feet	**Date Started:**	7/30/2024		
Termination Depth:	15.0 feet bgs	**Date Completed:**	7/30/2024		
Proposed Location:	Grandstone	**Logged By:**	CS	**During:** NE | ---	
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:** NE | ---	**At Completion:** 8.0 | ---
		Equipment:	ATV	**24 Hours:** --- | ---	**24 Hours:** --- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
							PAVEMENT		3" Asphalt, No Apparent Subbase	
						0.3	FILL		Gray Silty Sand with Gravel, Moist (FILL)	
0 - 2	S-1	X	20 - 12 - 7 - 7	4	19					
						2.0	RESIDUAL			
2 - 4	S-2	X	7 - 5 - 12 - 13	8	17				Brown Silty Sand, Moist, Medium Dense (SM)	
4 - 6	S-3	X	5 - 6 - 4 - 4	8	10	5.0			As Above (SM)	
6 - 8	S-4	X	2 - 2 - 2 - 2	4	4				Brown Silty Sand with Gravel, Moist, Loose (SM)	
8 - 10	S-5	X	2 - 3 - 4 - 4	10	7				As Above (SM)	
						10.0				
10 - 12	S-6	X	4 - 10 - 9 - 8	12	19				As Above, Medium Dense (SM)	
						13.0				
13 - 13.3	S-7	X	50/3"	2	50/3"		WEATHERED ROCK		Brown Weathered Rock, Moist, Very Dense (WR)	
						15.0				
									Boring Log B-5 Terminated at a Depth of 15.0 Feet Below Ground Surface	
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-6

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/29/2024		**Water Depth (feet bgs) | Elevation (feet)**		**Cave-In Depth (feet bgs) | Elevation (feet)**
Termination Depth:	24.0 feet bgs	**Date Completed:**	7/29/2024				
Proposed Location:	Grandstone	**Logged By:**	CS	**During:**	NE | ---		
Drill / Test Method:	HSA / SPT /	**Contractor:**	MR	**At Completion:**	NE | ---	**At Completion:**	13.0 | ---
	ROCK CORE	**Equipment:**	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
0 - 2	S-1		2 - 5 - 7 - 9	18	12		RESIDUAL		Brown Silty Sand, Moist, Loose (SM)	
2 - 4	S-2		8 - 7 - 4 - 2	12	11	4.0			As Above, with Gravel (SM)	
4 - 6	S-3		13 - 34 - 13 - 7	6	47	5.0 6.0			Brown Lean Clay, Moist (CL)	
6 - 8	S-4		4 - 16 - 18 - 18	18	34				Brown Silty Sand, with Gravel, Moist, Dense (SM)	
8 - 10	S-5		18 - 47 - 40 - 33	12	87				As Above, Very Dense (SM)	
10 - 12	S-6		21 - 23 - 25 - 17	12	48	10.0			Reddish-Brown Silty Sand with Gravel, Wet, Dense (SM)	
						13.0				
13 - 13.8	S-7		29 - 50/3"	4	50/3"		WEATHERED ROCK		Dark Brown Weathered Rock, Moist, Very Dense (WR)	
						15.0				
						18.0				
18 - 19	S-8		8 - 15 - 50/0"	8	65/6"	19.0				
	Total Elapsed Cut Time/Cut Time Per Ft.			REC	RQD		BEDROCK		Whitish-Gray Gneiss, Moderately to Slightly Weathered, No Apparent Binding	
			2:15 / 2:15			20.0				
			2:33 / 4:48							
19 - 24	R1	NQ	7:41 / 12:29	60" 100%	20" 33%					
			3:35 / 16:04							
			5:04 / 21:08			24.0				
						25.0			Boring Log B-6 Terminated at a Depth of 24.0 Feet Below Ground Surface	

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-7

Page 1 of 1

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/29/2024		**Water Depth | Elevation (feet bgs) | (feet)**		**Cave-In Depth | Elevation (feet bgs) | (feet)**
Termination Depth:	12.0 feet bgs	**Date Completed:**	7/29/2024				
Proposed Location:	Grandstone	**Logged By:**	CS	**During:**	NE | ---		
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:**	NE | ---	**At Completion:**	8.0 | ---
		Equipment:	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
0 - 2	S-1		2 - 6 - 12 - 8	20	18	1.0	TOPSOIL		12" Topsoil	
							RESIDUAL		Brown Silty Sand, Moist to Dry, Medium Dense (SM)	
2 - 4	S-2		6 - 15 - 10 - 8	14	25	4.0			As Above, Dense (SM)	
4 - 6	S-3		5 - 3 - 2 - 2	14	5	5.0			Brown Silt with Sand, Moist to Dry, Medium Stiff (ML)	Qu = 0.75 tsf
6 - 8	S-4		3 - 4 - 4 - 4	14	8				As Above (ML)	
						8.0				
8 - 10	S-5		15 - 20 - 18 - 18	16	38	10.0			Brownish-Red Silty Sand with Gravel, Moist, Dense (SM)	
10 - 12	S-6		23 - 32 - 43 - 44	12	75	12.0			As Above, Whitish-Brown, Very Dense (SM)	
									Boring Log B-7 Terminated at a Depth of 12.0 Feet Below Ground Surface Due to Auger Refusal	
						15.0				
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-8

Project:	Proposed Addition & Site Improvements	**WAI Project No.:**	GJ2422051.Y00
Location:	103 South Broadway; Tarrytown, Westchester County, NY	**Client:**	MEMASI

Surface Elevation:	± NS feet	**Date Started:**	7/29/2024	**Water Depth (feet bgs) | Elevation (feet)**		**Cave-In Depth (feet bgs) | Elevation (feet)**	
Termination Depth:	16.0 feet bgs	**Date Completed:**	7/29/2024				
Proposed Location:	Grandstone	**Logged By:**	CS	**During:**	NE | ---		
Drill / Test Method:	HSA / SPT	**Contractor:**	MR	**At Completion:**	NE | ---	**At Completion:**	12.0 | ---
		Equipment:	ATV	**24 Hours:**	--- | ---	**24 Hours:**	--- | ---

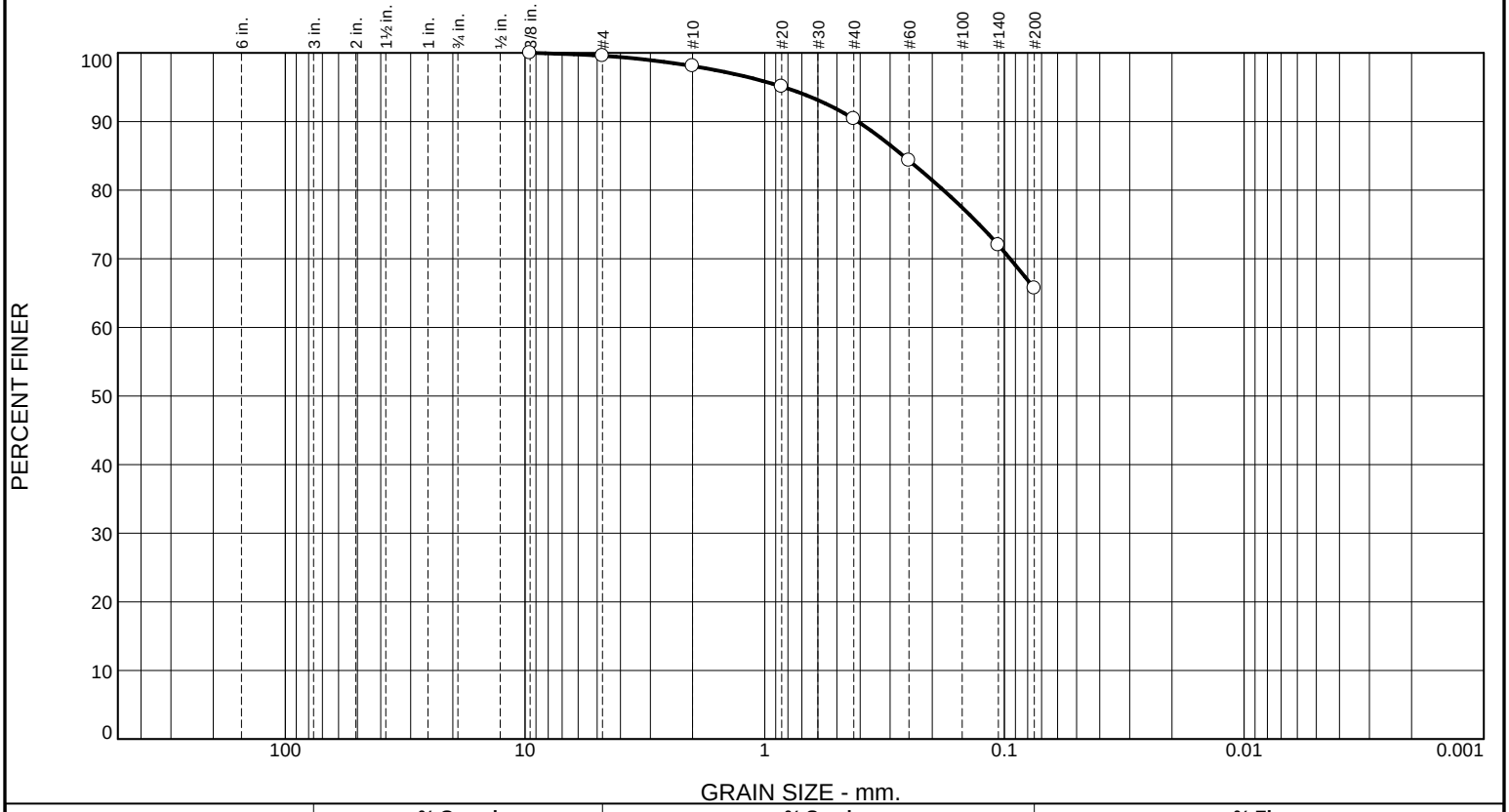
SAMPLE INFORMATION						DEPTH	STRATA		DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)				
						0.0				
						0.5	TOPSOIL		6" Topsoil	
0 - 2	S-1		5 - 5 - 5 - 5	17	10		RESIDUAL		Brown Silty Sand with Gravel, Moist, Medium Dense (SM)	
2 - 4	S-2		5 - 5 - 5 - 3	16	10				As Above (SM)	
						4.0				
4 - 6	S-3		2 - 2 - 5 - 5	16	7	5.0			Dark Brown Sandy Silt, Moist, Loose (ML)	
						6.0				
6 - 8	S-4		3 - 2 - 5 - 6	12	7				Brown Lean Clay, Moist to Dry, Medium Stiff (CL)	Qu = 0.75 tsf
						8.0				
8 - 10	S-5		5 - 10 - 15 - 18	14	25				Brownish-Red Silty Sand with Gravel, Moist, Dense (SM)	
						10.0				
10 - 10.8	S-6		22 - 50/3"	12	50/3"		WEATHERED ROCK		Whitish-Brown Weathered Rock with Silty Sand, Moist, Very Dense (WR)	
						14.0				
14 - 15.3	S-7		23 - 25 - 50/3"	14	50/3"	15.0			As Above (WR)	
						16.0				
									Boring Log B-8 Terminated at a Depth of 16.0 Feet Below Ground Surface Due to Auger Refusal	
						20.0				
						25.0				

NOTES: bgs = below ground surface, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

WHITESTONE

APPENDIX B
Laboratory Test Results

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.4	1.5	7.7	24.7	65.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3/8	100.0		
#4	99.6		
#10	98.1		
#20	95.1		
#40	90.4		
#60	84.3		
#140	72.0		
#200	65.7		

* (no specification provided)

Material Description

Sandy Silt

Atterberg Limits

PL= 24 LL= 25 PI= 1

Coefficients

D_{90}= 0.4089 D_{85}= 0.2642 D_{60}=
D_{50}= D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

W_n = 21.3 %

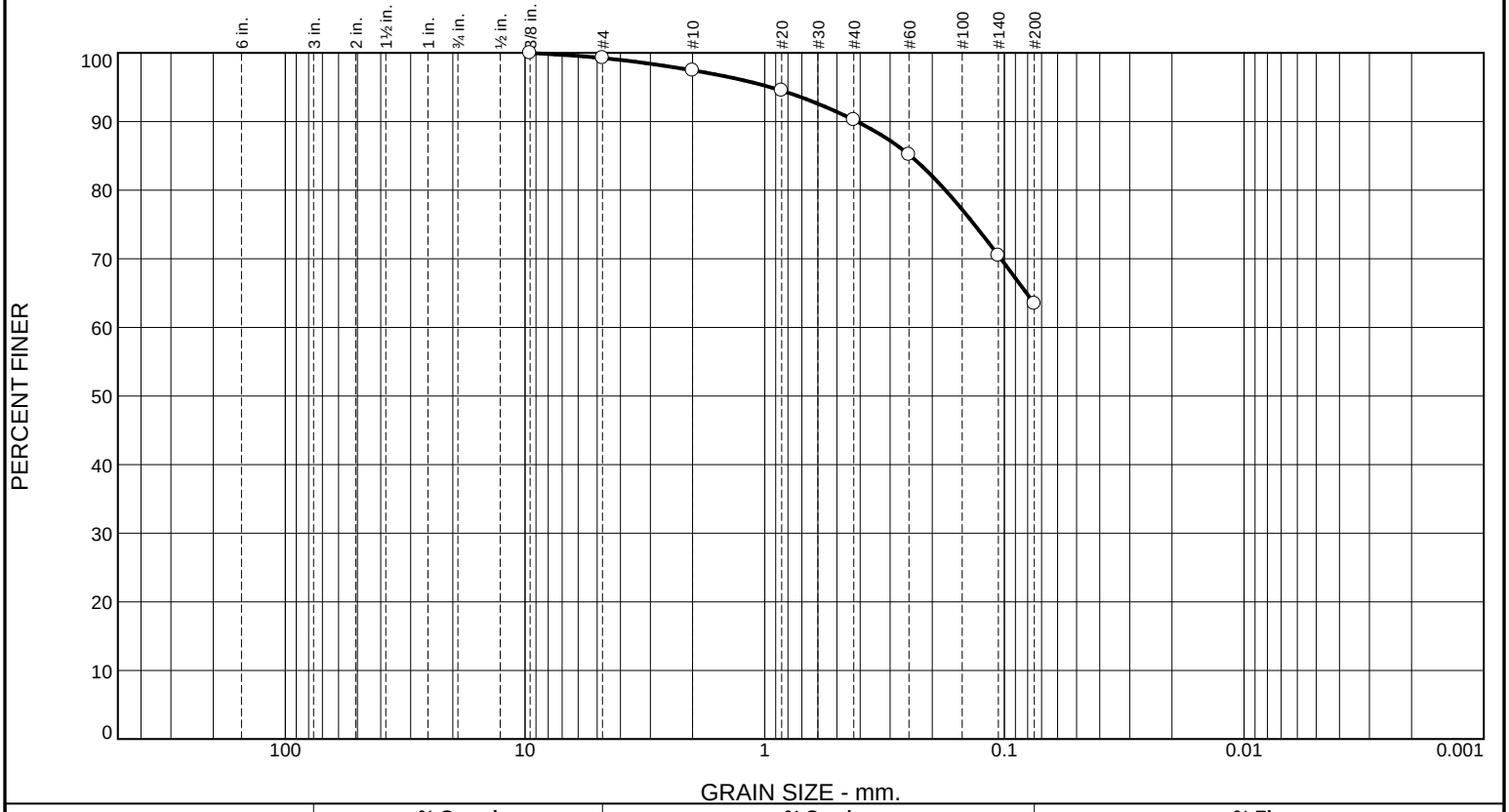
Source of Sample: B-1 **Depth:** 6.0' - 8.0'
Sample Number: S-4

Date: 09/27/2024

WHITESTONE ASSOCIATES, INC.
Warren, New Jersey

Client: MEMASI
Project: Proposed Addition & Site Improvements
103 South Broadway; Tarrytown, Westchester County, NY
Project No: GJ2422051.Y00 **Figure**

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.7	1.8	7.3	26.7	63.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3/8	100.0		
#4	99.3		
#10	97.5		
#20	94.5		
#40	90.2		
#60	85.2		
#140	70.5		
#200	63.5		

* (no specification provided)

Material Description

Sandy Silt

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D_{90}= 0.4127 D_{85}= 0.2465 D_{60}=
D_{50}= D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

W_n = 9.9 %

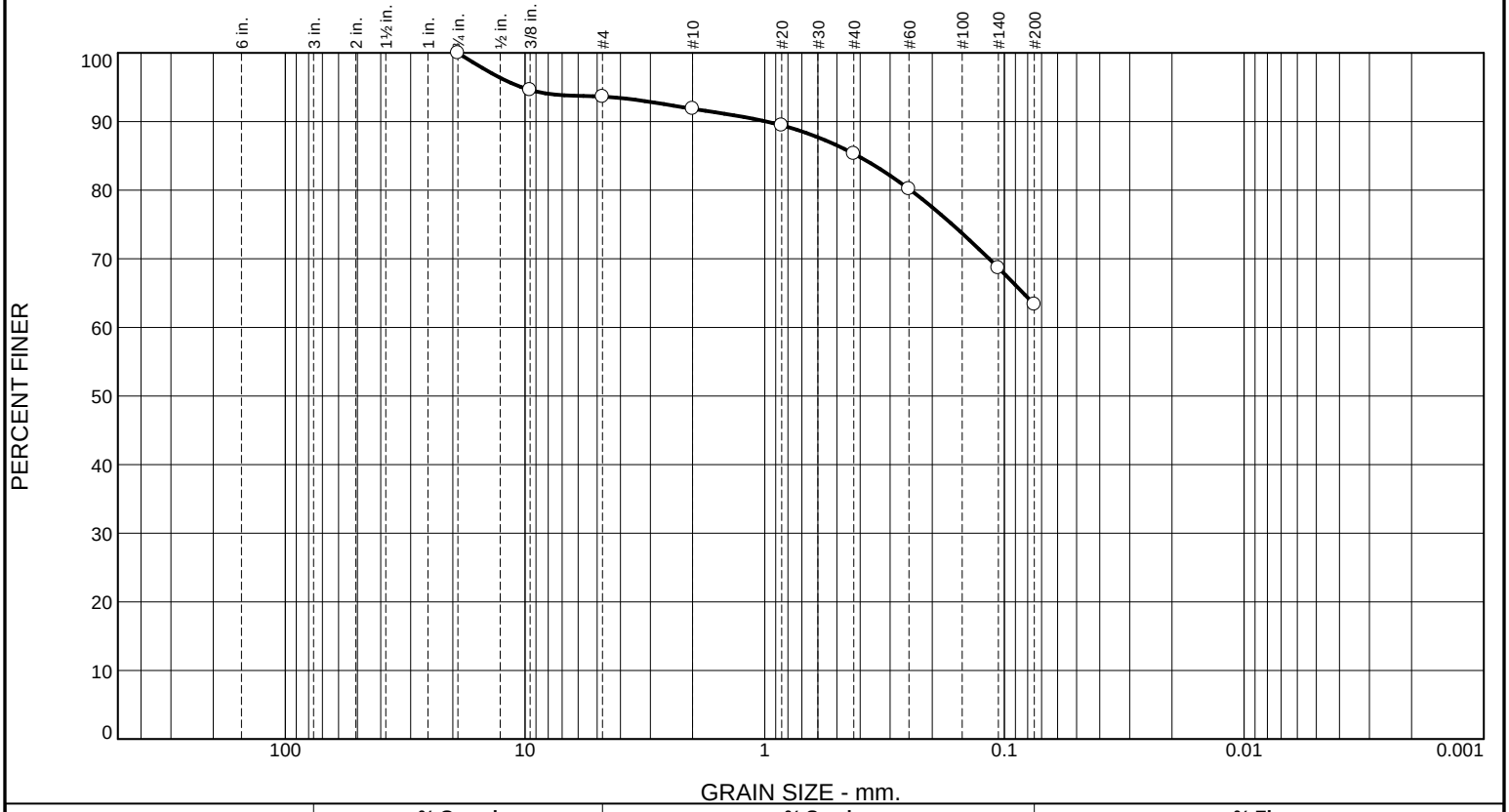
Source of Sample: B-3 **Depth:** 2.0' - 4.0'
Sample Number: S-2

Date: 09/27/2024

WHITESTONE ASSOCIATES, INC.
Warren, New Jersey

Client: MEMASI
Project: Proposed Addition & Site Improvements
103 South Broadway; Tarrytown, Westchester County, NY
Project No: GJ2422051.Y00 **Figure**

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	6.4	1.7	6.6	21.9	63.4	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3/4	100.0		
3/8	94.6		
#4	93.6		
#10	91.9		
#20	89.4		
#40	85.3		
#60	80.2		
#140	68.7		
#200	63.4		

* (no specification provided)

Material Description

Sandy Silt

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D_{90}= 0.9850 D_{85}= 0.4092 D_{60}=
D_{50}= D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

W_n = 11.2 %

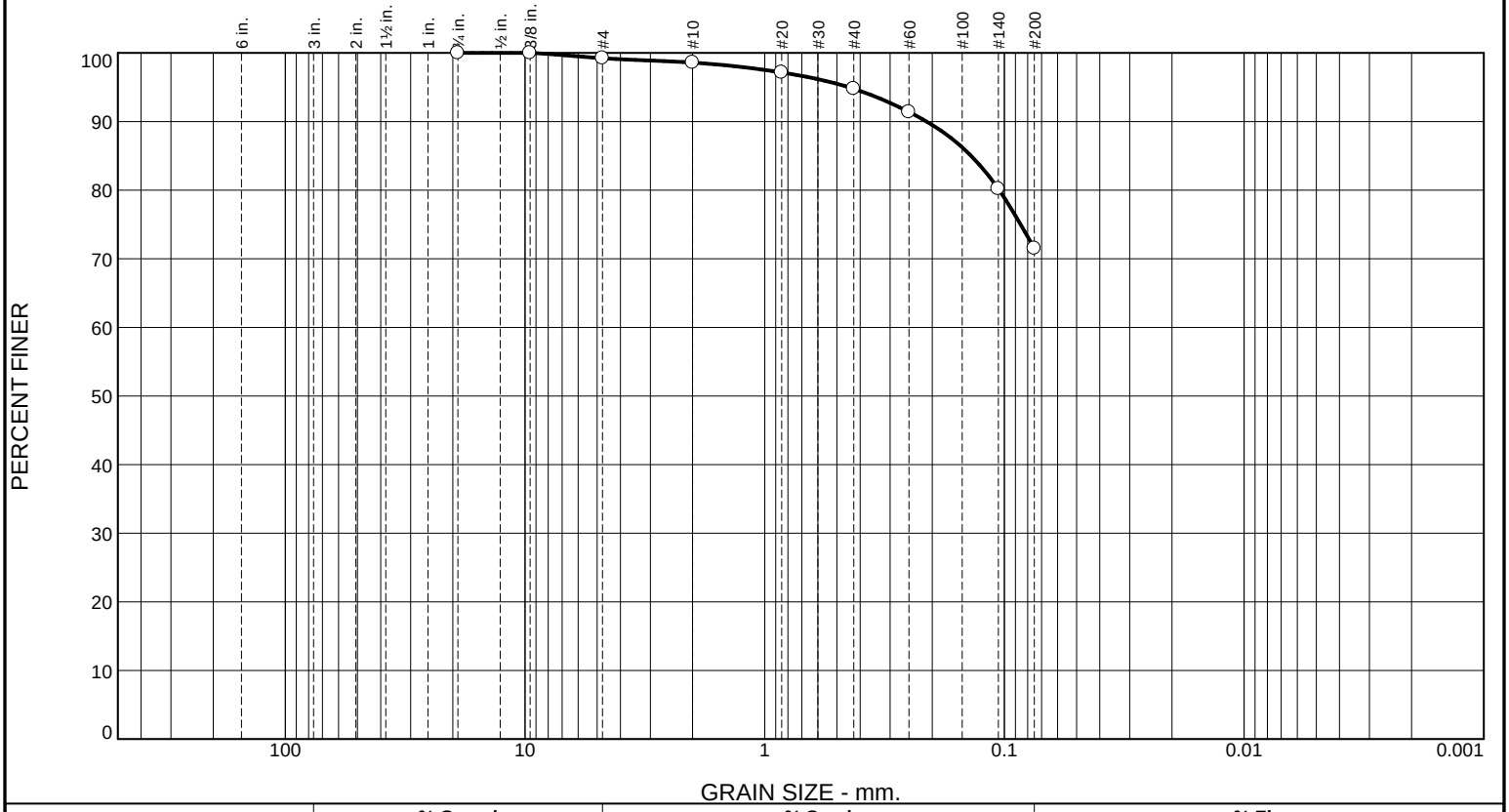
Source of Sample: B-4 **Depth:** 4.0' - 6.0'
Sample Number: S-3

Date: 09/27/2024

WHITESTONE ASSOCIATES, INC.
Warren, New Jersey

Client: MEMASI
Project: Proposed Addition & Site Improvements
103 South Broadway; Tarrytown, Westchester County, NY
Project No: GJ2422051.Y00 **Figure**

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.8	0.6	3.8	23.3	71.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3/4	100.0		
3/8	100.0		
#4	99.2		
#10	98.6		
#20	97.2		
#40	94.8		
#60	91.4		
#140	80.2		
#200	71.5		

* (no specification provided)

Material Description

Silt with Sand

Atterberg Limits

PL= 21 LL= 23 PI= 2

Coefficients

D_{90}= 0.2112 D_{85}= 0.1377 D_{60}=
D_{50}= D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

W_n = 17.8 %

Source of Sample: B-7 **Depth:** 4.0' - 6.0'
Sample Number: S-3

Date: 09/27/2024

WHITESTONE ASSOCIATES, INC.
Warren, New Jersey

Client: MEMASI
Project: Proposed Addition & Site Improvements
103 South Broadway; Tarrytown, Westchester County, NY
Project No: GJ2422051.Y00 **Figure**

WHITESTONE

APPENDIX C
Infiltration Test Results

WHITESTONE

INFILTRATION TEST

Client: MEMASI

Project: Proposed Addition & Site Improvements

Location: 103 South Broadway, Tarrytown, Westchester County, NY

File No. GJ2422051.Y00

Test Hole No.: I-1

Date: 7/29/2024 & 7/30/2024

Weather: Sunny

Field Engineer: CS

Surface Elevation: NS

Test Depth (Feet): 6.0

Test Depth (Elevation): NS

Reading No.	Time		Water Level Reading (inches)		Water Level Fall (Inches)	Time Interval (Hours)	Rate of Flow (Inches/Hour)
	Start	Finish	Start	Finish			
PS	7/29/24 10:30 AM	7/30/24 10:30 AM	24.0	22.0	2.0	24.0	-
1	10:30 AM	11:30 AM	24.0	24.0	0.0	1.0	0.0

Remarks: Field *i* = 0.0 in/hr

NOTES: PS = Pre Soak; NS = Not Surveyed

WHITESTONE

APPENDIX D
Supplemental Information (USCS, Terms & Symbols)

WHITESTONE

30 INDEPENDENCE BOULEVARD
SUITE 250
WARREN, NJ 07059
908.668.7777
whitestoneassoc.com

UNIFIED SOIL CLASSIFICATION SYSTEM

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			LETTER SYMBOL	TYPICAL DESCRIPTIONS
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	CLEAN GRAVELS (LITTLE OR NO FINES)	GW	WELL-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
			GP	POORLY-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)	GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES
			GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES
	SAND AND SANDY SOILS	CLEAN SAND (LITTLE OR NO FINES)	SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
			SP	POORLY-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	MORE THAN 50% OF COARSE FRACTION PASSING NO. 4 SIEVE	SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)	SM	SILTY SANDS, SAND-SILT MIXTURES
			SC	CLAYEY SANDS, SAND-CLAY MIXTURES
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMITS LESS THAN 50	ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
			CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
			OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS	LIQUID LIMITS GREATER THAN 50	MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
			CH	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS
			OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HIGHLY ORGANIC SOILS			PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS FOR SAMPLES WITH 5% TO 12% FINES

GRADATION*

% FINER BY WEIGHT

TRACE..... 1% TO 10%
LITTLE..... 10% TO 20%
SOME..... 20% TO 35%
AND..... 35% TO 50%

COMPACTNESS*
Sand and/or Gravel

RELATIVE DENSITY

LOOSE. ..... 0% TO 40%
MEDIUM DENSE.... 40% TO 70%
DENSE..... 70% TO 90%
VERY DENSE..... 90% TO 100%

CONSISTENCY*
Clay and/or Silt

RANGE OF SHEARING STRENGTH IN POUNDS PER SQUARE FOOT

VERY SOFT..... LESS THAN 250
SOFT..... ..... 250 TO 500
MEDIUM..... 500 TO 1000
STIFF..... 1000 TO 2000
VERY STIFF..... 2000 TO 4000
HARD..... GREATER THAN 4000

** VALUES ARE FROM LABORATORY OR FIELD TEST DATA, WHERE APPLICABLE. WHEN NO TESTING WAS PERFORMED, VALUES ARE ESTIMATED.*

L:\Geotechnical Forms and References\Reports\USCSTRMSSYM NJ.docx

WHITESTONE

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GEOTECHNICAL TERMS AND SYMBOLS

SAMPLE IDENTIFICATION

The Unified Soil Classification System is used to identify the soil unless otherwise noted.

SOIL PROPERTY SYMBOLS

N: Standard Penetration Value: Blows per ft. of a 140 lb. hammer falling 30" on a 2" O.D. split-spoon.
Qu: Unconfined compressive strength, TSF.
Qp: Penetrometer value, unconfined compressive strength, TSF.
Mc: Moisture content, %.
LL: Liquid limit, %.
PI: Plasticity index, %.
δd: Natural dry density, PCF.
▼: Apparent groundwater level at time noted after completion of boring.

DRILLING AND SAMPLING SYMBOLS

NE: Not Encountered (Groundwater was not encountered).
SS: Split-Spoon - 1 ⅜" I.D., 2" O.D., except where noted.
ST: Shelby Tube - 3" O.D., except where noted.
AU: Auger Sample.
OB: Diamond Bit.
CB: Carbide Bit
WS: Washed Sample.

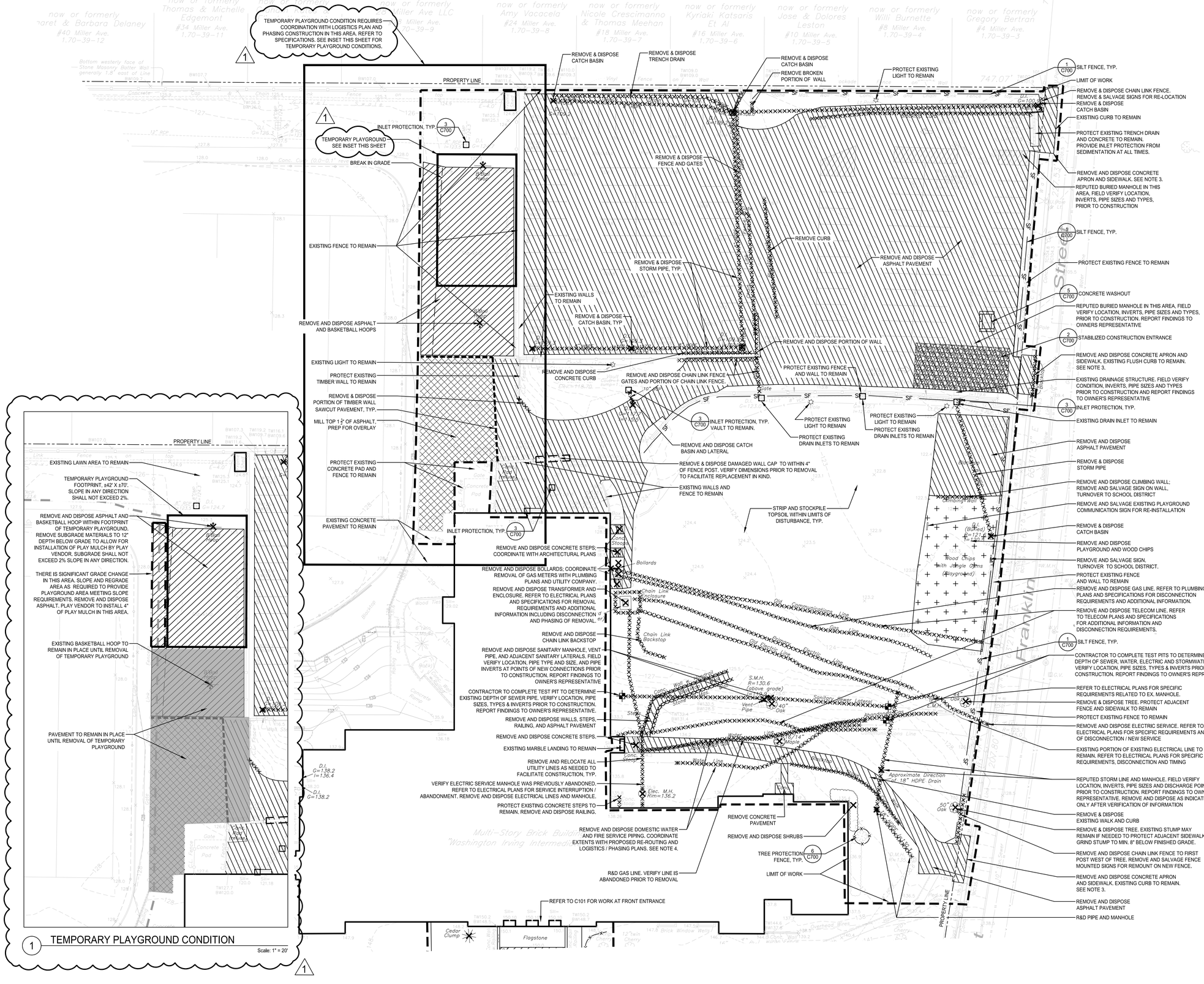
RELATIVE DENSITY AND CONSISTENCY CLASSIFICATION

Term (Non-Cohesive Soils)	Standard Penetration Resistance
Very Loose	0-4
Loose	4-10
Medium Dense	10-30
Dense	30-50
Very Dense	Over 50

Term (Cohesive Soils)	Qu (TSF)
Very Soft	0 - 0.25
Soft	0.25 - 0.50
Firm (Medium)	0.50 - 1.00
Stiff	1.00 - 2.00
Very Stiff	2.00 - 4.00
Hard	4.00+

PARTICLE SIZE

Boulders	8 in.+	Coarse Sand	5mm-0.6mm	Silt	0.074mm-0.005mm
Cobbles	8 in.-3 in.	Medium Sand	0.6mm-0.2mm	Clay	-0.005mm
Gravel	3 in.-5mm	Fine Sand	0.2mm-0.074mm		



PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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QUALITY ENVIRONMENTAL SOLUTIONS & TECHOLOGIES, INC
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WAPPINGERS FALLS, NY 12590
845.298.6031

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RYAN BIGGS I CLARK DAVIS ENGINEERING & SURVEYING DPC
257 USHERS ROAD
CLIFTON PARK, NY 12065
518.406.5506

MEP CONSULTANT
STANTEC
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203.352.1717

SITE - CIVIL CONSULTANT
THE LA GROUP
40 LONG ALLEY
SARATOGA SPRING, NY 12866

SECURITY CONSULTANT
BUILDING TECHNOLOGY CONSULTING LLC
992 BEDFORD STREET
BRIDGEWATER, MA. 02324

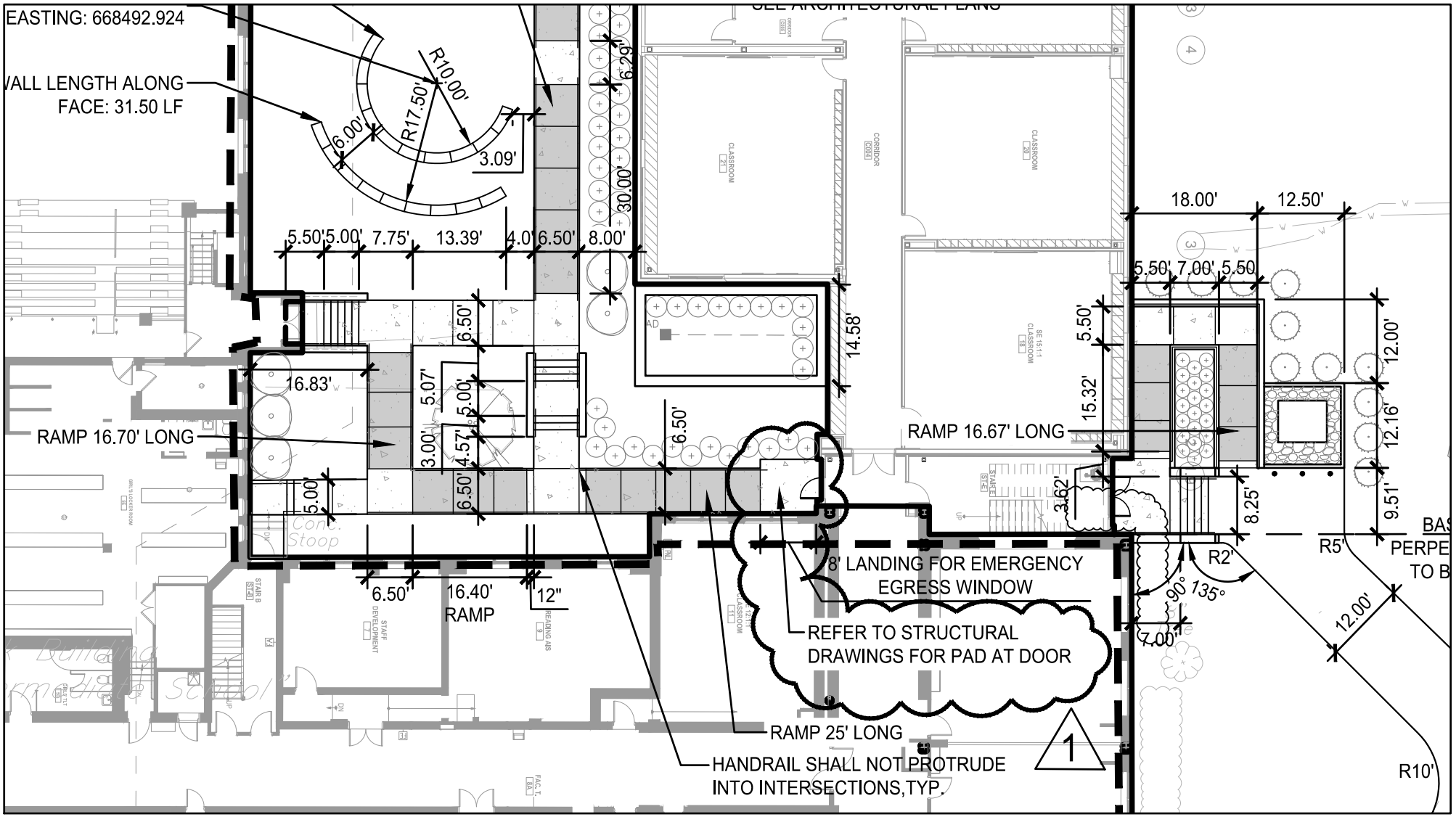
SEAL

ISSUE	DATE
⚠ ADDENDUM 4	04/17/2026
ISSUED FOR BID	03/27/2026

SED PROJECT NO.	66-04-01-03-0-002-017
MEMASI PROJECT NO.	101-2403

SITE PREP, DEMO AND EROSION CONTROL PLAN

C100



Public Schools of the Tarrytowns
2024 Capital Bond Project - Phase 2
Washington Irving Intermediate School

SK-C200-1

RE: SHEET C200 -
SITE PREP, DEMO, AND EROSION CONTROL PLAN

Addendum #4 | Concrete Door Pad

Tarrytown, NY

4/17/2026

20 10 0 5 10 15 20

Scale: 1" = 20'

N

The LA GROUP
Landscape Architecture & Engineering PC

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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BRIDGEWATER, MA. 02324

SEAL

⚠1 ADDENDUM 4	04/17/2026
ISSUED FOR BID	03/27/2026
ISSUE	DATE

SED PROJECT NO.	66-04-01-03-0-002-017
MEMASI PROJECT NO.	101-2403

SITE UTILITIES PLAN

C500

now or formerly Margaret & Barbara Delaney #40 Miller Ave. 1.70-39-12
now or formerly Thomas & Michelle Edgemont #34 Miller Ave. 1.70-39-11
now or formerly Graham Kates & Elizabeth Skoski #32 Miller Ave. 1.70-39-10
now or formerly 28 Miller Ave LLC #28 Miller Ave. 1.70-39-9
now or formerly Amy Vacacela #24 Miller Ave. 1.70-39-8
now or formerly Nicole Crescimanno & Thomas Meehan #18 Miller Ave. 1.70-39-7
now or formerly Kyriaki Katsaris Et Al #16 Miller Ave. 1.70-39-6
now or formerly Jose & Dolores Leston #10 Miller Ave. 1.70-39-5
now or formerly Willi Burnette #8 Miller Ave. 1.70-39-4
now or formerly Gregory Bertran #4 Miller Ave. 1.70-39-3

Miller Avenue
Street
Franklin
John Street
747.07'
74.28'
227.63'
S 65°31'53" E
Multi-Story Brick Building "Washington Irving Intermediate School"
i - Sport Athletic Field
LIMIT OF WORK
NEW ELECTRIC SERVICE. REFER TO ELECTRICAL PLANS FOR ADDITIONAL DETAILS AND INFORMATION
TEMPORARY ELECTRIC SERVICE. REFER TO ELECTRICAL PLANS AND SPECIFICATIONS FOR ADDITIONAL DETAILS AND INFORMATION
SMH-1 RIM: 131.5 INV. IN: VERIFY INV. OUT: 119.0
SMH-2 RIM: 127.1 INV. IN: 118.33 INV. OUT: 118.23
SMH-3 RIM: 123.20 INV. IN: 117.15 INV. OUT:117.05
SMH-4 RIM: 130.25 INV. IN: 116.42 INV. OUT: VERIFY
108 LF OF 6" SDR-26 @1.0%
68 LF OF 6" SDR-26 @1.0%
63 LF OF 6" SDR-26 @1.0%
6" & 6" SANITARY WYE CONNECTION
3" & 6" WATERLINE BYPASS PIPING
45° BEND AND THRUST BLOCK, TYP.
3" x 3" TEE W/ 3" GATE VALVE & THRUST BLOCK
6" x 6" TEE W/ 6" GATE VALVE & THRUST BLOCK
SANITARY SEWER BUILDING LATERAL AND CLEAN OUT
FIRE DEPARTMENT CONNECTION POINT
CUT AND CAP BYPASS WATERLINES , SEE NOTES
CONTRACTOR TO COMPLETE TEST PIT TO LOCATE EXISTING SANITARY PIPE PRIOR TO CONSTRUCTION & PROVIDE INVERT ELEVATION TO OWNER'S REPRESENTATIVE
NEW BUILDING ADDITION
EXISTING 3" WATER SERVICE TO REMAIN
EXISTING 6" WATER SERVICE TO REMAIN
CUT AND CAP EXISTING WATERLINES, SEE NOTES
3" x 3" TAPPING SLEEVE W/ 3" GATE VALVE AND THRUST BLOCK
6" x 6" TAPPING SLEEVE W/ 6" GATE VALVE AND THRUST BLOCK
TEMPORARY GAS SERVICE, TO BE ABANDONED ONCE NEW ELECTRICAL SERVICE AND HEATING SYSTEM IS OPERATIONAL, AND UPON DIRECTION FROM OWNER'S REPRESENTATIVE. REFER TO PLUMBING PLANS AND SPECS FOR DETAILS AND ADDITIONAL INFORMATION.
TEMPORARY ELECTRIC SERVICE. REFER TO ELECTRICAL PLANS FOR ADDITIONAL DETAILS AND INFORMATION
CONTRACTOR TO COMPLETE TEST PIT TO LOCATE EXISTING SANITARY PIPE PRIOR TO CONSTRUCTION & PROVIDE INVERT ELEVATION TO OWNER'S REPRESENTATIVE
3" x 3" TAPPING SLEEVE W/ 3" GATE VALVE & THRUST BLOCK
6" x 6" TAPPING SLEEVE W/ 6" GATE VALVE & THRUST BLOCK
REMOVE EXISTING TELECOMMUNICATIONS SERVICE. REFER TO TELE-COM PLANS AND SPECIFICATIONS FOR DETAILS AND ADDITIONAL INFORMATION.
NEW ELECTRICAL SERVICE AND TRANSFORMER. REFER TO ELECTRICAL PLANS FOR ADDITIONAL DETAILS AND INFORMATION.
STORMWATER PIPING, SEE GRADING AND DRAINAGE PLANS.

WATERLINE INSTALLATION SEQUENCING

1. NOTIFY VILLAGE ENGINEER PRIOR TO WORK AND COORDINATE TIMING / SEQUENCING. CONTACT DONATO PENNELLA, VILLAGE ENGINEER, (914) 631-3668.
2. INSTALL TWO (2) 3" TAPPING SLEEVES AND TWO (2) 6" TAPPING SLEEVES ON THE EXISTING 3" AND 6" WATER SERVICES RESPECTIVELY AS NOTED.
3. INSTALL 3" AND 6" WATER LINES AROUND PROPOSED BUILDING ADDITION. INSTALL 3" AND 6" TEES AS NOTED FOR FUTURE WATERLINE CONNECTIONS WITHIN PROPOSED BUILDING ADDITION.
4. COMPLETE PRESSURE TEST, CHLORINATION, AND MICROBIAL TESTING IN ACCORDANCE WITH THE SPECIFICATION ON NEW WATERLINE.
5. SHUT OFF 3" AND 6" WATER LINES AT ROAD. CUT AND CAP EXISTING WATER LINES WITHIN PROPOSED BUILDING ADDITION FOOTPRINT. COORDINATE WITH OWNER FOR TIMING FOR WATERLINE SHUTDOWN.
6. TURN ON WATER LINE. WATER SERVICE TO THE BUILDING SHALL UTILIZE THE NEWLY INSTALLED WATERLINE AROUND THE PROPOSED BUILDING ADDITION.
7. UPON COMPLETION OF WATERLINE INSTALLATION WITHIN PROPOSED BUILDING ADDITION, CUT AND CAP WATERLINE BYPASS AROUND BUILDING. WATERLINE BYPASS PIPING TO BE ABANDONED IN PLACE.

LEGEND

- LIMIT OF WORK LINE
- PROPERTY LINE
- 247 PROPOSED CONTOUR LINE
- 247 EXISTING CONTOUR LINE
- ST PROPOSED STORM LINE
- PROPOSED TEMPORARY ELECTRICAL LINE
- E PROPOSED ELECTRICAL LINE
- G PROPOSED GAS LINE
- T PROPOSED TELE-COM LINE
- SAN PROPOSED SANITARY SEWER LINE
- PROPOSED BYPASS WATER LINE
- W PROPOSED WATER LINE
- SS EXISTING SANITARY SEWER LINE
- W EXISTING WATER LINE
- G EXISTING GAS LINE
- E EXISTING ELECTRIC LINE
- T EXISTING TELE-COM LINE
- ST EXISTING STORM LINE

20 10 0 5 10 15 20
Scale: 1" = 20'

SHEET NOTES

A. VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE THE START OF ANY WORK. CONTRACTOR MUST NOTIFY THE ARCHITECT OF ANY VARIATIONS FROM CONDITIONS SHOWN ON THESE DRAWING.

B. MECHANICAL EQUPMENTS, PIPING, WALL SLEEVE, AND INTAKE LOUVER TO BE REMOVED BY MC. SEE MECH DWGS.

KEY NOTES

D1 REMOVE VCT / VAT FLOORING, INCLUDING BASE AND MASTIC UNDER ABATEMENT CONDITIONS. REFER TO ABATEMENT DRAWINGS.
D2 REMOVE VCT FLOORING, INCLUDING BASE AND MASTIC.
D3 AIR CONDITIONER UNIT AND ALL SUPPORTS TO BE REMOVED BY MC. SEE MECH DWGS.
D5 REMOVE SUSPENDED (ACT / GRID) CEILING SYSTEM IN ITS ENTIRETY, INCLUDING ALL HANGERS AND FASTENERS. REFER TO ELECT AND MECH DWGS FOR EQUIPMENT REMOVALS.
D11 (1) REMOVE ACOUSTIC TILES FROM UNDERSIDE OF STRUCTURE. (2) LIGHT FIXTURE TO BE REMOVED BY E.C. REFER TO ELECTRICAL DRAWINGS. PATCH AND REPAIR SURFACE TO RECEIVE NEW FINISH.
D12 REMOVE EXISTING GWB SOFFIT.
D13 REMOVE EXISTING COUNTER DOOR AND FRAME.
D26 REMOVE MUSICAL INSTRUMENT STORAGE LOCKERS AND TURN OVER TO OWNER. REINSTALL IN BASEMENT.
D79 REMOVE EXIST CONCRETE STAIR ASSEMBLY IN ITS ENTIRETY INCLUDING STAIR RAILING AND ALL ASSOCIATED SUPPORTS.
D82 REMOVE AND DISPOSE OF ALL EXISTING METAL LOCKERS AND LOCKER BASES IN THEIR ENTIRETY.
D83 EXISTING PARTITION WALL TO BE REMOVED IN THEIR ENTIRETY. REFER TO PLUMBING DWG'S FOR FIXTURE REMOVALS
D84 EXISTING SHOWER PARTITION TO BE REMOVED IN THEIR ENTIRETY.
D85 REMOVE EXISTING WIRE MESH WALL AND ALL ASSOCIATED SUPPORTS IN ITS ENTIRETY.
D86 REMOVE EXISTING WOOD STUD, PLYWOOD PARTITION AND DOOR IN ITS ENTIRETY INCLUDING ALL ASSOCIATED SUPPORTS.
D87 EXISTING DRINKING FOUNTAIN TO BE REMOVED AND PROTECTED FOR REINSTALLATION IN ITS EXISTING LOCATION.
D88 EXISTING STAIR RAILING TO BE REMOVED AND PROTECTED FOR REINSTALLATION IN ITS EXISTING LOCATION.
D97 REMOVE ABANDONED CONCRETE LOCKER BASES DOWN TO CONCRETE FLOOR SLAB.
D98 REMOVE EXISTING WOODEN ARENA BENCH SEATING.
D107 BASE BID: REMOVE EXISTING DOOR, FRAME, AND HARDWARE.
D108 ALTERNATE 2: REMOVE EXISTING DOOR, FRAME, AND HARDWARE.
D114 REMOVE EXISTING NOTICE BOARD/TEACHING BOARD TO ALLOW FOR NEW WALL OPENING OR NEW WALL PARTITION. PATCH EXISTING WALL TO RECEIVE NEW FINISHES TO MATCH EXISTING.

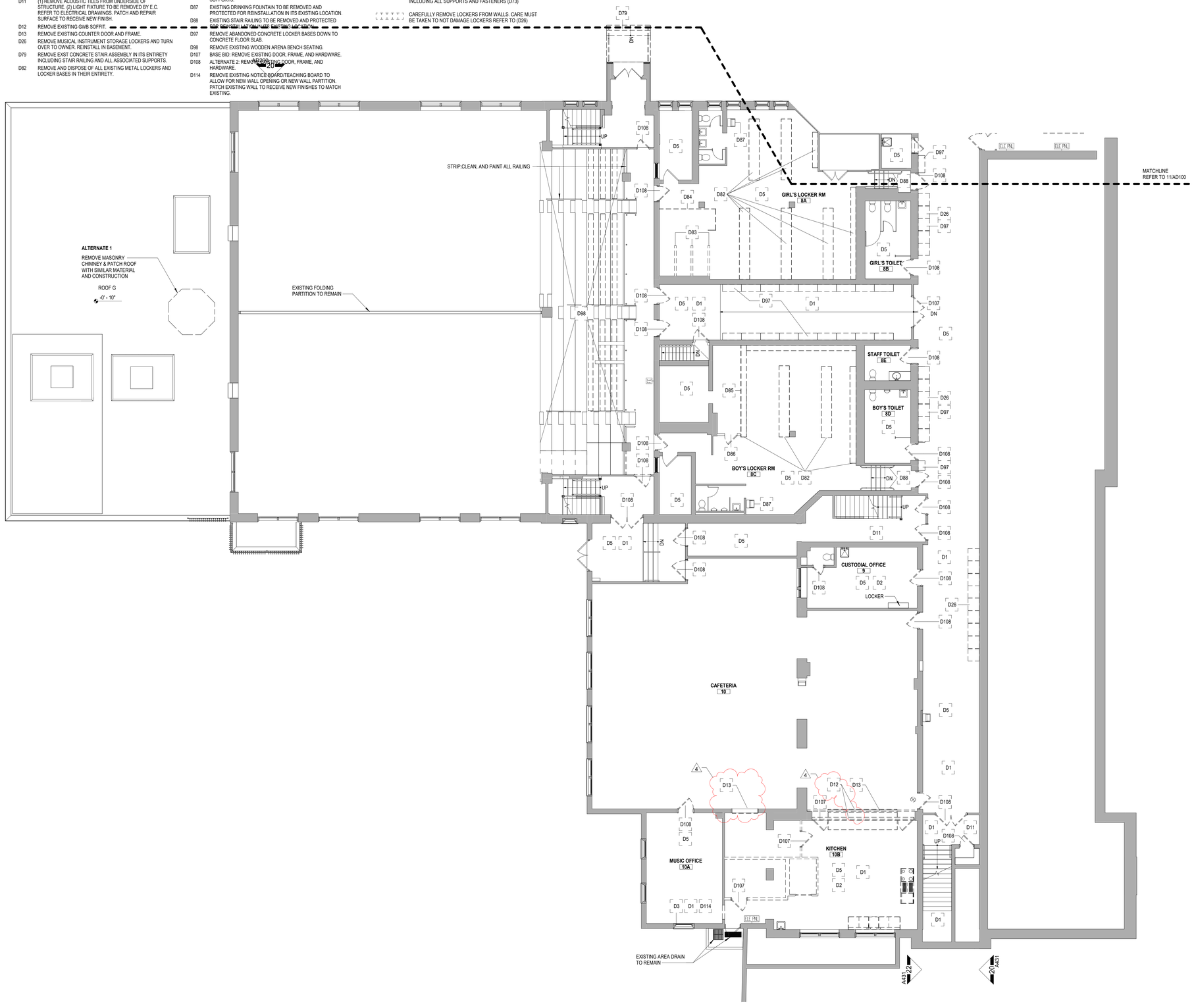
DEMOLITION LEGEND

REMOVE EXISTING DOOR ASSEMBLY, INCLUDING FRAME AND ALL ASSOCIATED HARDWARE

REMOVE EXISTING PARTITION ASSEMBLY, FROM FLOOR TO SLAB ABOVE, INCLUDING ALL SUPPORTS AND FASTENERS. INFILL PLASTER CEILING TO MATCH ADJACENT SURFACES TO MAINTAIN FIRE BARRIER (D21)

REMOVE EXISTING HEATING/VENTILATION SHAFT FROM TOP OF FLOOR SLAB TO BOTTOM OF SLAB ABOVE, INCLUDING ALL SUPPORTS AND FASTENERS (D73)

CAREFULLY REMOVE LOCKERS FROM WALLS. CARE MUST BE TAKEN TO NOT DAMAGE LOCKERS REFER TO (D26)



GROUND FLOOR DEMOLITION PLAN - AREA B
1/8" = 1'-0"

10

Public Schools of the Tarrytowns

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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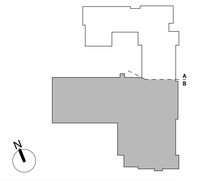
SEAL

BID ADDENDUM	4	04/17/2026
ISSUED FOR BID		03/27/2026
ISSUE		DATE

KEY PLAN



PROJECT NO. 66 - 04 - 01 - 03 - 0 - 002 - 017
MEMASI PROJECT NO. 101 - 2403

DEMOLITION PART PLAN - GROUND FLOOR AREA B

AD101-B

SHEET NOTES

1. FURNITURE SHOWN FOR COORDINATION PURPOSES. NOT PART OF GC SCOPE.
2. PATCH AND REPAIR EXISTING WALL SURFACES WHERE TRIMS, MOLDINGS, OR FIXTURES WERE REMOVED IN PREPARATION TO RECEIVE NEW FINISHES. REPAIR EXISTING WALL SURFACES AS REQUIRED TO PROVIDE AN EVEN AND UNIFORM SURFACE IN THE WALL'S ENTIRETY. REFER TO CONSTRUCTION NOTES #5.1- #5.18 ON G002 FOR MORE INFORMATION AND PROJECT SPECS.
3. REFER TO AF001 FOR MATERIAL FINISHES.
4. PROVIDE WOOD BLOCKING BEHIND ALL WALL MOUNTED EQUIPMENT.
5. PROVIDE ACCESS PANELS INSIDE CASEWORK WHERE REQUIRED FOR ACCESS TO MECH EQUIPMENT BEYOND.
6. GC SHALL COORDINATE CASEWORK RECEPTACLES LOCATIONS WITH ARCH AND ELEC DWGS.

KEY NOTES

CASEWORK LEGEND

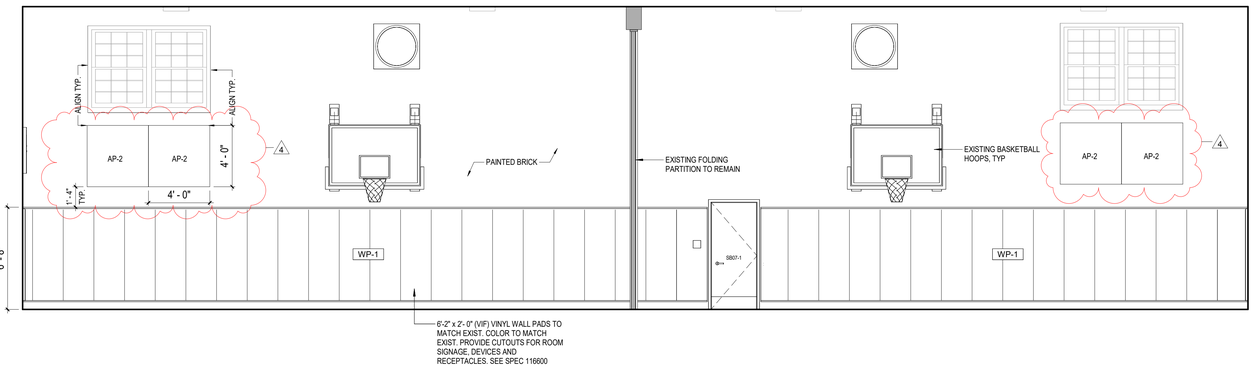
HEIGHT OF CASEWORK — XX/XX — WIDTH OF CASEWORK
CABINET DESIGN SERIES (CDS) NUMBER — XX/XX — DEPTH OF CASEWORK

SYMBOL/ABBREVIATION LEGEND

SB	SMARTBOARD
WB	WHITE BOARD
TB	TACK BOARD
	HALFTONE INDICATES NEW FURNITURE
SS	SOLID SURFACE
PB	PIN-UP BOARD
AWP-"XX"	ACOUSTIC WALL PANEL
AP-"XX"	ACOUSTIC PANEL

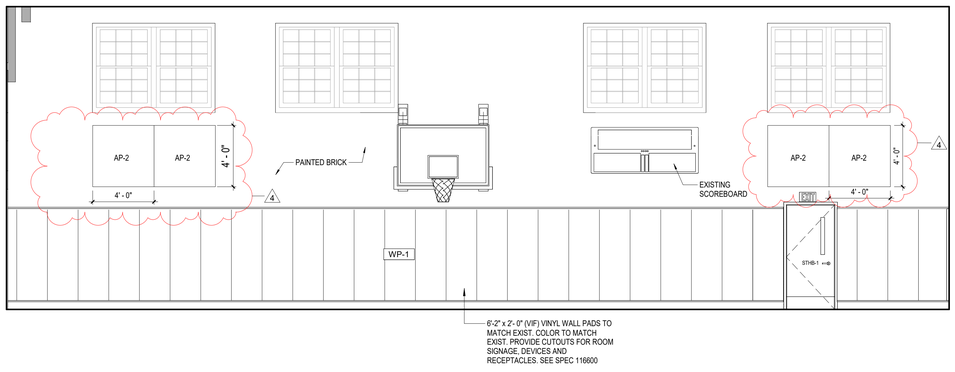
LEGEND

- SHAFT
- INFILL SLAB WHERE SHAFT/STAIR WAS REMOVED
- CASEWORK
- NOT IN SCOPE



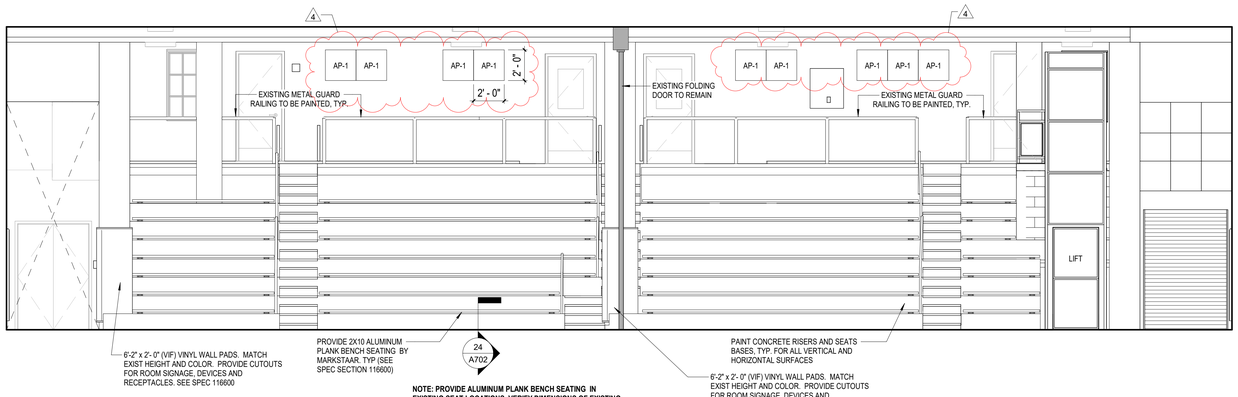
WEST ELEVATION - GYM
1/4" = 1'-0"

21



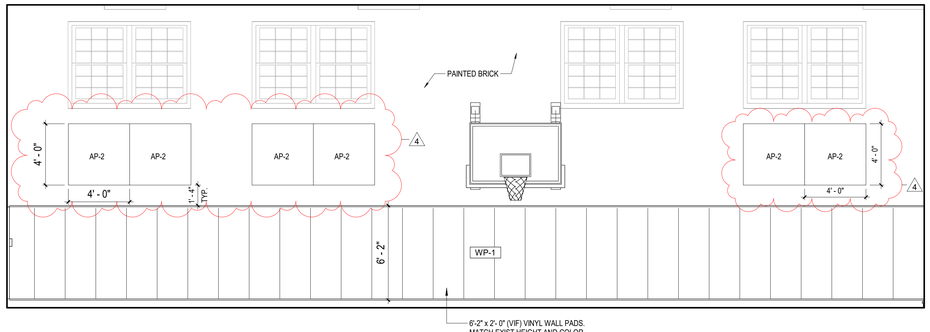
SOUTH ELEVATION - GYM
1/4" = 1'-0"

20



EAST ELEVATION - GYM
1/4" = 1'-0"

11



NORTH ELEVATION - GYM
1/4" = 1'-0"

10

Public Schools of the Tarrytowns

2024 CAPITAL BOND PROJECT - PHASE 2

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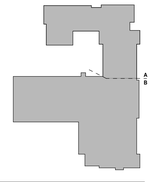
SECURITY CONSULTANT
BUILDING TECHNOLOGY CONSULTING LLC
992 BEDFORD STREET
BRIDGEWATER, MA. 02324

SEAL

BID ADDENDUM	4	04/17/2026
ISSUED FOR BID		03/27/2026
ISSUE		DATE

KEY PLAN



PROJECT NO. 66 - 04 - 01 - 03 - 0 - 002 - 017
MEMASI PROJECT NO. 101 - 2403

GYMNASIUM INTERIOR ELEVATIONS

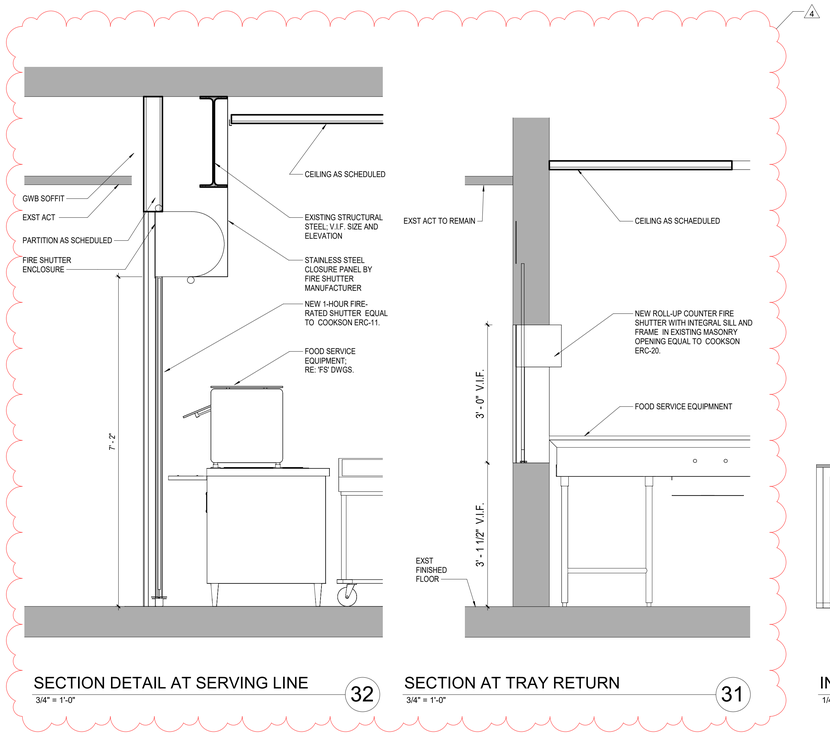
A403

SHEET NOTES

1. FURNITURE SHOWN FOR COORDINATION PURPOSES. NOT PART OF GC SCOPE.
2. PATCH AND REPAIR EXISTING WALL SURFACES WHERE TRIMS, MOLDINGS, OR FIXTURES WERE REMOVED IN PREPARATION TO RECEIVE NEW FINISHES. REPAIR EXISTING WALL SURFACES AS REQUIRED TO PROVIDE AN EVEN AND UNIFORM SURFACE IN THE WALL'S ENTIRETY. REFER TO CONSTRUCTION NOTES #5.1- #5.18 ON G002 FOR MORE INFORMATION AND PROJECT SPECS.
3. REFER TO AF001 FOR MATERIAL FINISHES.
4. PROVIDE WOOD BLOCKING BEHIND ALL WALL MOUNTED EQUIPMENT.
5. PROVIDE ACCESS PANELS INSIDE CASEWORK WHERE REQUIRED FOR ACCESS TO MECH EQUIPMENT BEYOND.
6. GC SHALL COORDINATE CASEWORK RECEPTACLES LOCATIONS WITH ARCH AND ELEC DWGS.

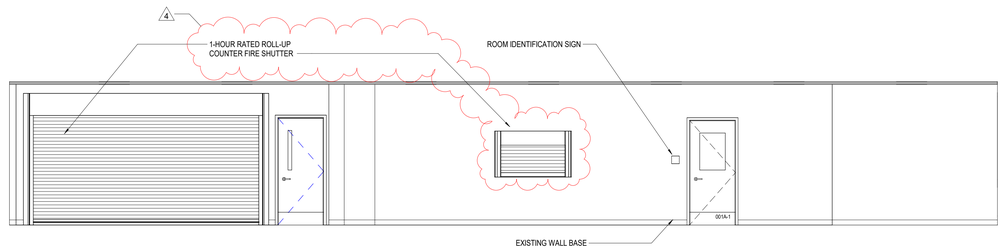
KEY NOTES

10.16 PROVIDE STAINLESS STEEL CORNER GUARD.

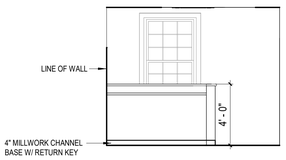


SECTION DETAIL AT SERVING LINE 32
3/4" = 1'-0"

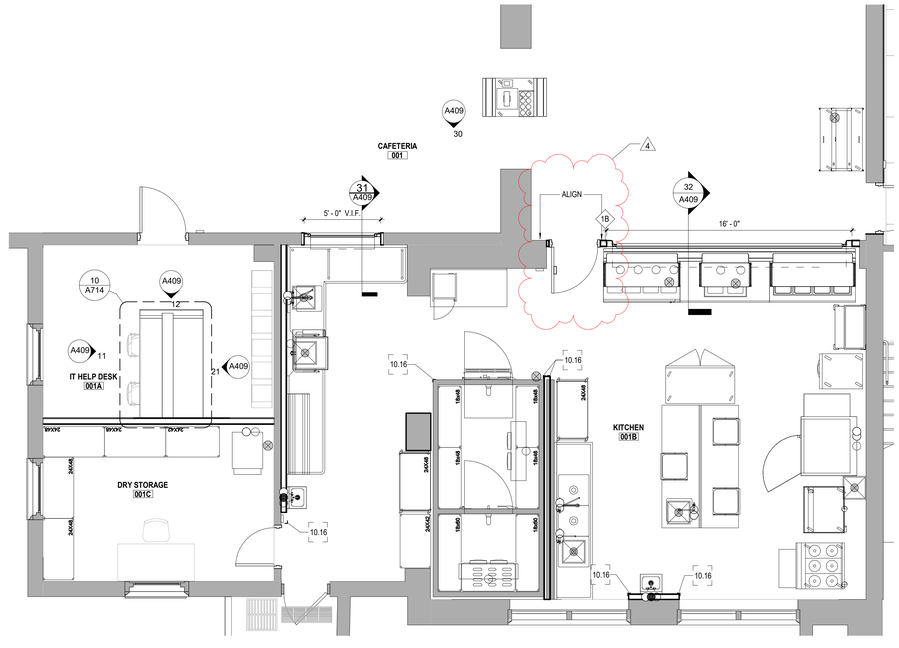
SECTION AT TRAY RETURN 31
3/4" = 1'-0"



INTERIOR ELEVATION - SERVING LINE 30
1/4" = 1'-0"

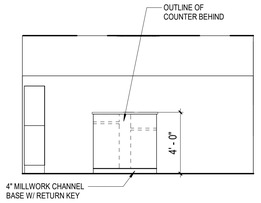


EAST ELEVATION - IT HELP DESK 21
1/4" = 1'-0"

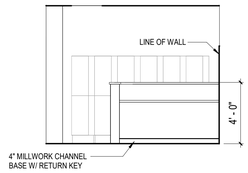


ENLARGED PLAN - KITCHEN & HELP DESK 10
1/4" = 1'-0"

NOTE: REFER TO FOOD SERVICE DRAWINGS FOR COMMERCIAL KITCHEN EQUIMENT.



WEST ELEVATION - IT HELP DESK 12
1/4" = 1'-0"



NORTH ELEVATION - IT HELP DESK 11
1/4" = 1'-0"

Public Schools of the Tarrytowns

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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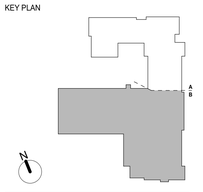


PROJECT NO. 66 - 04 - 01 - 03 - 0 - 002 - 017
MEMASI PROJECT NO. 101 - 2403

KITCHEN & HELP DESK ENLARGED PLAN & INTERIOR ELEVATIONS

A409

SHEET NOTES

DEFINITIONS

KEY NOTES

3.01	PARGE AND LEVEL CONCRETE LANDING.
6.01	REPLICATE EXISTING WOOD PANELS/TRIM IN WESTERN RED CEDAR. PREPARE AND PAINT.
6.02	REPLICATE PILASTER BASE IN WESTERN RED CEDAR. PREP AND PAINT.
7.03	PROVIDE NEW ALUMINUM LEADER HEAD AND LEADER. CONNECT TO EXISTING UNDERGROUND STORM DRAINAGE PIPING.
7.07	REMOVE EXISTING SHEET METAL FLASHING AND REPLACE WITH NEW. MECHANICALLY FASTEN TO SUBSTRATE.
8.06	EXISTING TRANSOM TO REMAIN.
9.06	SCOPE OF WORK BY OWNER TO PAINT WHERE NECESSARY.
9.85	PREPARE AND PAINT EXISTING METAL RAILING
9.86	PREPARE AND PAINT ALL EXISTING WOOD COMPONENTS FROM PILASTER BASE TO CORNICE.
10.18	REINSTALL EXISTING SIGN.
12.02	PROVIDE NEW RUBBER DOOR MAT.
D36	REMOVE RAIN LEADER HEAD AND LEADER.
D115	PREPARE CONCRETE LANDING FOR PARGING.
D116	REMOVE EXISTING DOOR MAT. REPAIR AND CLEAN EXISTING STONE STOOP. DEMO
D117	DISMANTLE EXISTING WOOD PANELING/TRIM. AVOID DAMAGING ADJACENT SURFACES. PATCH AND REPAIR ANY DAMAGE TO ADJACENT SURFACES.
D118	DISMANTLE DECAYED WOOD PILASTER BASE.
D119	DISMANTLE AND STORE EXISTING SIGN FOR REINSTALLTION.

SOUTH ENTRY REFERENCE PHOTO - SOUTH 33

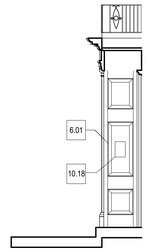
3/16" = 1'-0"

SOUTH ENTRY REFERENCE PHOTO - EAST 15

1/32" = 1'-0"

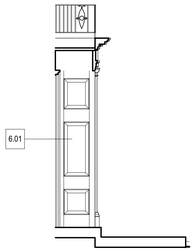
SOUTH ENTRY REFERENCE PHOTO - WEST 13

1/16" = 1'-0"



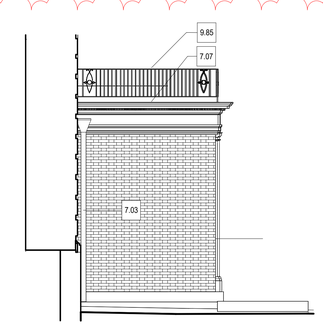
SOUTH ENTRY INT. ELEVATION - WEST 31

1/4" = 1'-0"



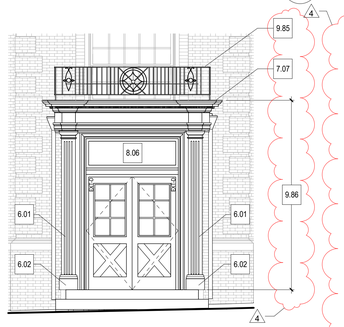
SOUTH ENTRY INT. ELEVATION - EAST 30

1/4" = 1'-0"



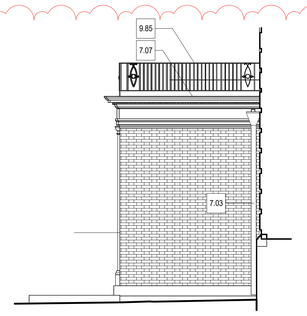
SOUTH ENTRY EXT. ELEVATION - WEST 22

1/4" = 1'-0"



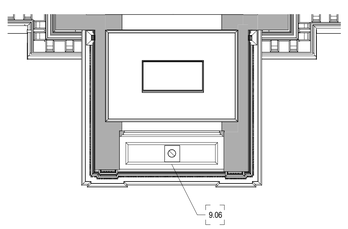
SOUTH ENTRY ELEVATION - SOUTH 21

1/4" = 1'-0"



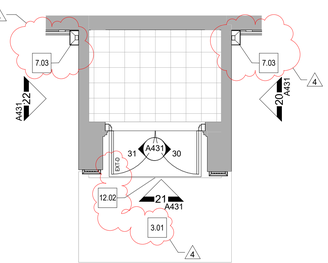
SOUTH ENTRY EXT. ELEVATION - EAST 20

1/4" = 1'-0"



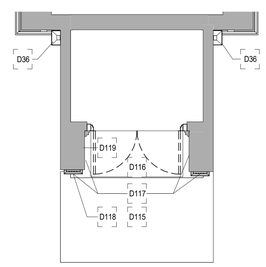
SOUTH ENTRY RCP 12

1/4" = 1'-0"



ENLARGED PLAN AT SOUTH ENTRY 11

1/4" = 1'-0"



SOUTH ENTRY DEMOLITION PLAN 10

1/4" = 1'-0"

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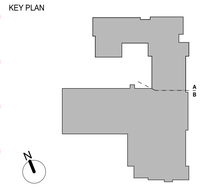


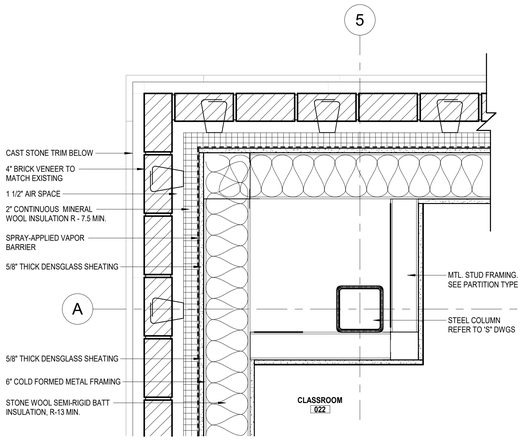
PROJECT NO.	66 - 04 - 01 - 03 - 0 - 002 - 017
MEMASI PROJECT NO.	101 - 2403

ENLARGED PLANS AND ELEVATIONS SOUTH ENTRY

A431

SHEET NOTES

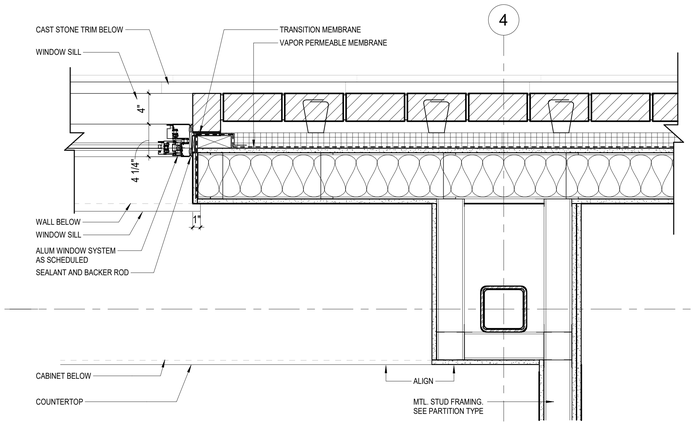
KEY NOTES



EXTERIOR PLAN DETAIL @ COLUMN A-5

1 1/2" = 1'-0"

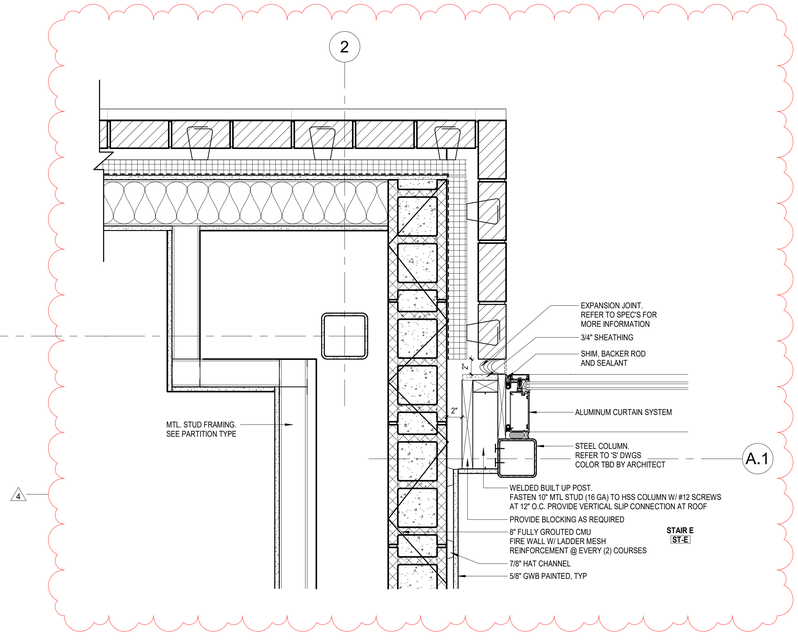
22



EXTERIOR JAMB DETAIL NEAR COLUMN A-4

1 1/2" = 1'-0"

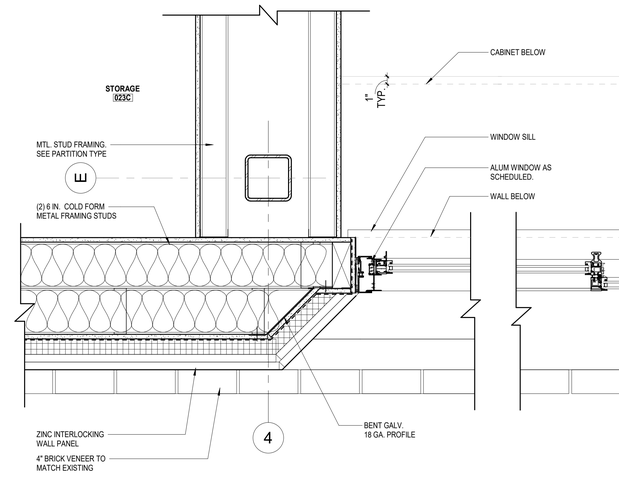
21



PLAN DETAIL @ COLUMN A-2

1 1/2" = 1'-0"

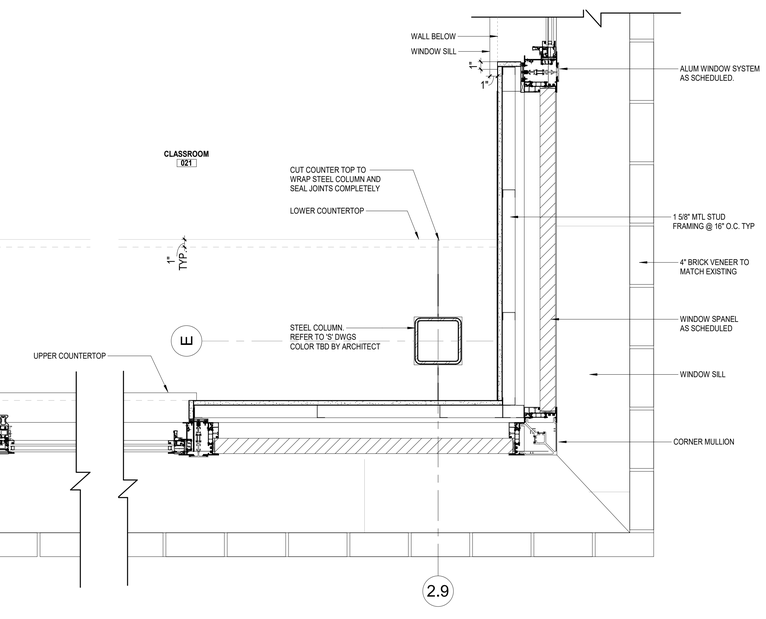
20



EXTERIOR JAMB DETAIL NEAR COLUMN E-4

1 1/2" = 1'-0"

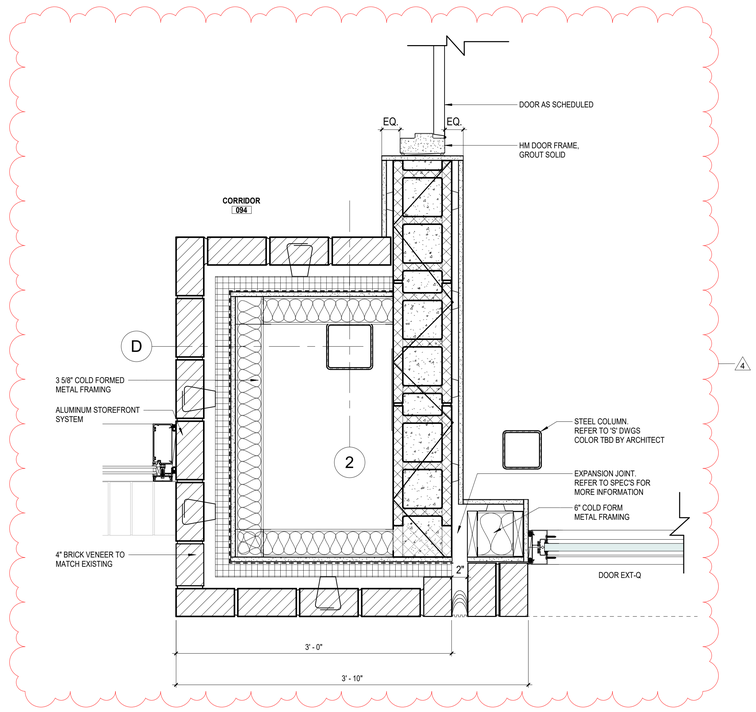
12



EXTERIOR JAMB DETAIL NEAR COLUMN E-2.9

1 1/2" = 1'-0"

11



PLAN DETAIL @ EXPASION JOINT

1 1/2" = 1'-0"

10

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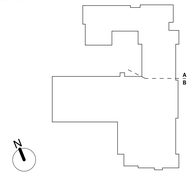


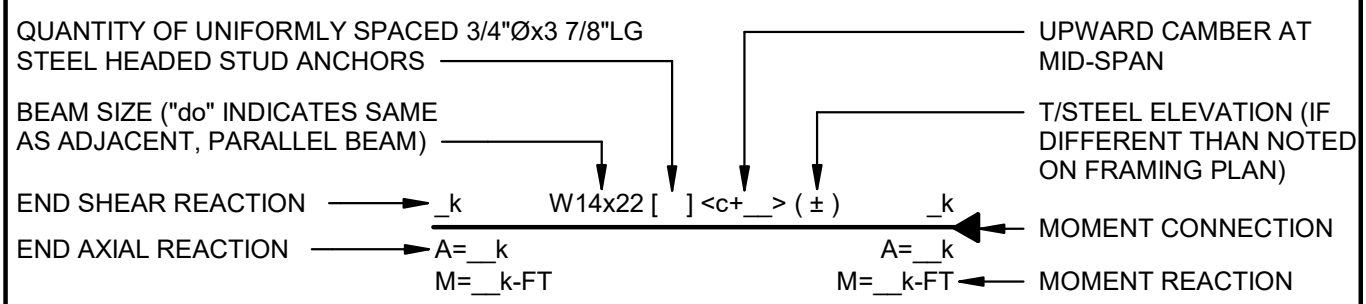
PROJECT NO.	66 - 04 - 01 - 03 - 0 - 002 - 017
MEMASI PROJECT NO.	101 - 2403

PLAN DETAILS

A502

4

STEEL BEAM LEGEND

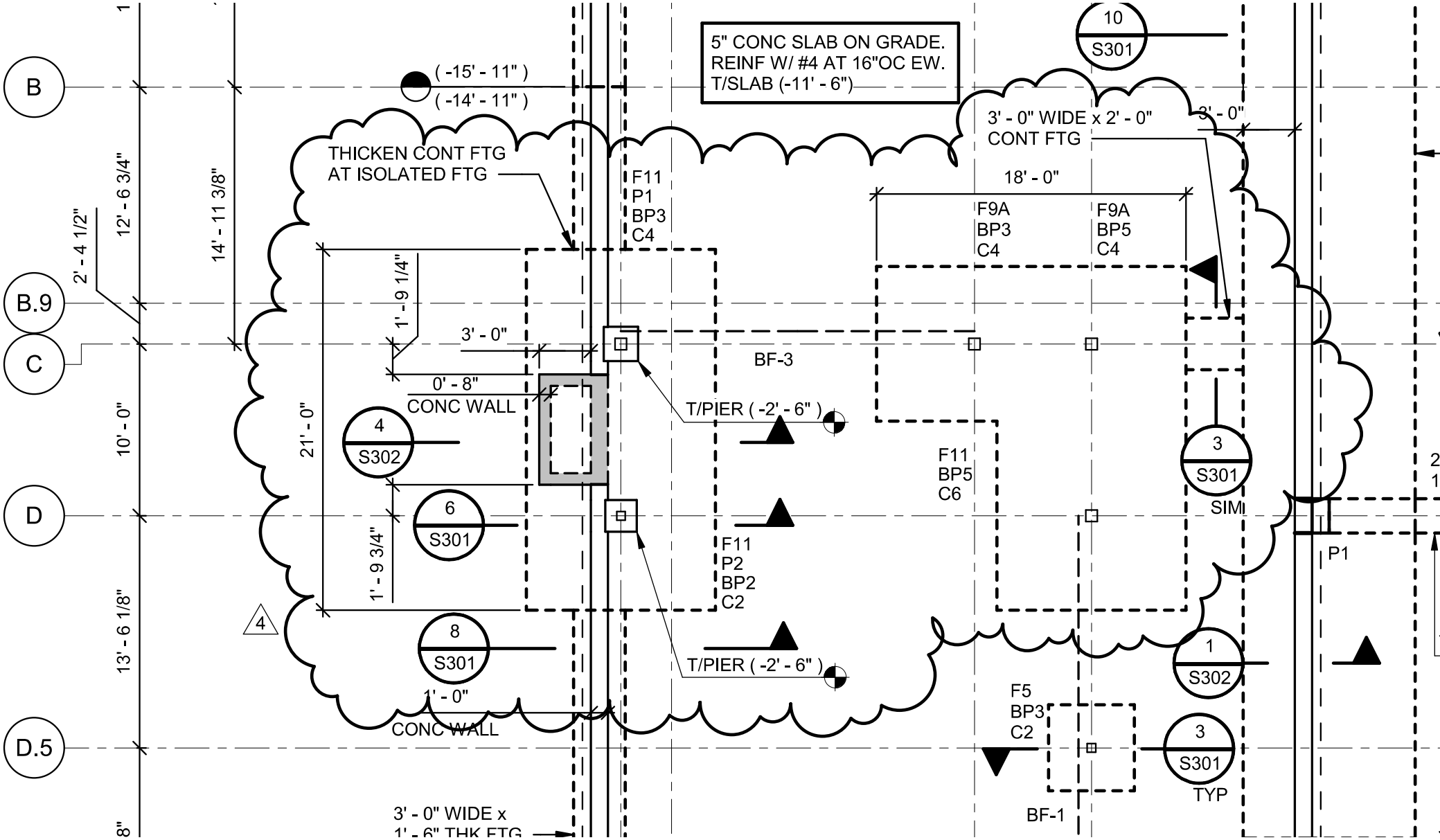


NOTES:

1. DO NOT CAMBER BEAMS UNLESS A VALUE FOR CAMBER IS SPECIFIED.
2. REFER TO THE SPECIFICATION FOR CONNECTION DESIGN CRITERIA WHERE LOADS AND MOMENTS ARE NOT SHOWN.
3. LOADS AND MOMENTS ARE LRFD (LOAD AND RESISTANCE FACTOR DESIGN).
4. SHEAR REACTIONS ARE THE SAME AT EACH END OF THE BEAM, UNLESS NOTED OTHERWISE.

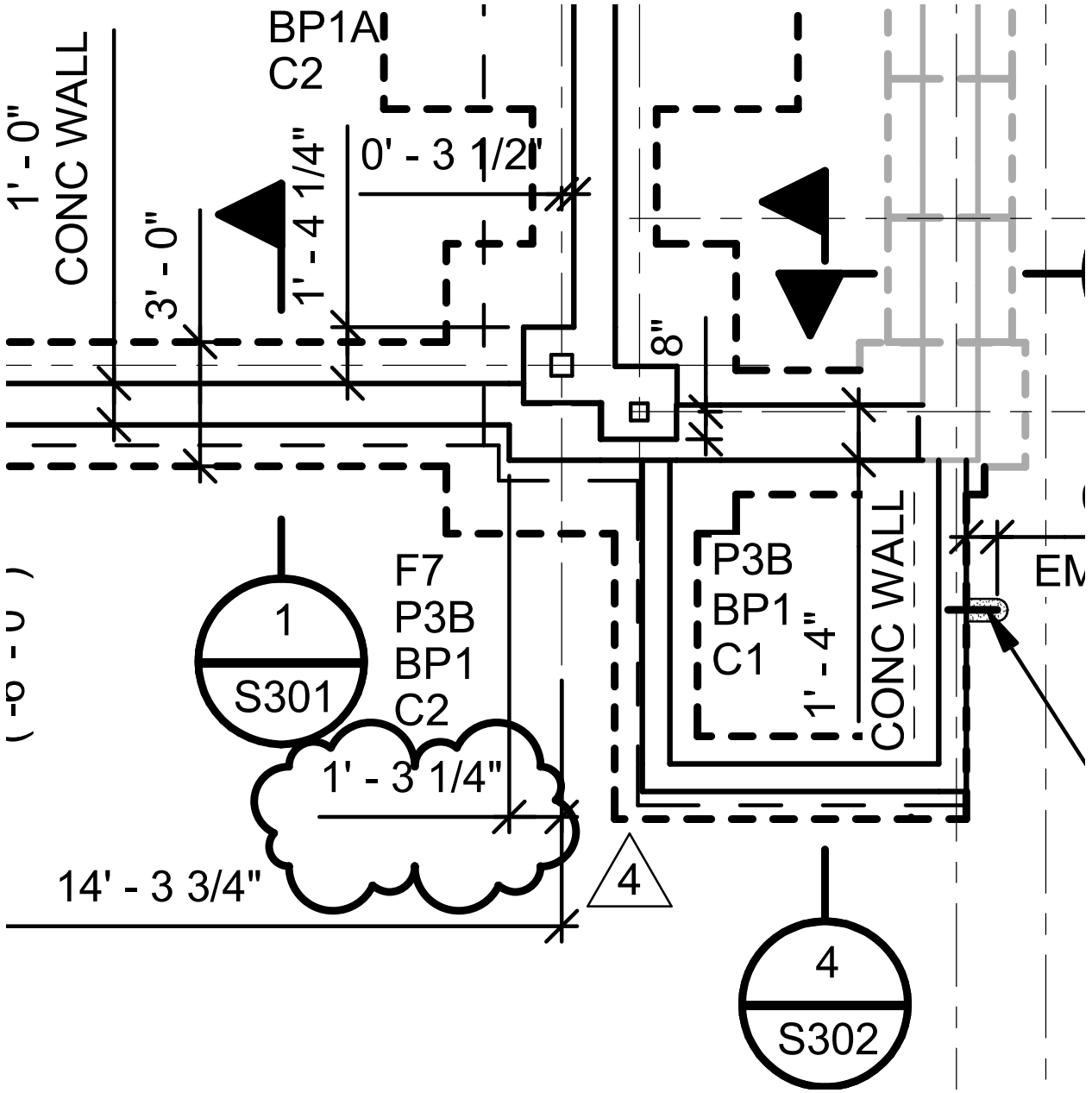
GENERAL NOTES CONTINUED

S002



BASEMENT
FOUNDATION
PLAN - AREA A

S100



BASEMENT FOUNDATION PLAN - AREA A

S100

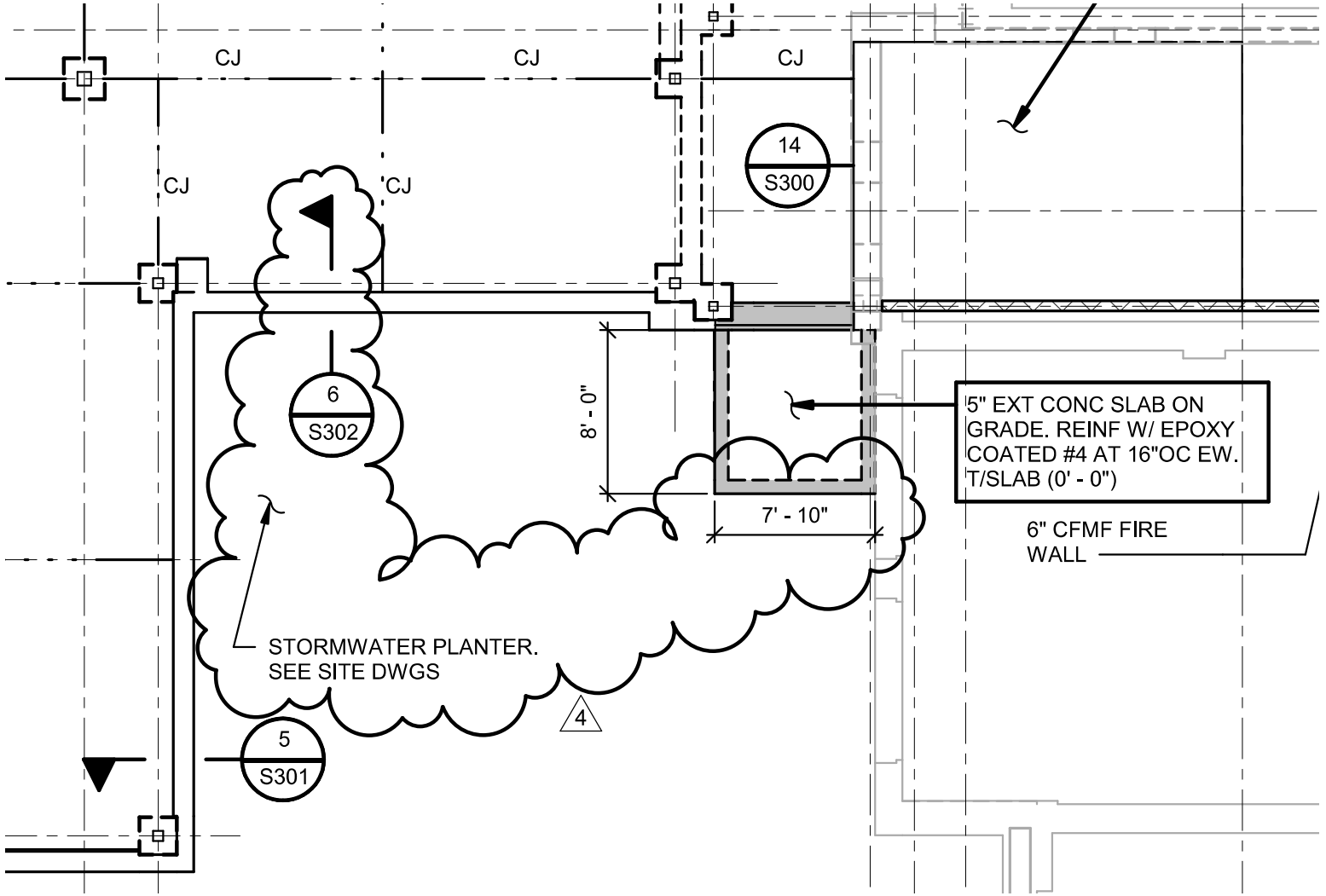
COLUMN FOOTING SCHEDULE

MARK	SIZE	REINFORCING	REMARKS
F4	4' - 0" x 4' - 0" x 1' - 6"	(6)#5 EW, BOTTOM	
F5	5' - 0" x 5' - 0" x 1' - 6"	(7)#5 EW, BOTTOM	
F6	6' - 0" x 6' - 0" x 1' - 6"	(8)#5 EW, BOTTOM	
F6A	6' - 0" x 6' - 0" x 1' - 6"	(8)#5 EW, TOP AND BOTTOM	
F65	6' - 6" x 6' - 6" x 1' - 6"	(9)#5 EW, BOTTOM	
F7	7' - 0" x 7' - 0" x 1' - 6"	(9)#5 EW, BOTTOM	
F8	8' - 0" x 8' - 0" x 1' - 6"	(11)#5 EW, BOTTOM	
F8A	8' - 0" x 8' - 0" x 1' - 6"	(11)#5 EW, TOP AND BOTTOM	
F85	8' - 6" x 8' - 6" x 1' - 6"	(11)#5 EW, BOTTOM	
F9	9' - 0" x 9' - 0" x 1' - 6"	(12)#5 EW, BOTTOM	
F9A	9' - 0" x 9' - 0" x 2' - 0"	(12)#7 EW, TOP AND BOTTOM	TOP BARS W/ ACI STD 90 DEG HOOK
F10	10' - 0" x 9' - 0" x 1' - 6"	(8)#7 EW, TOP AND BOTTOM	
F105	10' - 6" x 10' - 6" x 1' - 6"	(11)#6 EW, TOP AND BOTTOM	TOP BARS W/ ACI STD 90 DEG HOOK
F11	11' - 0" x 11' - 0" x 2' - 0"	(13)#7 EW TOP, (18) #7 EW BOTTOM	TOP BARS W/ ACI STD 90 DEG HOOK

4

BASEMENT FOUNDATION PLAN - AREA A

S100

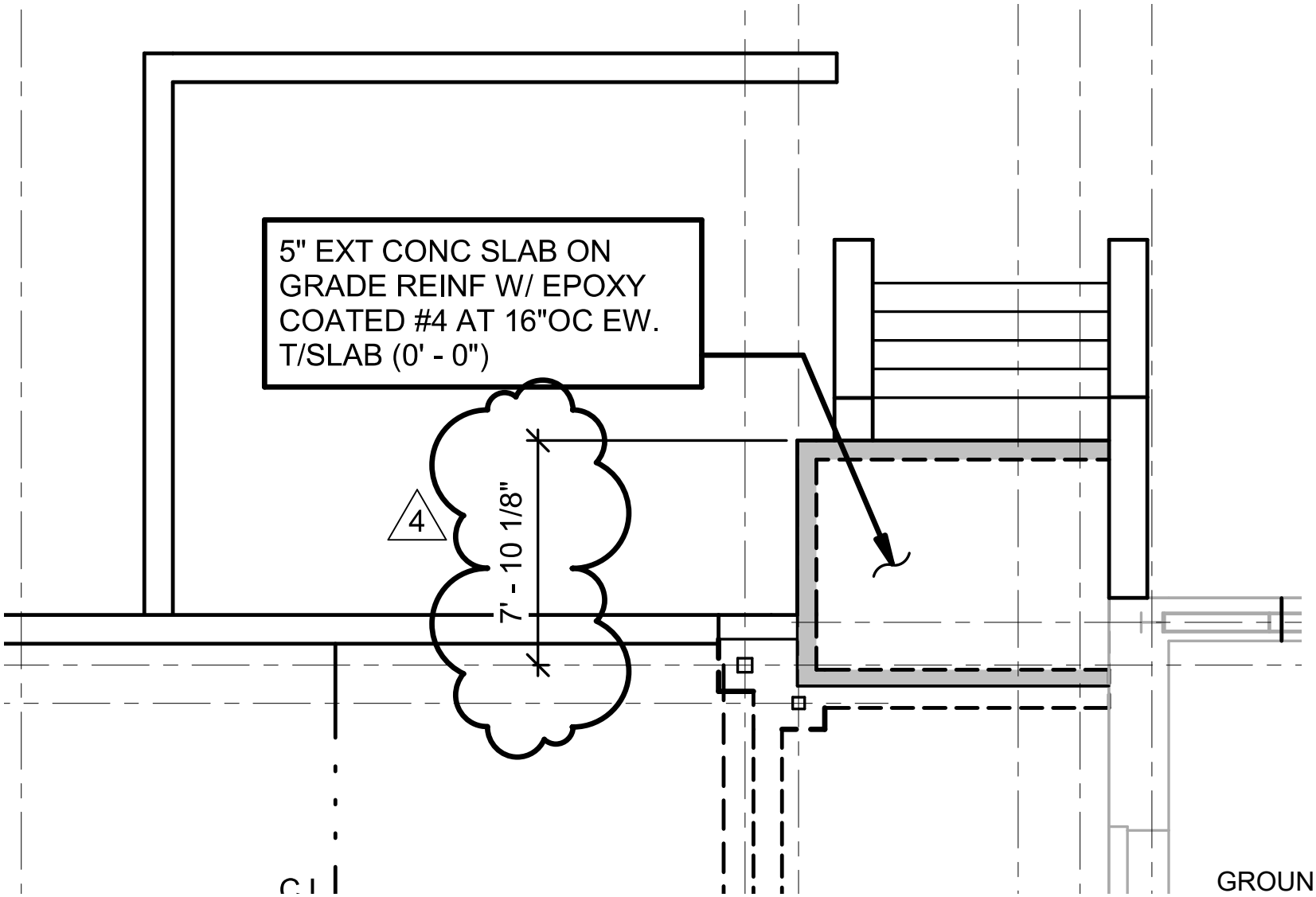


GROUND FLOOR FRAMING PLAN - AREA A

S101

BID ADDENDUM 4 04/17/2026

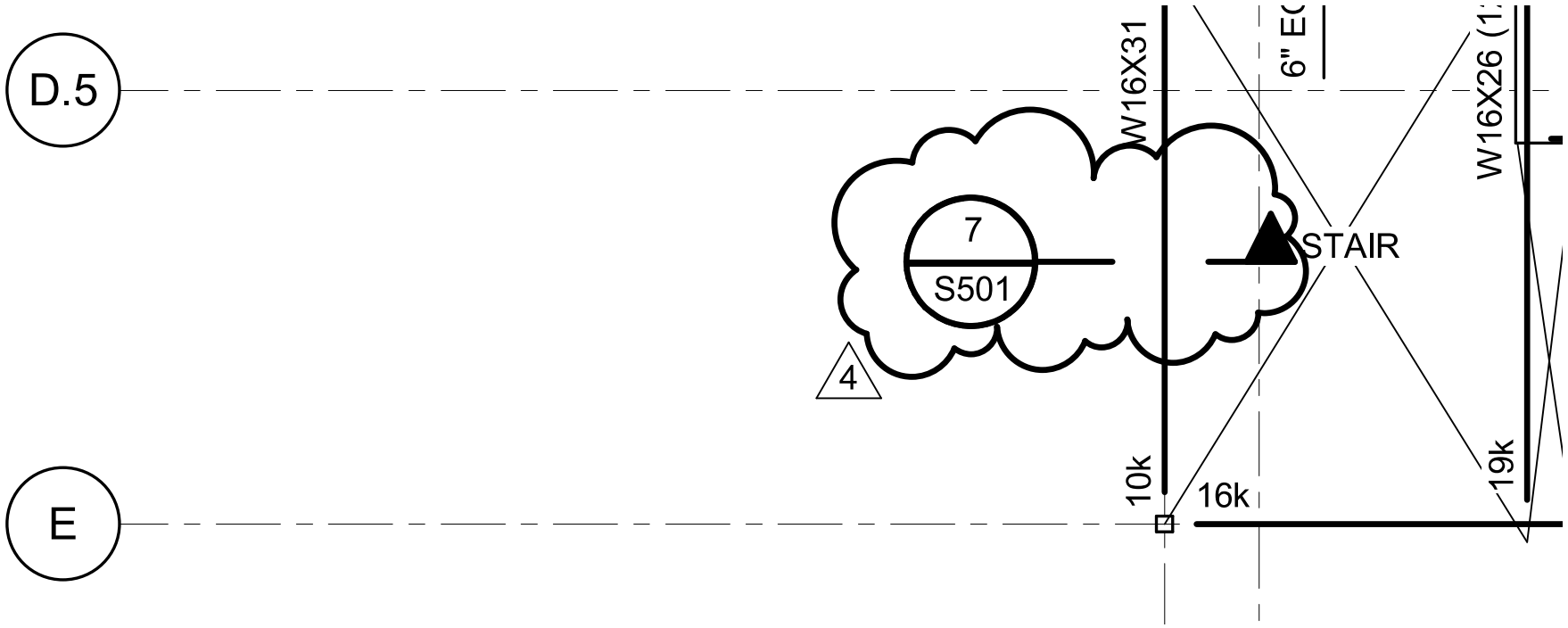
14585.5



GROUND FLOOR FRAMING PLAN - AREA A

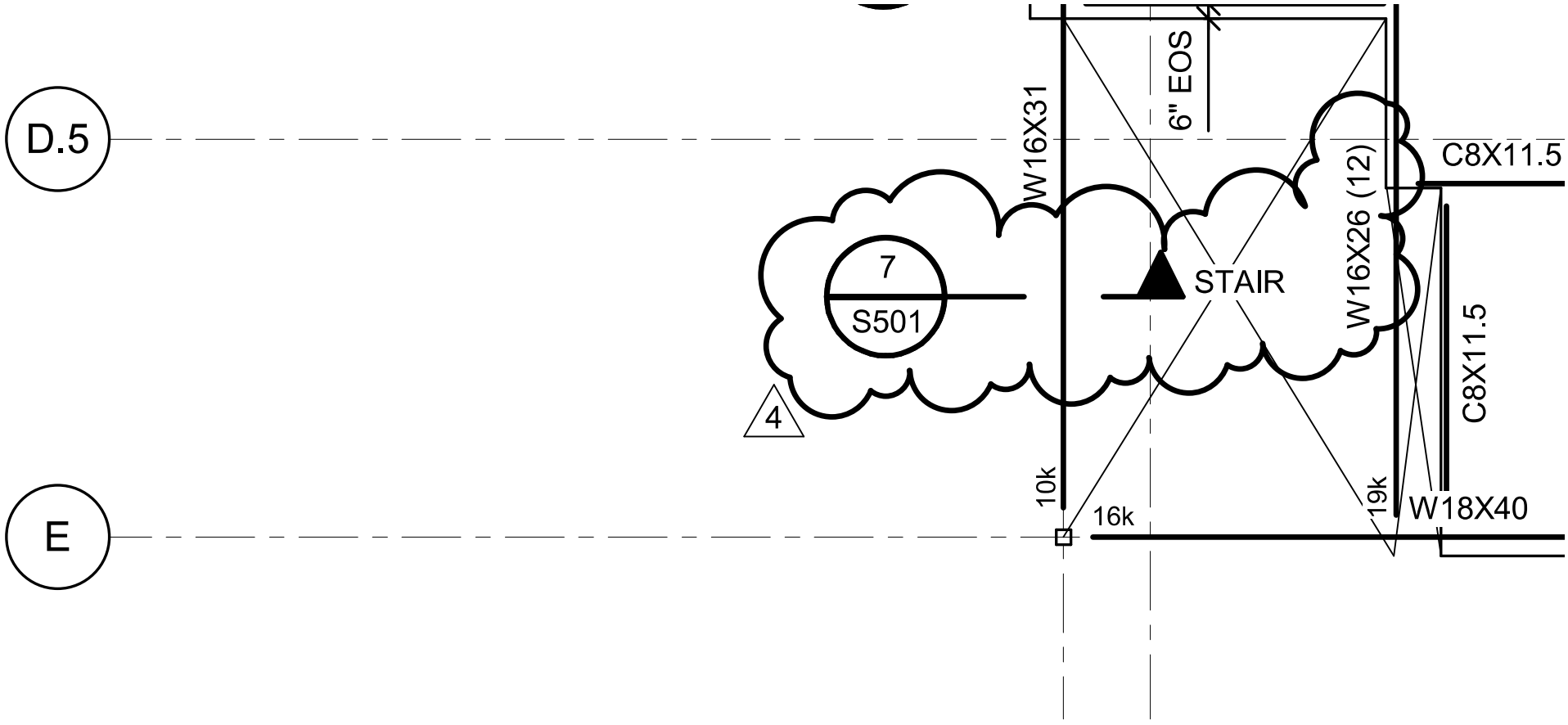
S101

14585.5



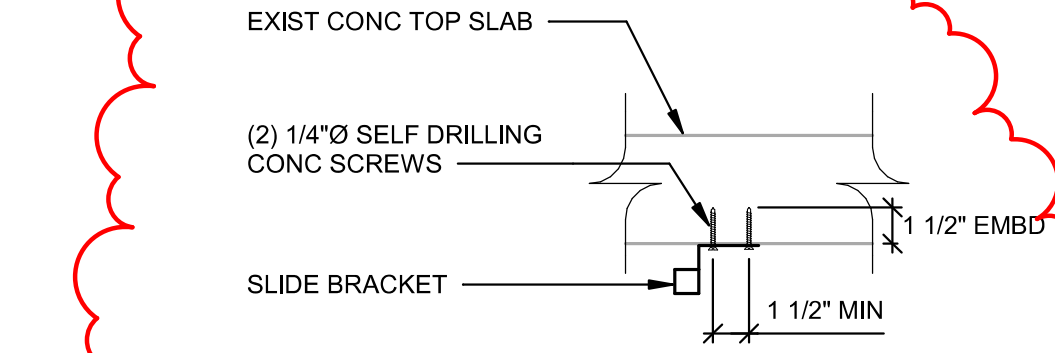
FIRST FLOOR FRAMING PLAN - AREA A

S102



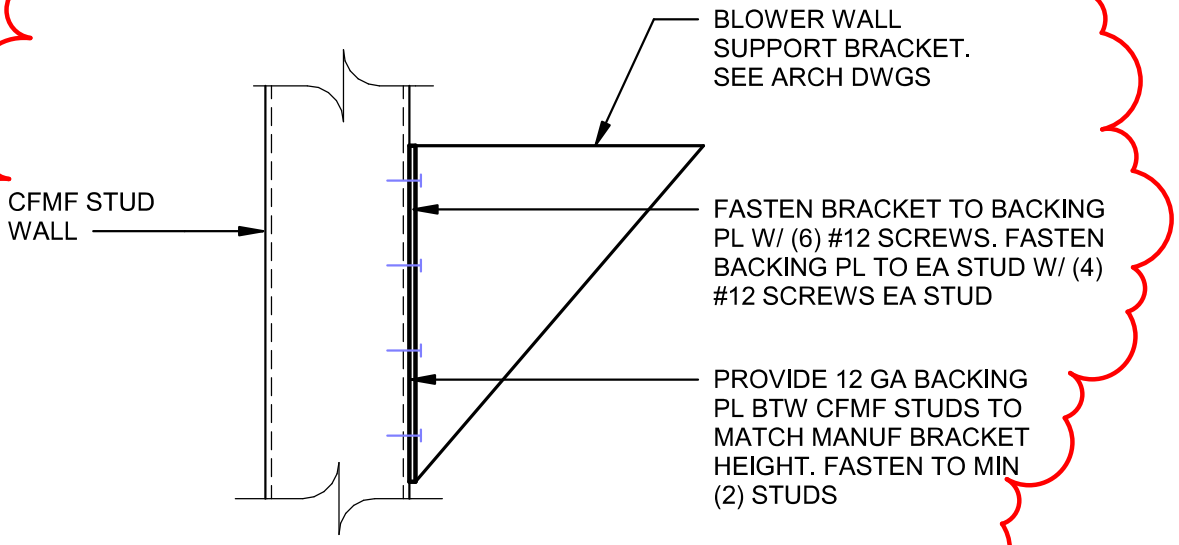
SECOND FLOOR FRAMING PLAN - AREA A

S103



PULLEY ATTACHMENT

4



HOOD ATTACHMENT

10 / S200 SECTION

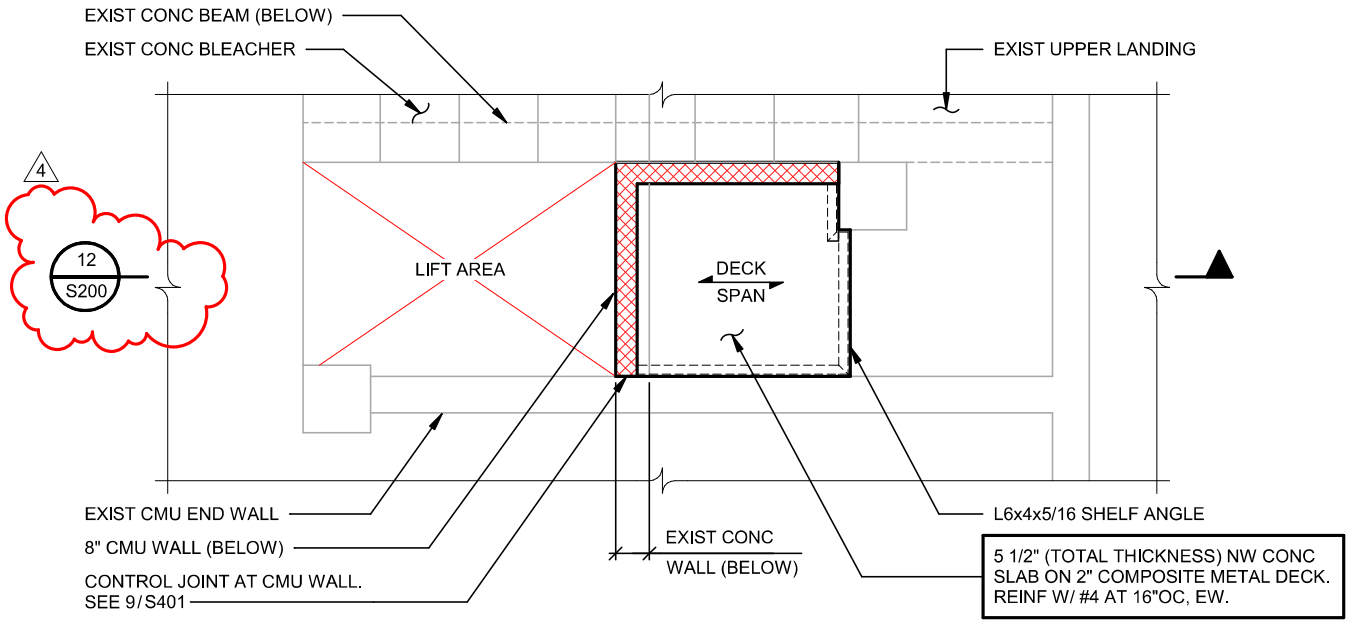
NOT TO SCALE

NOTE:

1. COORDINATE ALL DIMENSIONS WITH EXISTING CONDITIONS AND MANUFACTURER'S REQUIREMENTS.
2. PROVIDE PULLEY ATTACHMENTS ONLY TO THE UNDERSIDE OF THE EXISTING TOP SLAB.
3. PROVIDE UNPUNCHED SUPPORT BRACKET FOR SCREW FASTENING THROUGH BACKING PLATE TO CFMF STUDS.

PARTIAL PLANS AND DETAILS

S200



4

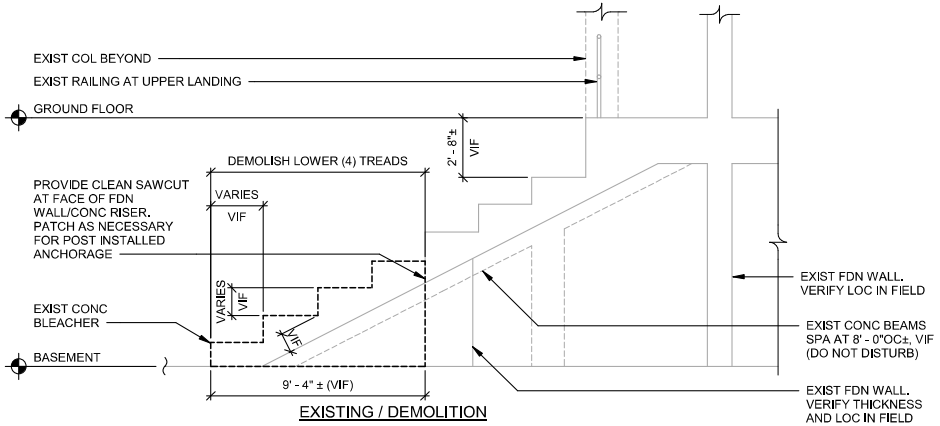
11 S200 PARTIAL PLAN AT GYMNASIUM BLEACHER

SCALE: 1/4" = 1'-0"

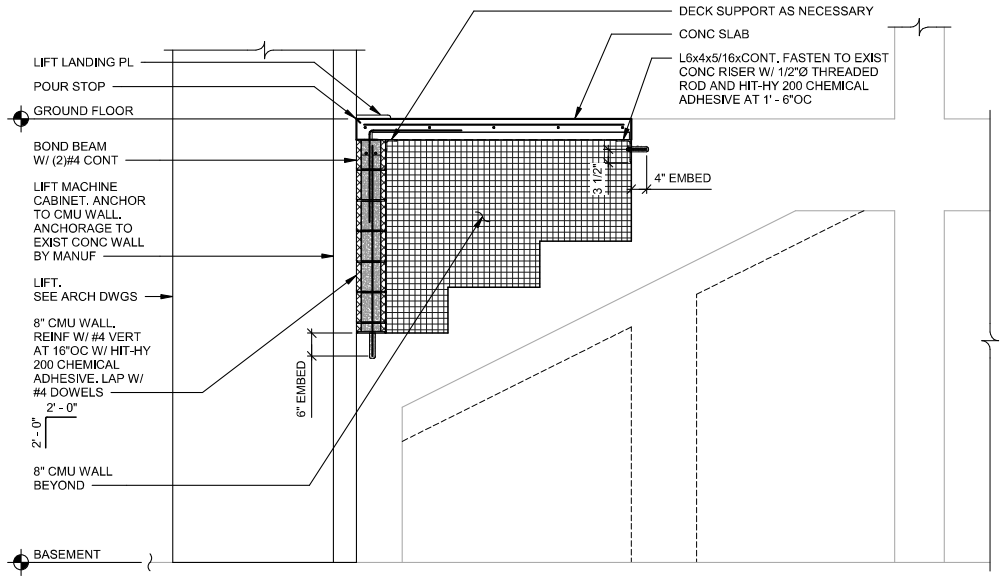
NOTE:
SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

PARTIAL PLANS AND DETAILS

S200



EXISTING / DEMOLITION



PROPOSED

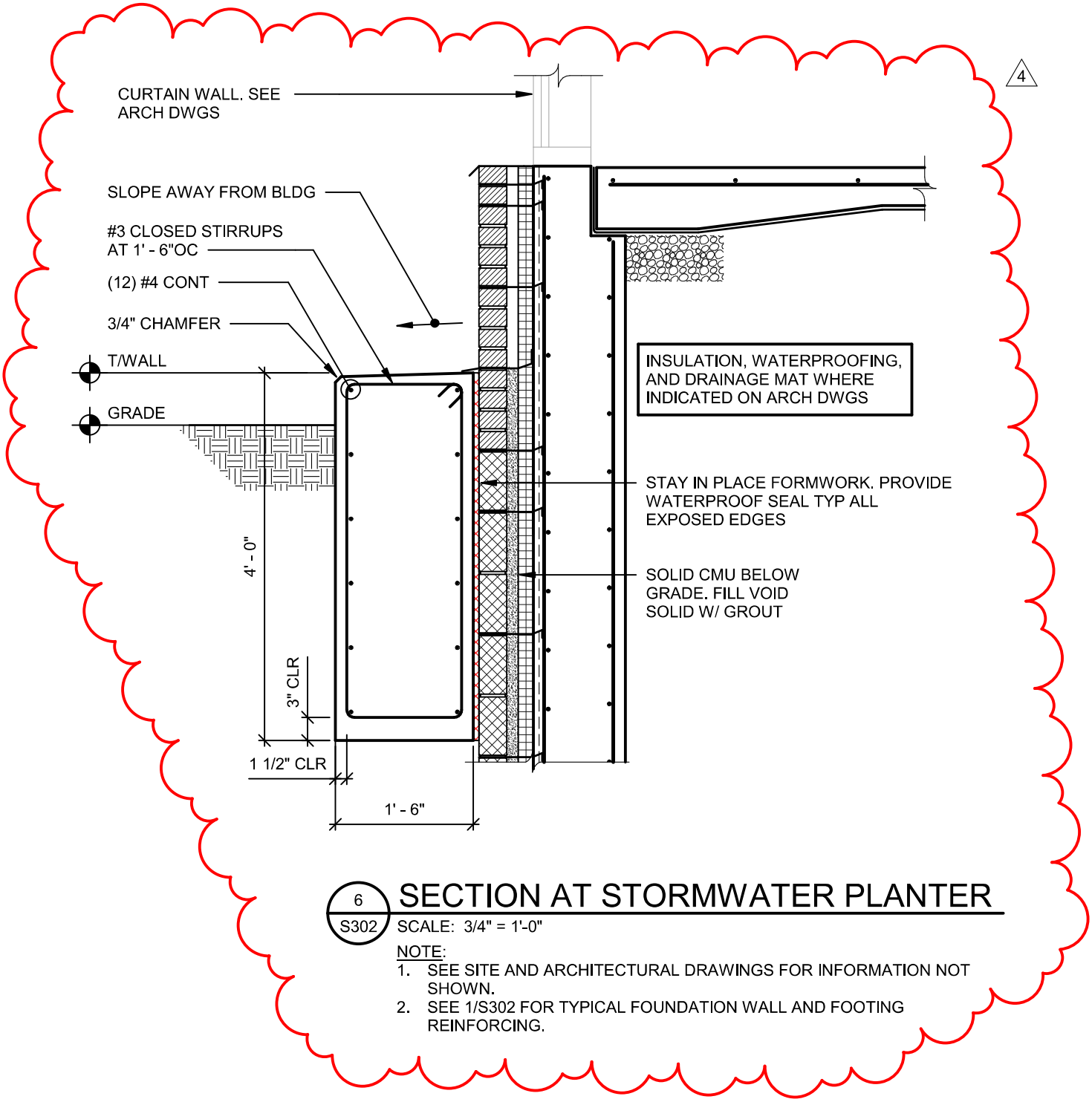
4 / 12 / S200 SECTION
SCALE: 1/4" = 1'-0"

NOTE:
1. CONTRACTOR TO COORDINATE ALL DIMENSIONS WITH LIFT MANUFACTURER.
2. PROVIDE MACHINE CABINET ANCHORS AT ALL ELEVATIONS PER MANUFACTURER'S STANDARD SPACING. ANCHORS SHALL BE COMPATIBLE WITH CONCRETE AND GROUTED CMU.

PARTIAL PLANS AND DETAILS

BID ADDENDUM 4 04/17/2026

S200



6 / S302 SECTION AT STORMWATER PLANTER

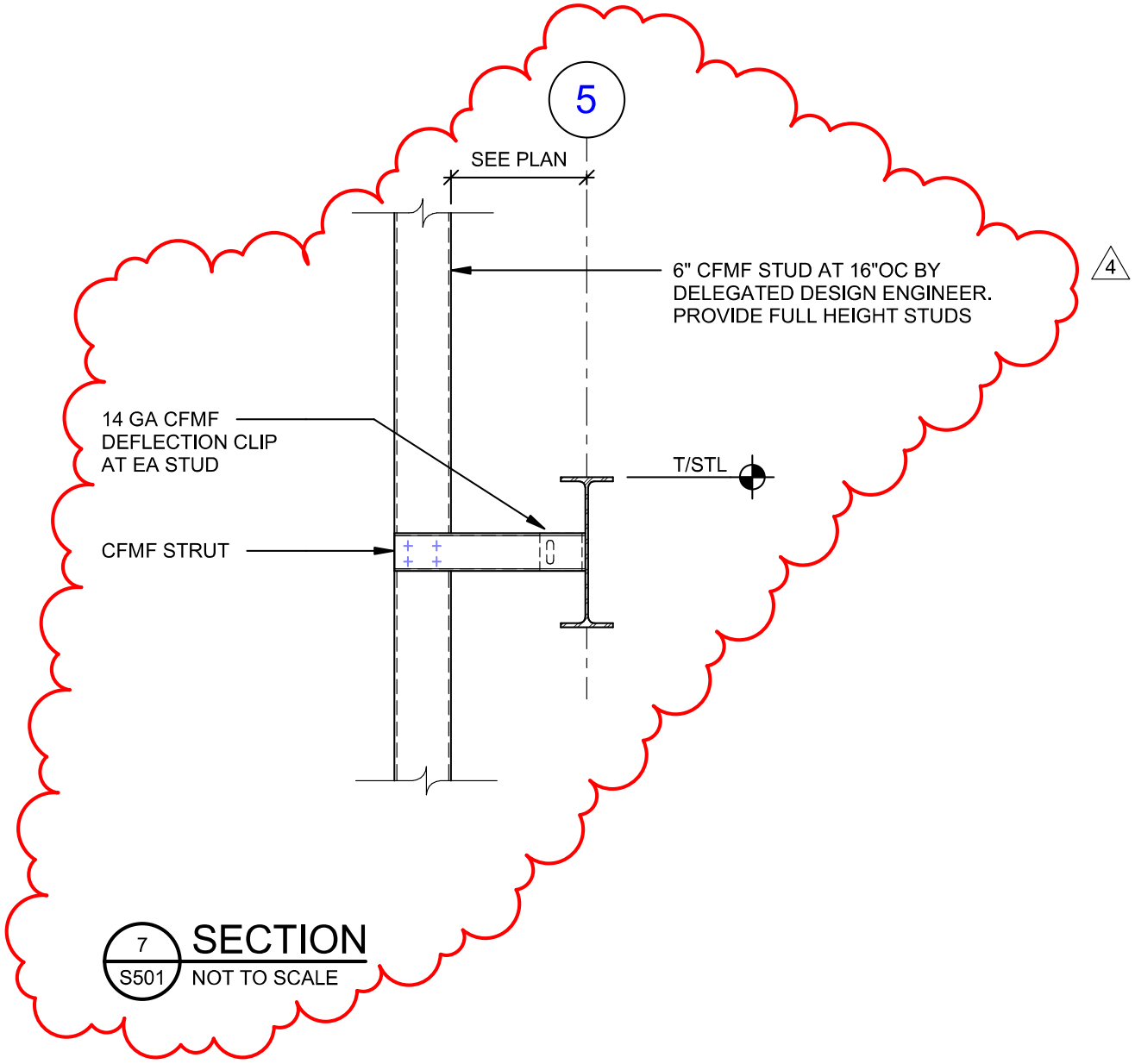
SCALE: 3/4" = 1'-0"

NOTE:

1. SEE SITE AND ARCHITECTURAL DRAWINGS FOR INFORMATION NOT SHOWN.
2. SEE 1/S302 FOR TYPICAL FOUNDATION WALL AND FOOTING REINFORCING.

CONCRETE SECTIONS AND DETAILS

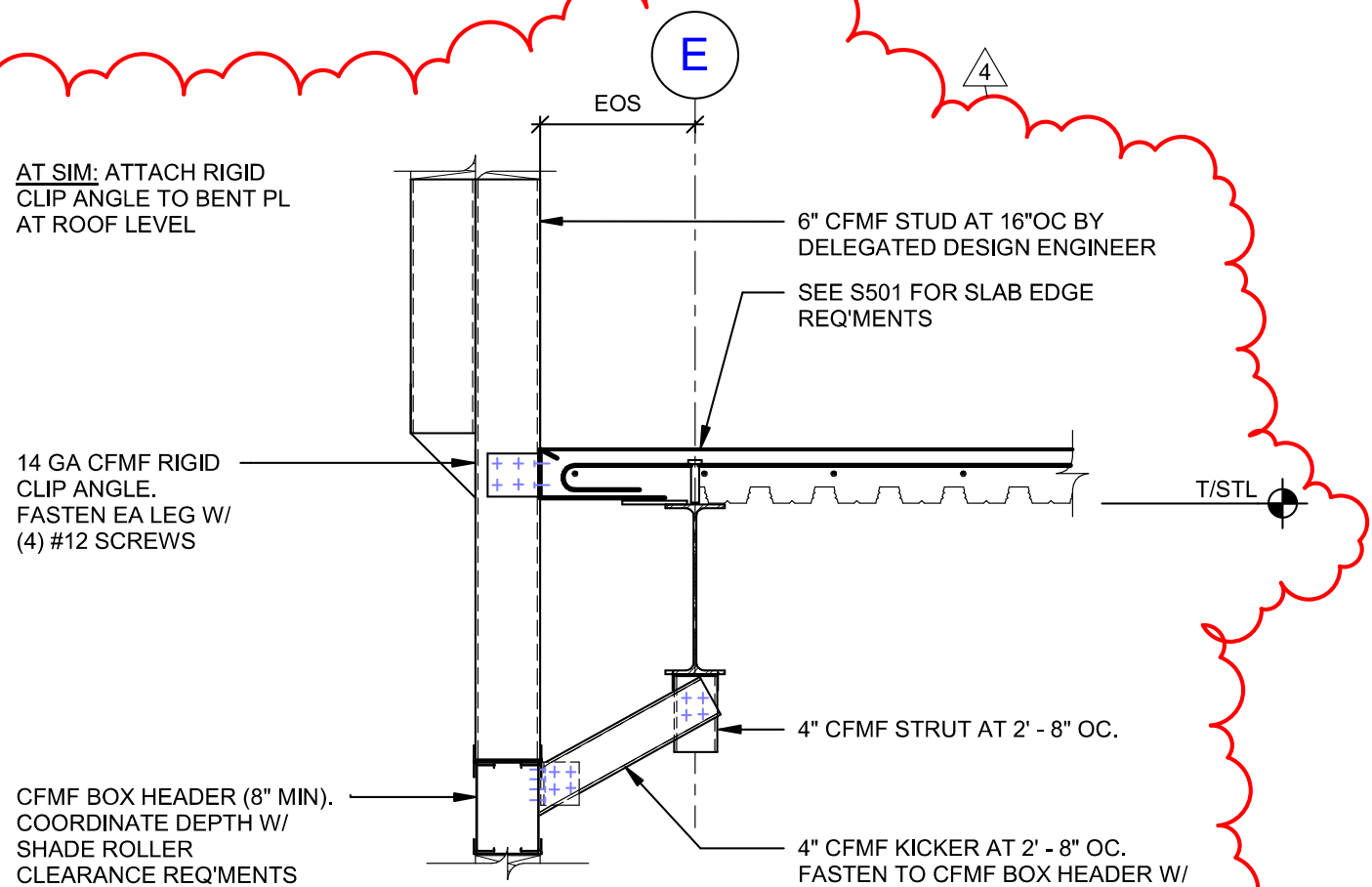
S302



STEEL SECTIONS
AND DETAILS

S501

14585.5



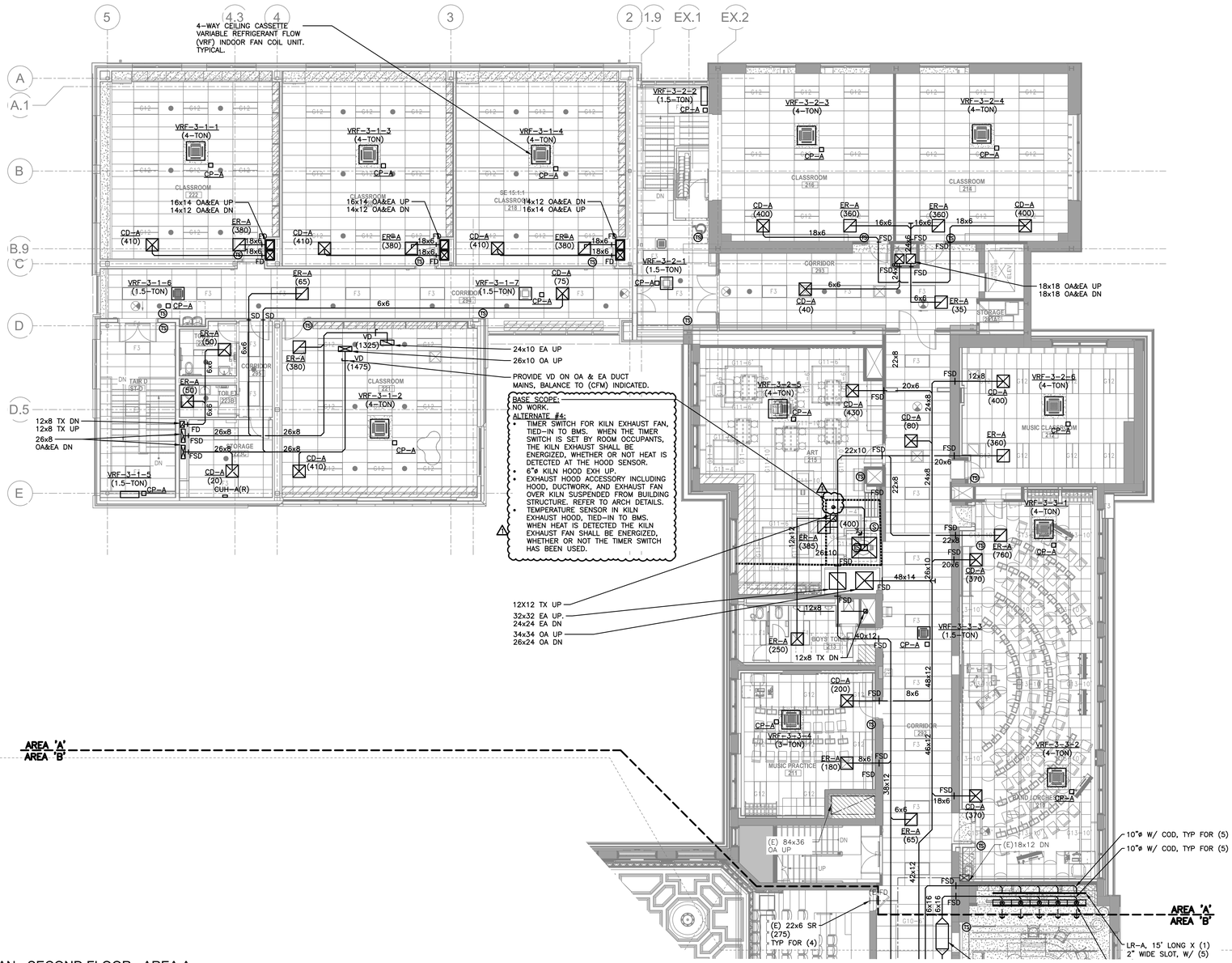
7 / S502 SECTION

NOT TO SCALE

NOTE:
COORDINATE CFMF WALL DIMENSION AT FACADE SECTION BEYOND WITH CLEARANCE REQUIREMENTS AND RIGID CLIP ANGLE DIMENSIONS. SEE 2/S502.

SECTIONS AND DETAILS

S502



MECHANICAL DUCTWORK PART PLAN - SECOND FLOOR - AREA A

1/8" = 1'-0"

GENERAL NOTES – DUCTWORK:

A. BRANCH DUCTS TO INDIVIDUAL DIFFUSERS AND REGISTERS SHALL BE THE SAME SIZE AS THE DIFFUSER OR REGISTER NECK, UNLESS OTHERWISE NOTED.

B. ALL DUCTWORK SHALL BE INSTALLED TIGHT TO BOTTOM OF STRUCTURAL MEMBERS UNLESS OTHERWISE NOTED OR ABSOLUTELY REQUIRED BY FIELD CONDITIONS.

C. DO NOT INSTALL DUCTWORK DIRECTLY UNDER AND PARALLEL TO THE WEB OF STRUCTURAL MEMBERS. OFFSET IN ORDER TO ALLOW FUTURE DUCTWORK AND PIPING TO CROSS OVER IN BETWEEN STRUCTURAL MEMBERS.

D. BRANCH DUCTS TO INDIVIDUAL DIFFUSERS AND REGISTERS SHALL BE PROVIDED WITH VOLUME DAMPERS, WHETHER OR NOT THE VOLUME DAMPERS ARE SHOWN ON PLAN.

E. VOLUME DAMPERS LOCATED ABOVE INACCESSIBLE CEILINGS SHALL BE CABLE OPERATED TYPE, WITH CABLE OPERATORS LOCATED IN ACCESSIBLE LOCATIONS AND CLEARLY LABELED FOR DIFFUSER OR REGISTER SERVED.

F. UNLESS OTHERWISE NOTED, ALL EXPOSED DUCTWORK IN FINISHED SPACES SHALL BE SPIRAL ROUND OR FLAT OVAL TYPE, WITH SOLID OUTER WALL, PERFORATED INNER WALL, 1 INCH THICK INTERSTITIAL ACOUSTICAL LINING, AND FLAT SEAMS.

GENERAL NOTES – PIPING:

G. ALL PIPING SHALL BE INSTALLED TIGHT TO BOTTOM OF STRUCTURAL MEMBERS UNLESS OTHERWISE NOTED OR ABSOLUTELY REQUIRED BY FIELD CONDITIONS.

H. DO NOT INSTALL PIPING DIRECTLY UNDER AND PARALLEL TO THE WEB OF STRUCTURAL MEMBERS. OFFSET IN ORDER TO ALLOW FUTURE DUCTWORK AND PIPING TO CROSS OVER IN BETWEEN STRUCTURAL MEMBERS.

I. CONDENSATE DRAIN (CD) AND CONDENSATE PUMP DISCHARGE (PD) PIPING SHALL BE RIGID COPPER, TYPE L, MINIMUM 3/4" NOMINAL PIPE SIZE, BRAZED OR SOLDERED, WITH 1" INSULATION, UNLESS OTHERWISE NOTED ON DRAWINGS.

J. ALL NEW AND EXISTING INSULATED HVAC PIPING EXPOSED TO VIEW IN FINSHED SPACES SHALL BE PROVIDED WITH PVC JACKETS.

GENERAL NOTES – CUTTING AND PATCHING:

H. WHERE EXISTING DUCTS, PIPES, LOUVERS, GRILLES, WIRES, CONDUITS, AND PNEUMATIC TUBING THROUGH EXISTING WALLS, PARTITIONS, SHAFTS, CHASES, AND SLABS ARE REMOVED BY THE MECHANICAL CONTRACTOR, THE MECHANICAL CONTRACTOR SHALL BE RESPONSIBLE FOR INFILLING AND REPAIRING OPENINGS TO MATCH EXISTING CONSTRUCTION, INCLUDING FIRE RATING, SMOKE RATING, INSULATION VALUE, MOISTURE BARRIER, PAINTING, AND GENERAL FINISH APPEARANCE.

I. WHERE NEW DUCTS, PIPES, LOUVERS, GRILLES, WIRES, AND CONDUITS INSTALLED BY THE MECHANICAL CONTRACTOR PENETRATE EXISTING WALLS, PARTITIONS, SHAFTS, CHASES, AND SLABS, THE MECHANICAL CONTRACTOR SHALL BE RESPONSIBLE FOR CUTTING NEW OPENINGS AND FIRESTOPPING. WHERE WALL PENETRATIONS WILL BE 8" OR GREATER IN WIDTH, THE MECHANICAL CONTRACTOR SHALL PROVIDE A NEW LINTEL AS PER DETAIL ON STRUCTURAL DRAWING S402.

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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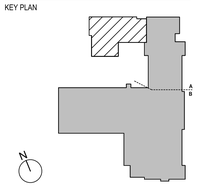
BRIDGEWATER, MA. 02324

ISSUE	DATE
⚠1 ADDENDUM #4	4/17/2026
ISSUED FOR BID	03/27/2026

KEY PLAN

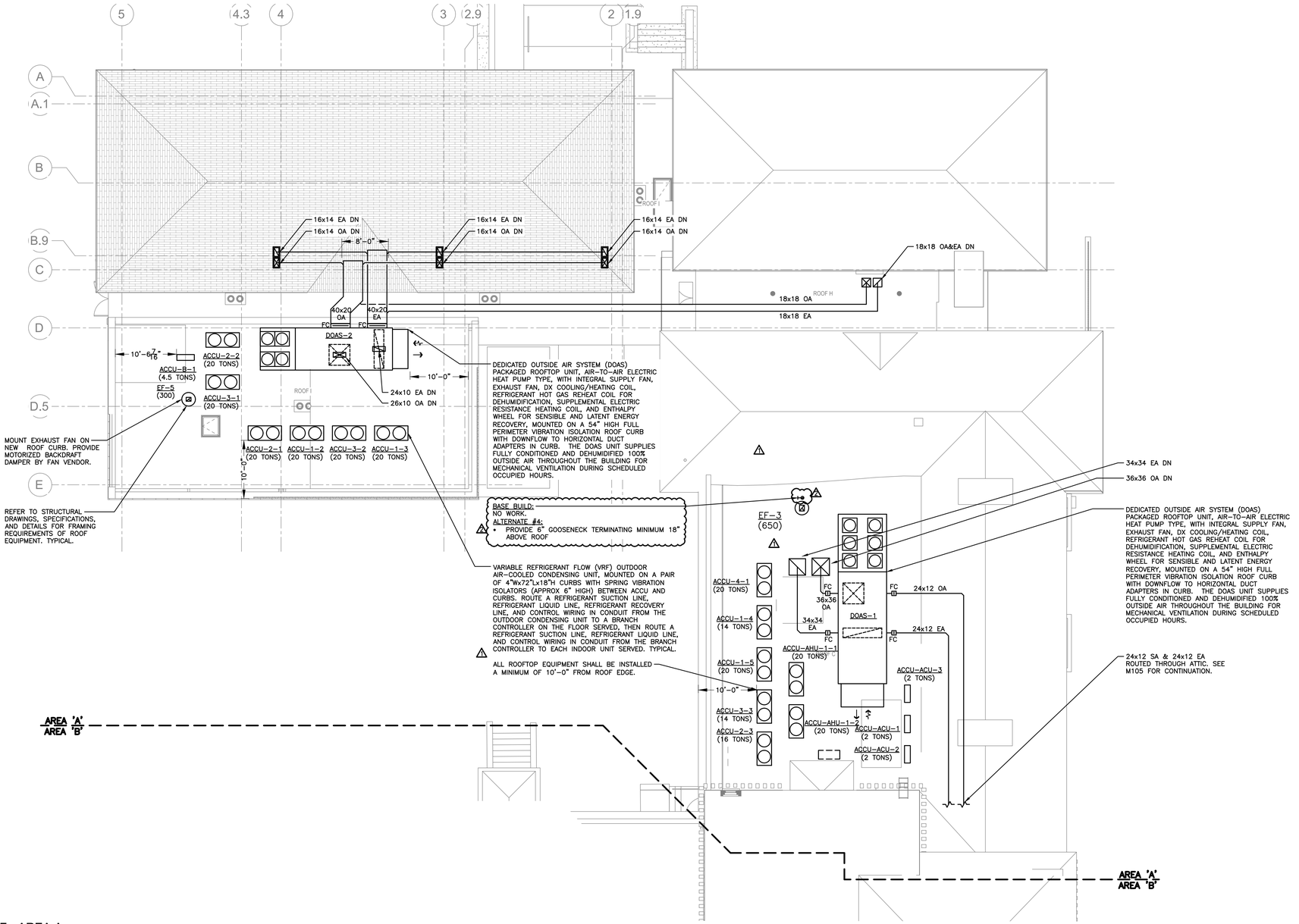


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

MECHANICAL DUCTWORK PART PLAN - SECOND FLOOR - AREA A

M104-A



MECHANICAL PART PLAN - ROOF - AREA A
1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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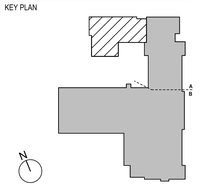

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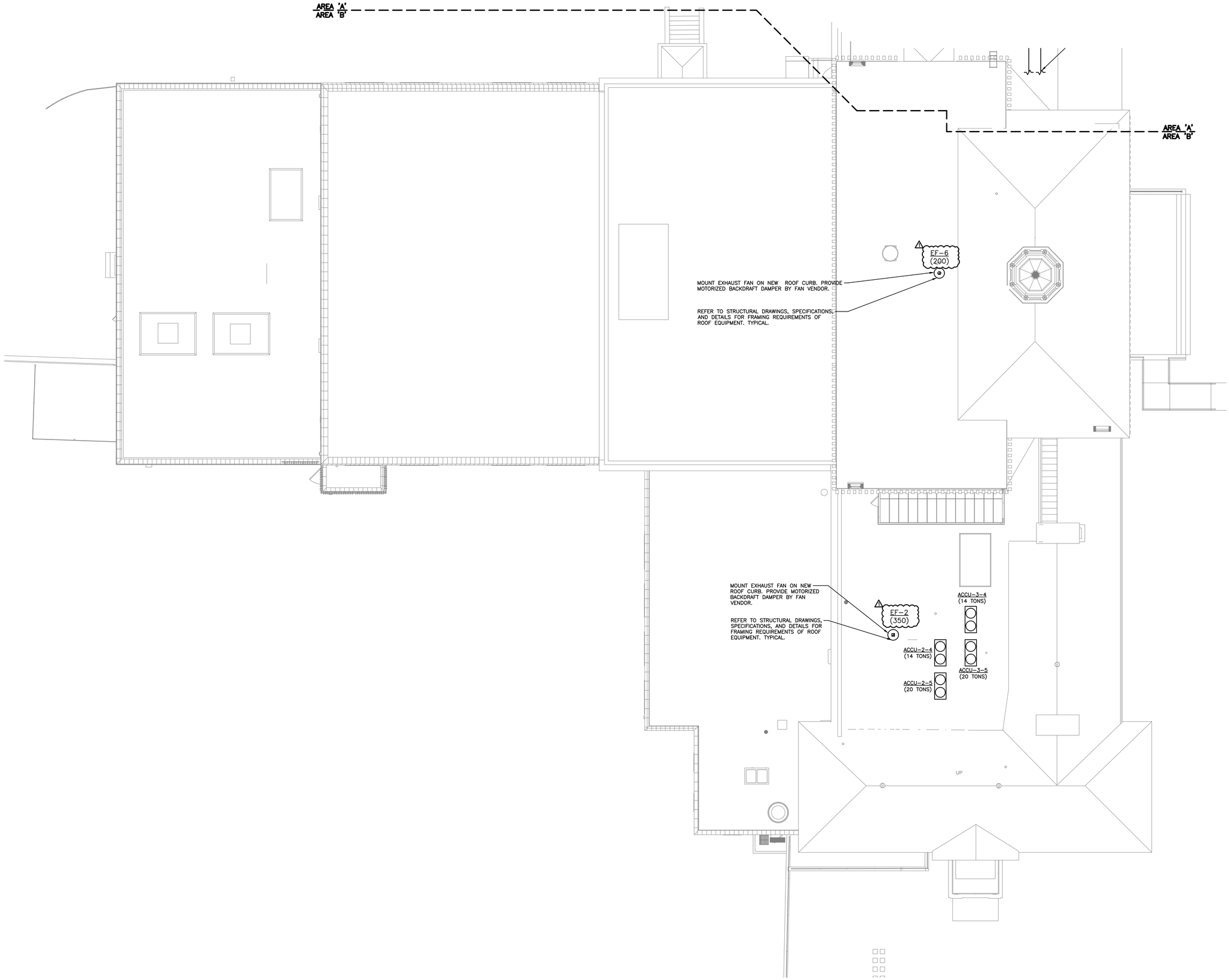
MEMASI PROJECT NO.	101-2403

MECHANICAL PART PLAN - ROOF - AREA A

M106-A



MECHANICAL PART PLAN - ROOF - AREA B

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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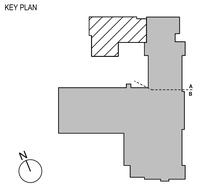

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MEMASI PROJECT NO.	101-2403

MECHANICAL PART PLAN - ROOF - AREA B

M106-B

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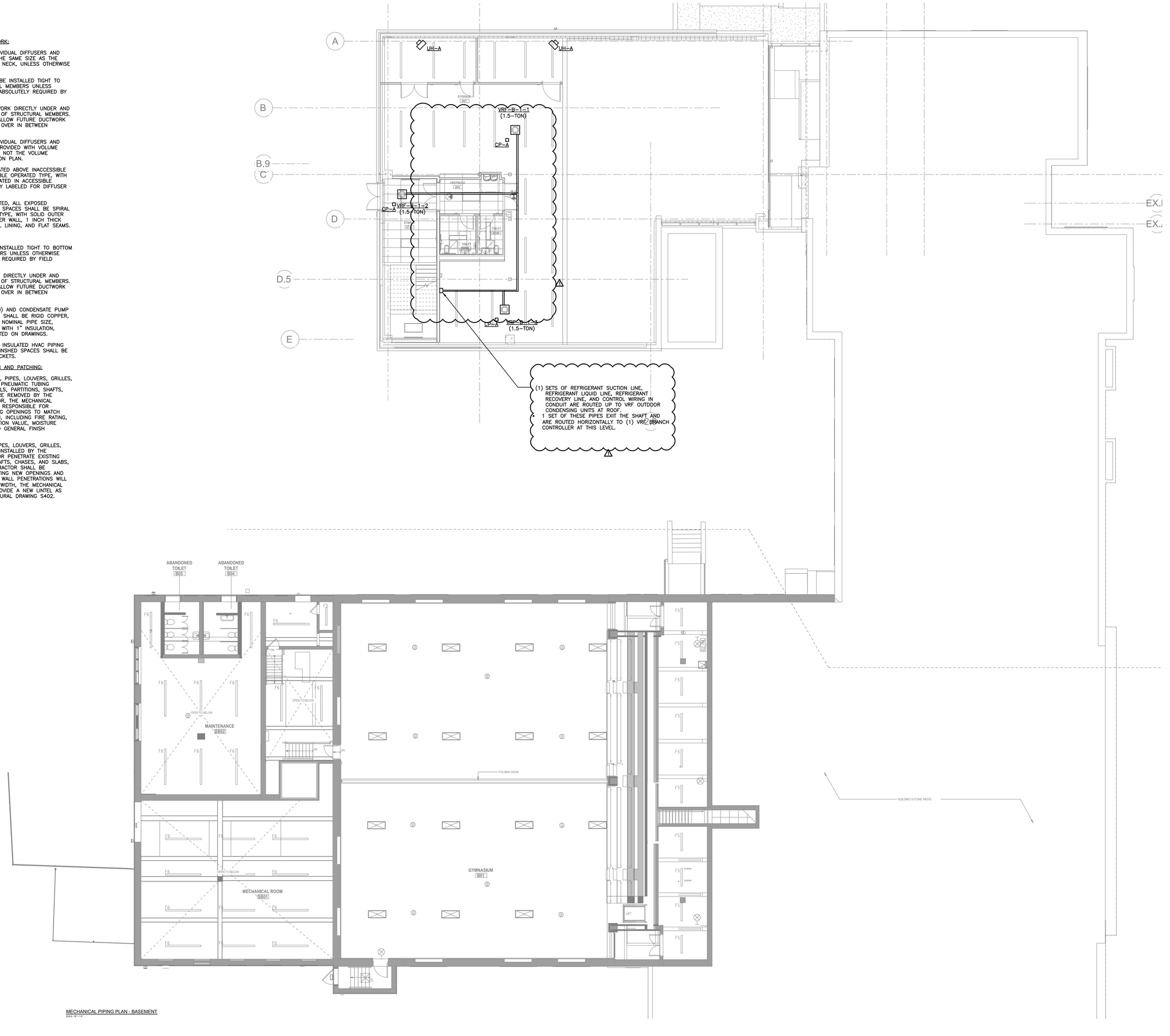

MECHANICAL PIPING PLAN - BASEMENT
SCALE: 1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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BRIDGEWATER, MA. 02324

ISSUE	DATE
⚠ ADDENDUM #4	4/17/2026
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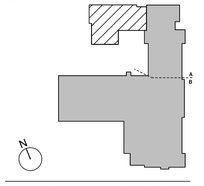
KEY PLAN

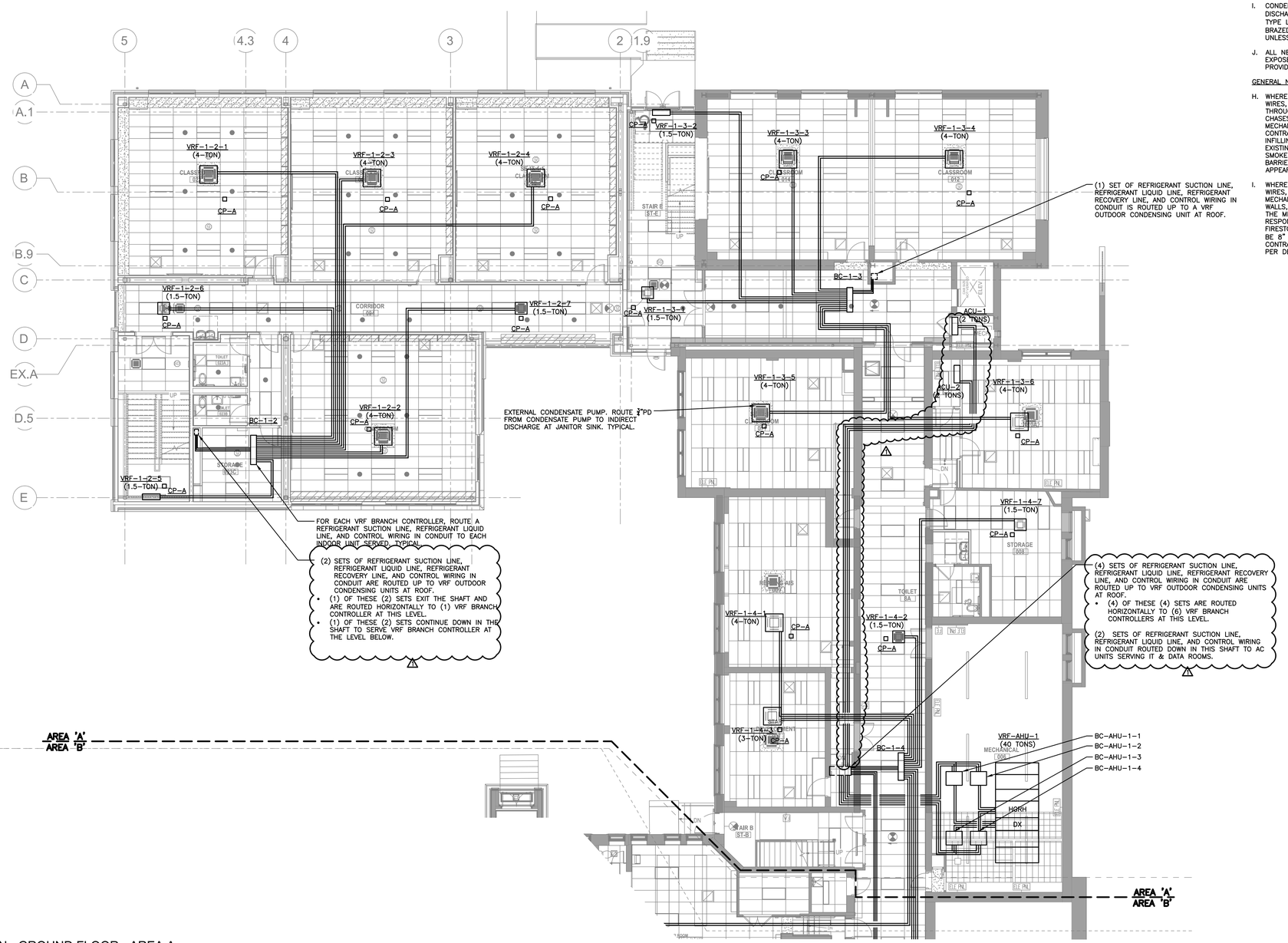


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

MECHANICAL PIPING PLAN - BASEMENT

M201



MECHANICAL PIPING PART PLAN - GROUND FLOOR - AREA A

1/8" = 1'-0"

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PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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BRIDGEWATER, MA. 02324

ISSUE	DATE
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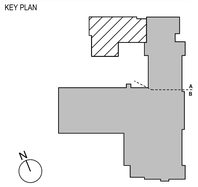
ISSUED FOR BID	03/27/2026

KEY PLAN

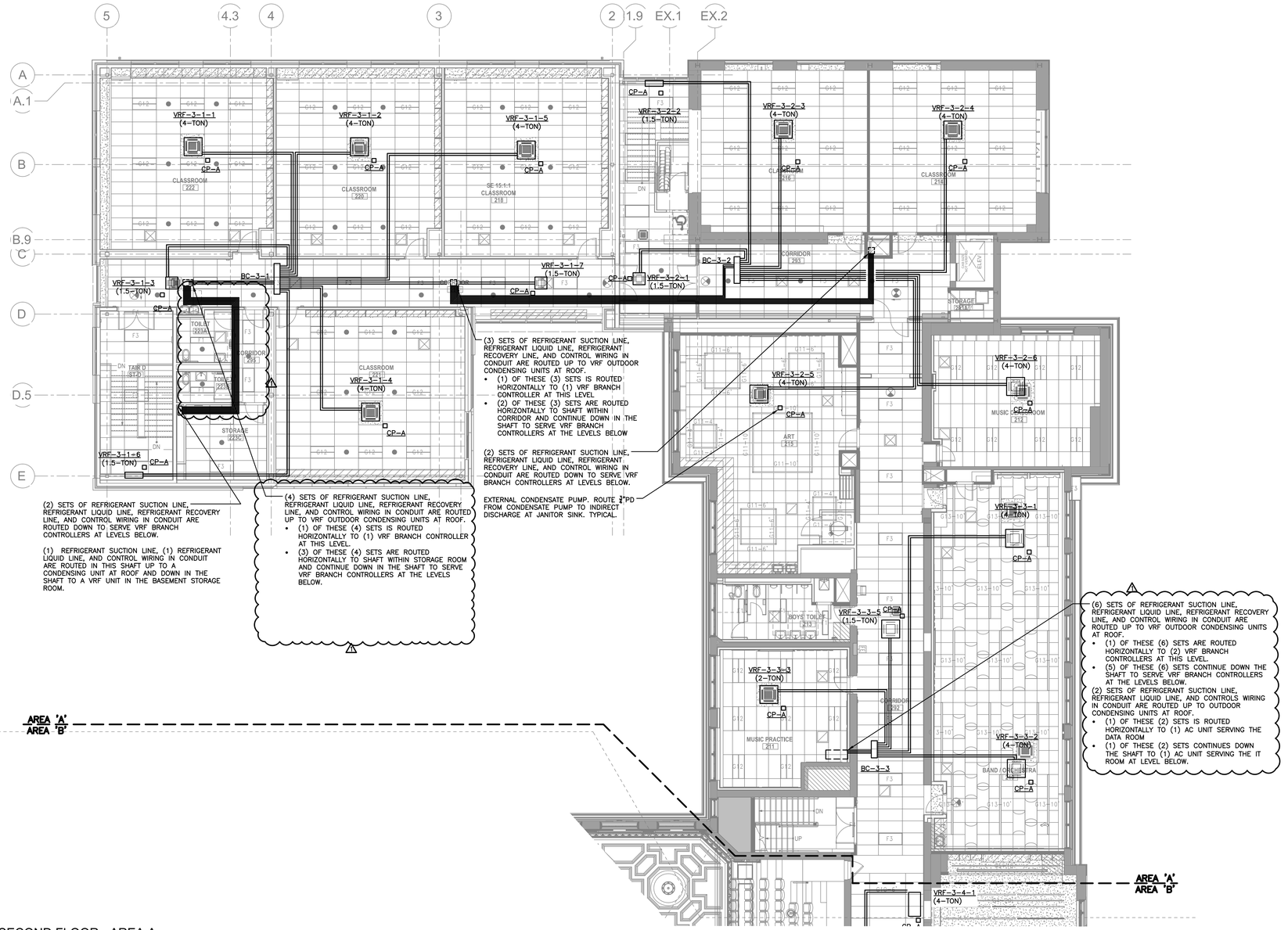


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

MECHANICAL PIPING PART PLAN - GROUND FLOOR - AREA A

M202-A



MECHANICAL PIPING PART PLAN - SECOND FLOOR - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

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BRIDGEWATER, MA. 02324

⚠ ADDENDUM #4	4/17/2026
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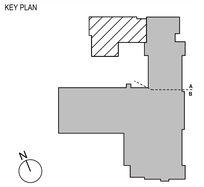
ISSUE	DATE

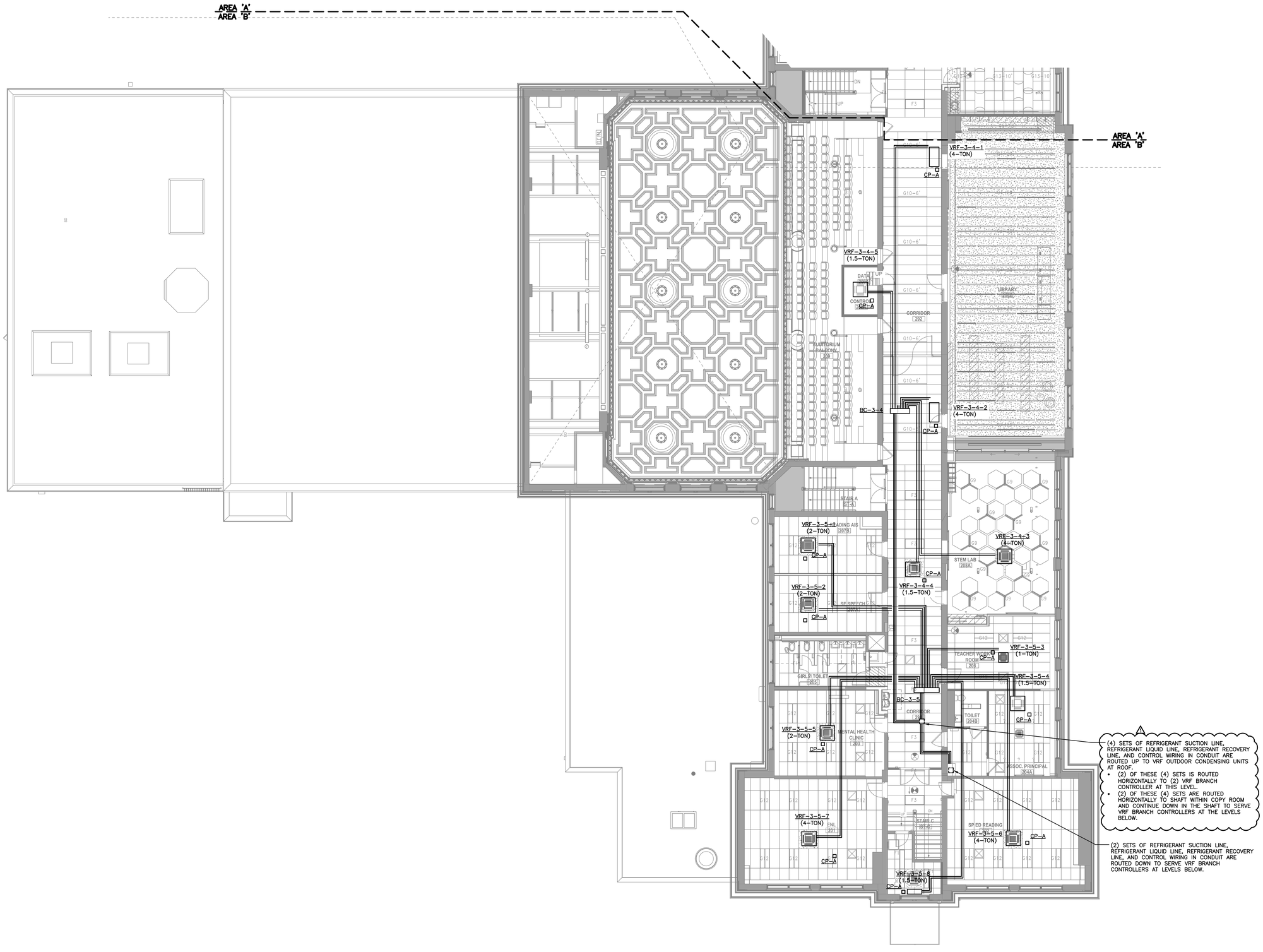
KEY PLAN



SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

MECHANICAL PIPING PART PLAN - SECOND FLOOR - AREA A

M204-A



MECHANICAL PIPING PART PLAN - SECOND FLOOR - AREA B
1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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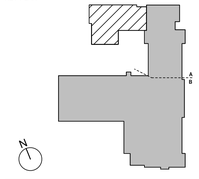

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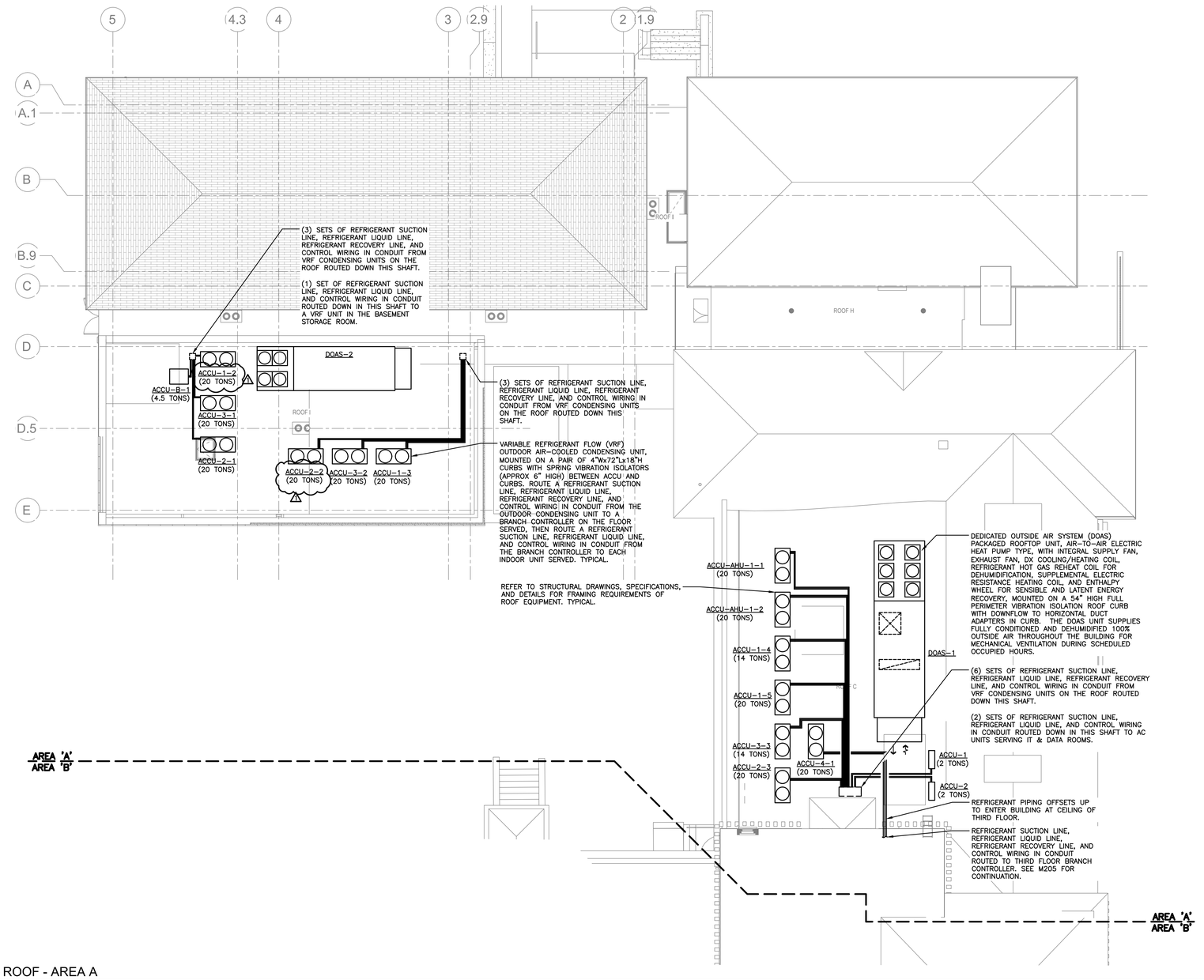
MECHANICAL PIPING PART PLAN - SECOND FLOOR - AREA B

M204-B



MECHANICAL PIPING PART PLAN - ROOF - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

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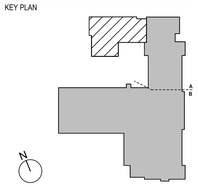

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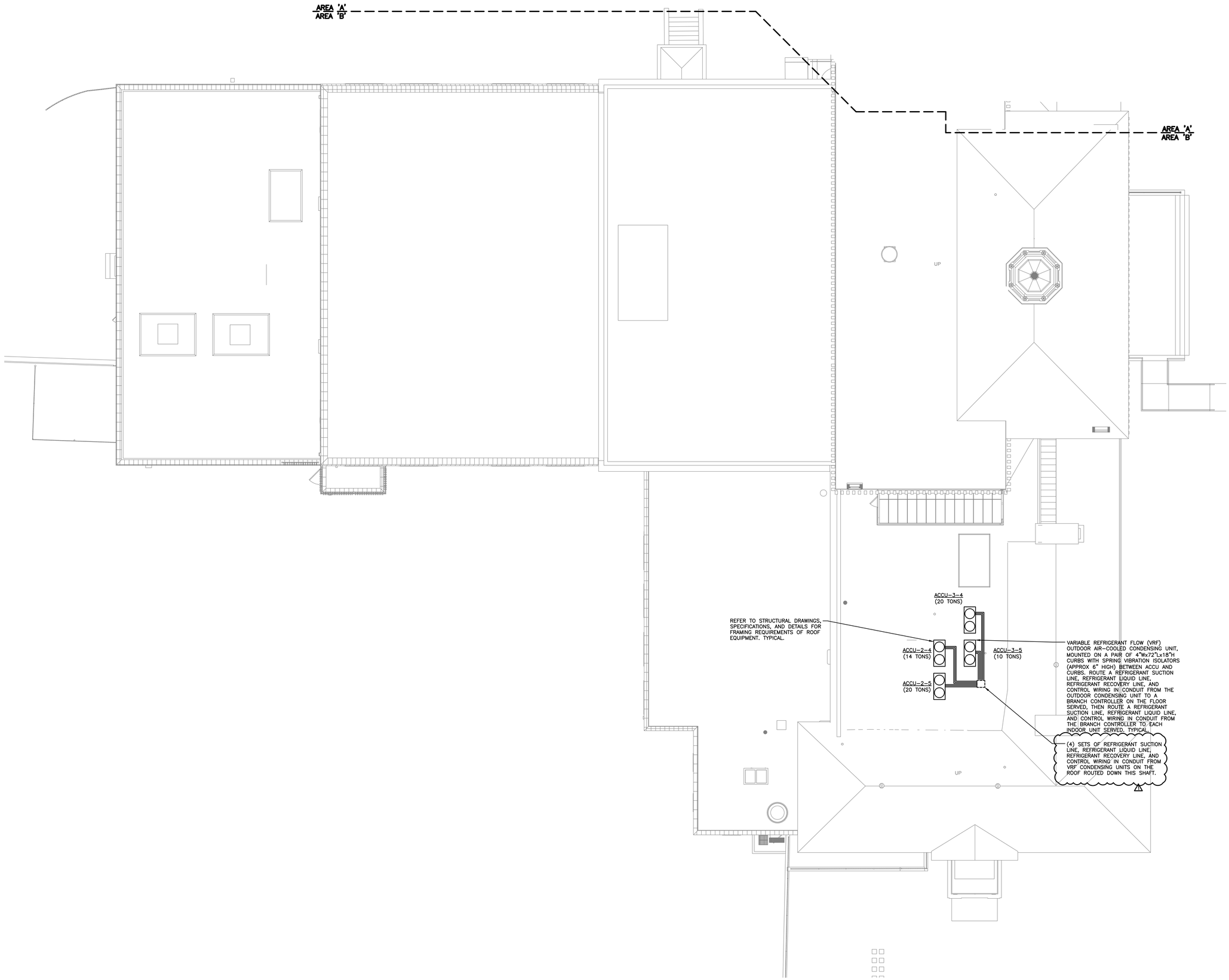
MEMASI PROJECT NO. 101-2403

MECHANICAL PIPING PART PLAN - ROOF - AREA A

M206-A



MECHANICAL PIPING PART PLAN - ROOF - AREA B

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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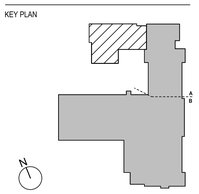

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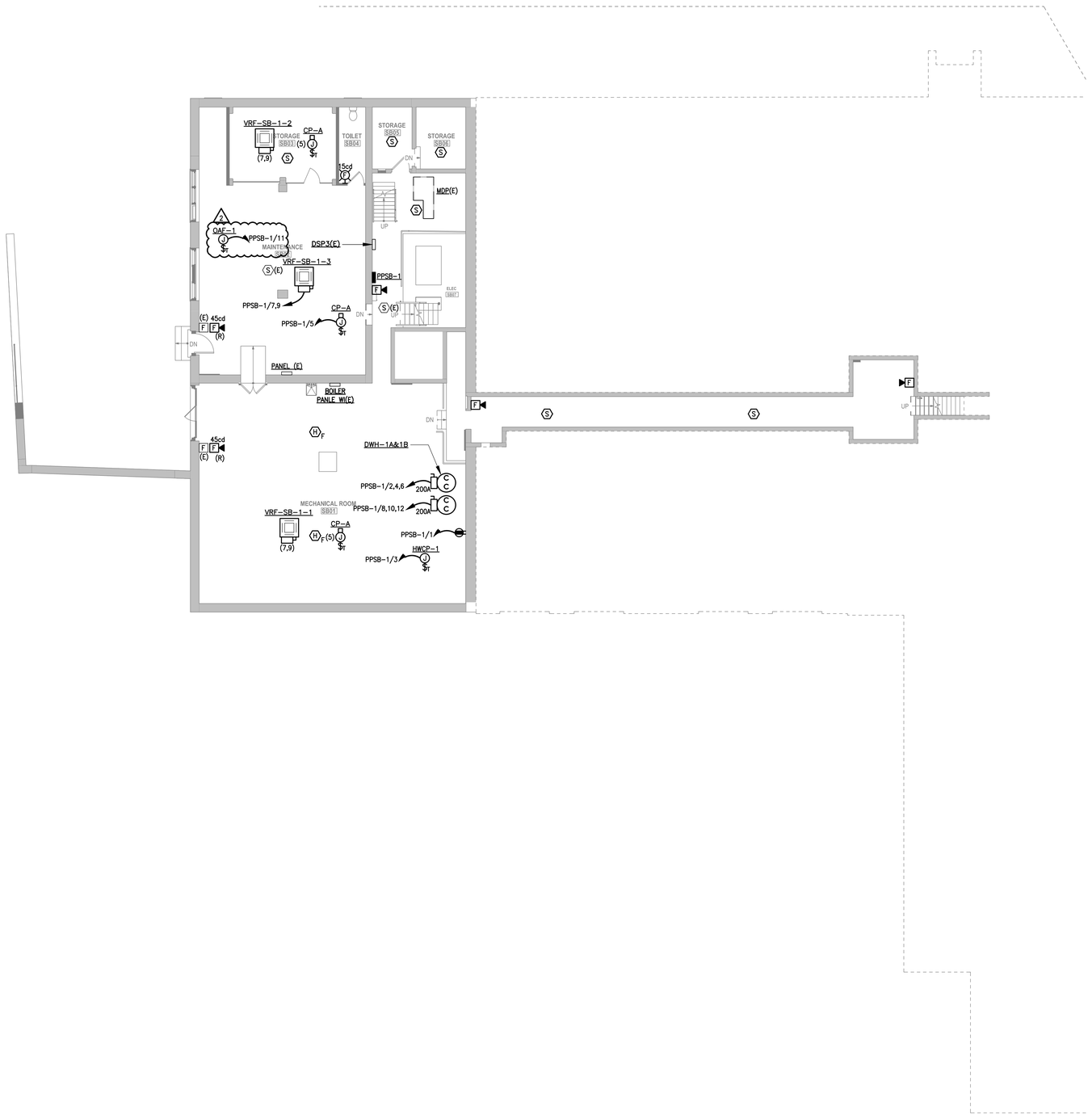
MEMASI PROJECT NO. 101-2403

MECHANICAL PIPING PART PLAN - ROOF - AREA B

M206-B

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.



ELECTRICAL POWER PLAN - SUB-BASEMENT

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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⚠2 ADDENDUM #4	04/17/2026
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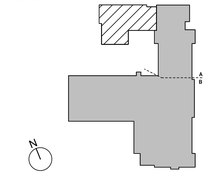
KEY PLAN



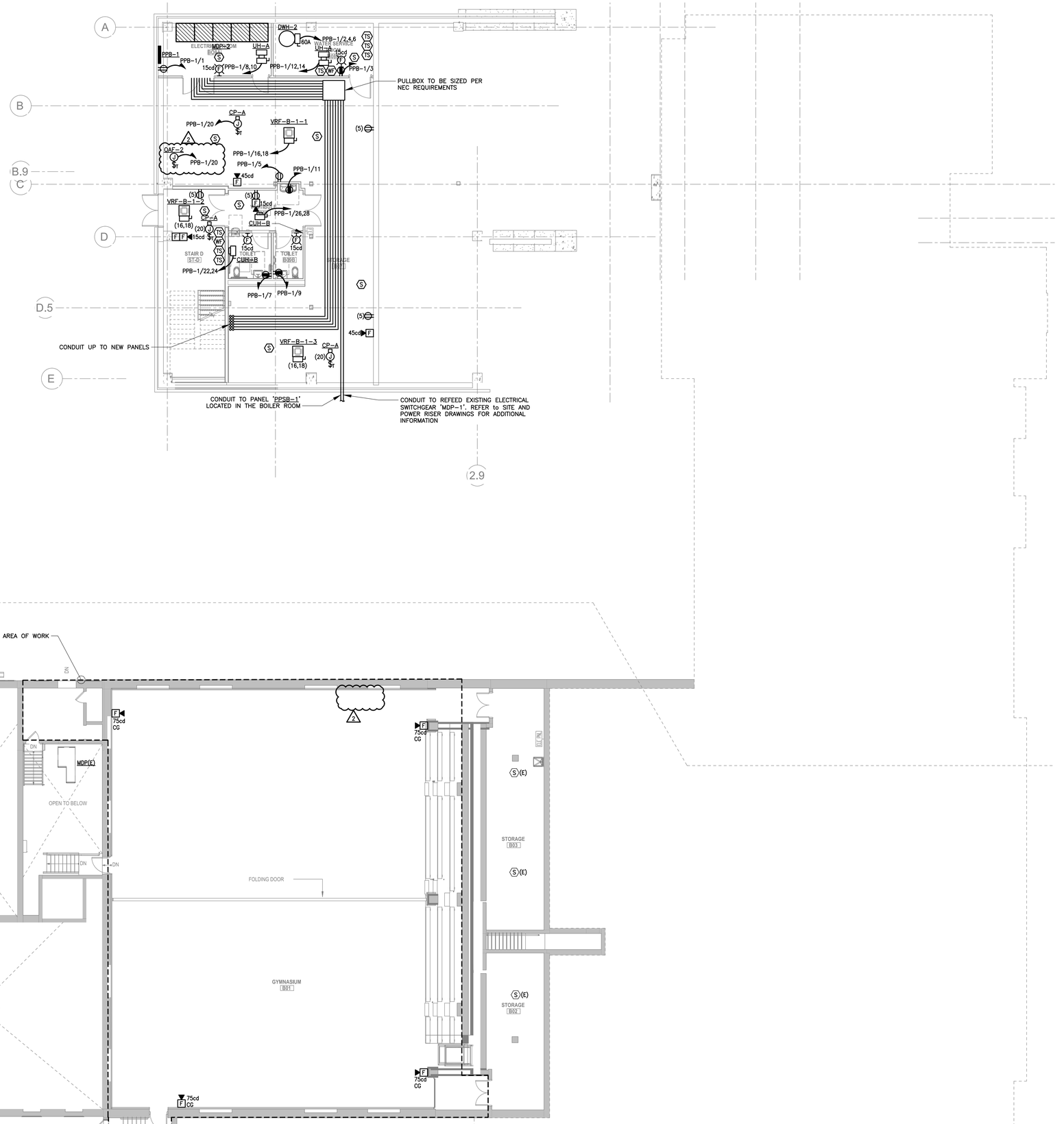
SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

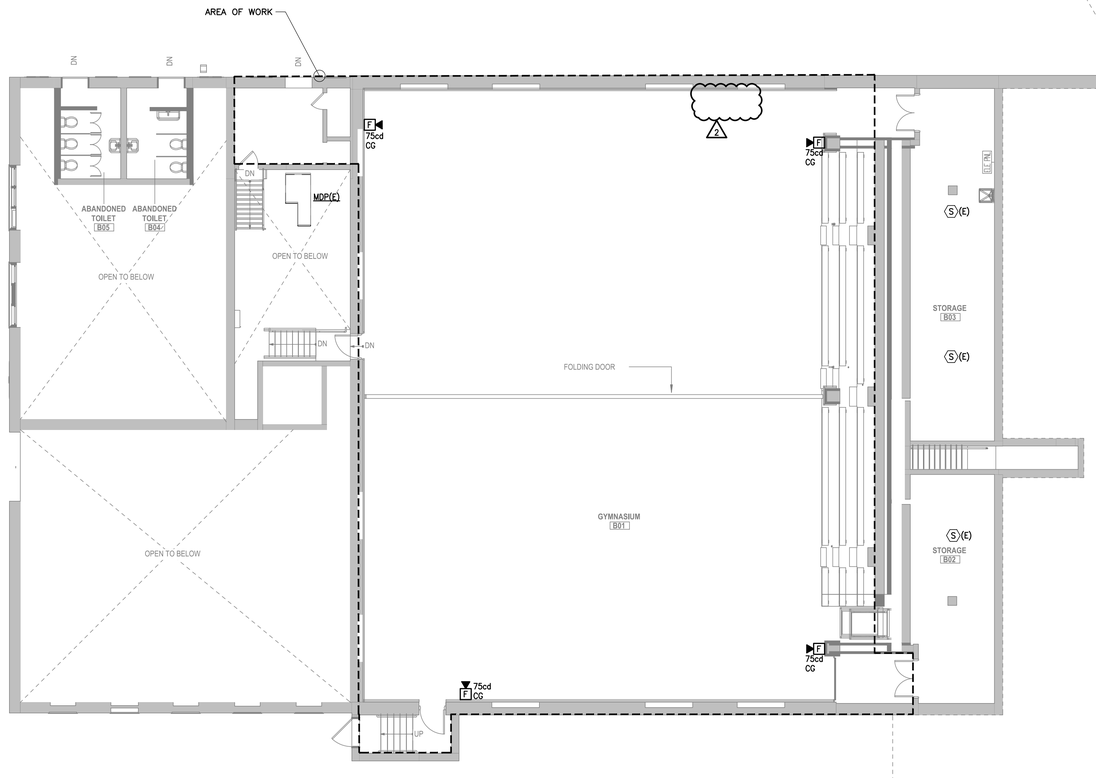
ELECTRICAL POWER PLAN - SUB-BASEMENT

E100

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.





ELECTRICAL POWER PLAN - BASEMENT
1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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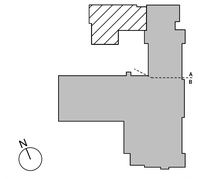


SED PROJECT NO. 28-05-01-06-1-009-XXX
MEMASI PROJECT NO. 101-2403

E101

POWER PLAN NOTES:

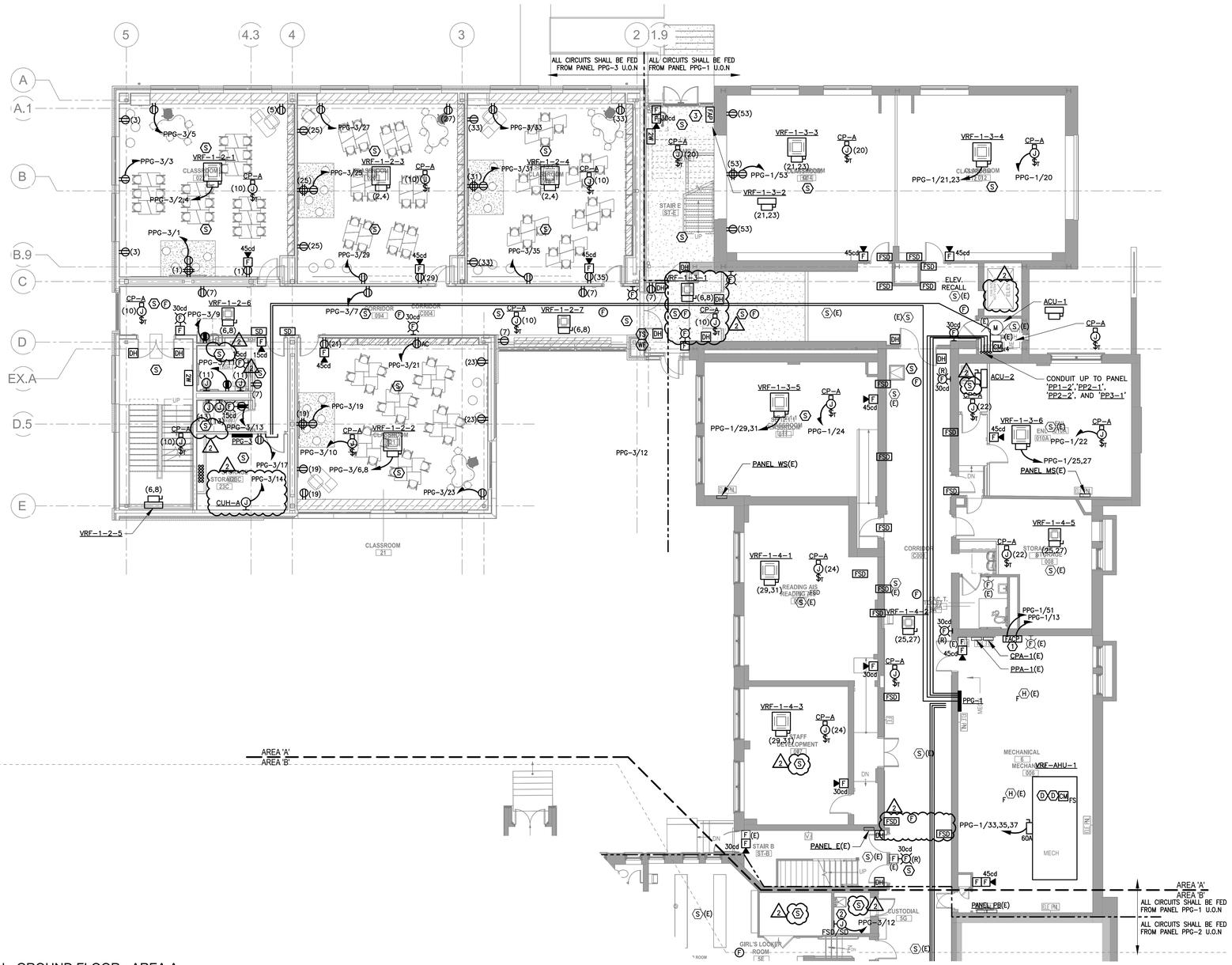
1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.

POWER PLAN KEY NOTES:

(1) PROVIDE VOICE CAPABLE EDWARDS EST4 ALARM CONTROL PANEL AT INDICATED LOCATION TO REPLACE EXISTING EST3. TIE ALL EXISTING INITIATING AND NOTIFICATION FIELD DEVICES AND CIRCUITS THAT ARE INDICATED TO REMAIN INTO THE NEW EDWARDS EST4 FIRE ALARM CONTROL PANEL. ALL NEW ADDITION AREA DEVICES TO BE TIED INTO THE NEW EDWARDS EST4 FIRE ALARM CONTROL PANEL. CONTACT NICHOLAS DELFICIO FROM EVERON SOLUTIONS (914)-403-2697 FOR ADDITIONAL COORDINATION.

(2) PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.

(3) REPLACE EXISTING ANNUNCIATOR PANEL WITH NEW EDWARDS EST4 REMOTE LCD ANNUNCIATOR. PROVIDE ADJACENT TO PANEL A FRAMED GRAPHIC PLAN OF BUILDING WITH FIRE ALARM EQUIPMENT IN ACCORDANCE WITH NYS-SED REQUIREMENTS APPROVED BY THE LOCAL FIRE MARSHALL.



ELECTRICAL POWER PART PLAN - GROUND FLOOR - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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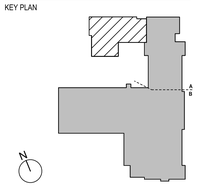

SED PROJECT NO.	28-05-01-06-1-009-XXX
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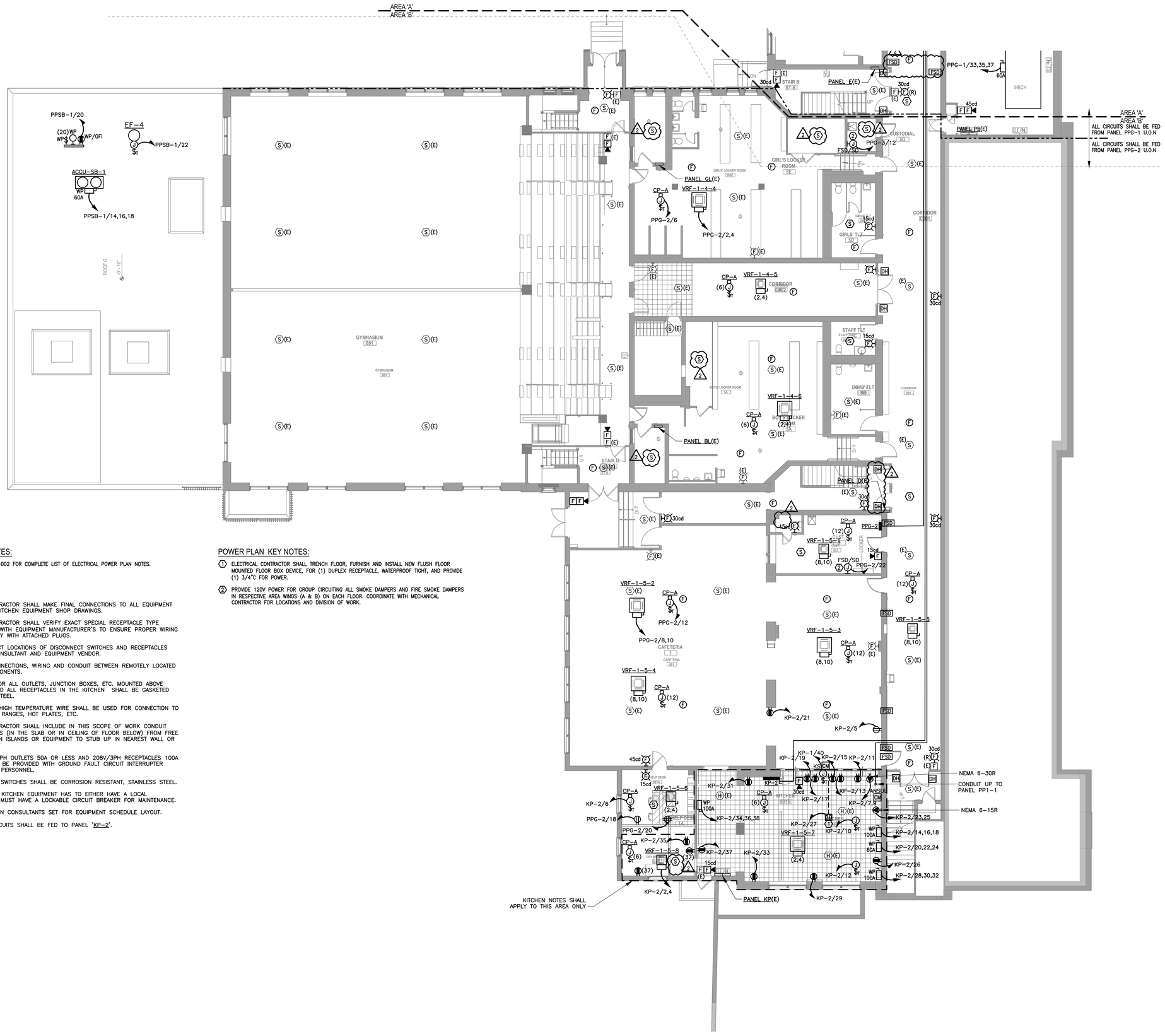
ELECTRICAL POWER PART PLAN - GROUND FLOOR - AREA A

E102-A



POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.

KITCHEN NOTES:

1. ELECTRICAL CONTRACTOR SHALL MAKE FINAL CONNECTIONS TO ALL EQUIPMENT PER APPROVED KITCHEN EQUIPMENT SHOP DRAWINGS.
2. ELECTRICAL CONTRACTOR SHALL VERIFY EXACT SPECIAL RECEPTACLE TYPE CONFIGURATIONS WITH EQUIPMENT MANUFACTURER'S TO ENSURE PROPER WIRING AND COMPATIBILITY WITH ATTACHED PLUGS.
3. COORDINATE EXACT LOCATIONS OF DISCONNECT SWITCHES AND RECEPTACLES WITH KITCHEN CONSULTANT AND EQUIPMENT VENDOR.
4. PROVIDE ALL CONNECTIONS, WIRING AND CONDUIT BETWEEN REMOTELY LOCATED EQUIPMENT COMPONENTS.
5. COVER PLATES FOR ALL OUTLETS, JUNCTION BOXES, ETC. MOUNTED ABOVE COUNTERTOPS AND ALL RECEPTACLES IN THE KITCHEN SHALL BE GASKETED AND STAINLESS STEEL.
6. APPROVED TYPE HIGH TEMPERATURE WIRE SHALL BE USED FOR CONNECTION TO HOOD LIGHTING, RANGES, HOT PLATES, ETC.
7. ELECTRICAL CONTRACTOR SHALL INCLUDE IN THIS SCOPE OF WORK CONDUIT INTERCONNECTIONS (IN THE SLAB OR IN CEILING OF FLOOR BELOW) FROM FREE STANDING KITCHEN ISLANDS OR EQUIPMENT TO STUB UP IN NEAREST WALL OR COLUMN.
8. ALL 120/208V-1PH OUTLETS 50A OR LESS AND 208V/3PH RECEPTACLES 100A OR LESS SHALL BE PROVIDED WITH GROUND FAULT CIRCUIT INTERRUPTER PROTECTION FOR PERSONNEL.
9. ALL DISCONNECT SWITCHES SHALL BE CORROSION RESISTANT, STAINLESS STEEL.
10. ALL HARD WIRED KITCHEN EQUIPMENT HAS TO EITHER HAVE A LOCAL DISCONNECT, OR MUST HAVE A LOCKABLE CIRCUIT BREAKER FOR MAINTENANCE.
11. REFER TO KITCHEN CONSULTANTS SET FOR EQUIPMENT SCHEDULE LAYOUT.
12. ALL KITCHEN CIRCUITS SHALL BE FED TO PANEL 'KP-2'.

POWER PLAN KEY NOTES:

1. ELECTRICAL CONTRACTOR SHALL TRENCH FLOOR, FURNISH AND INSTALL NEW FLUSH FLOOR MOUNTED FLOOR BOX DEVICE, FOR (1) DUPLEX RECEPTACLE, WATERPROOF TIGHT, AND PROVIDE (1) 3/4"C FOR POWER.
2. PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.

ELECTRICAL POWER PART PLAN - GROUND FLOOR - AREA B

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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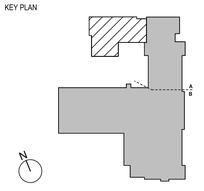

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MEMASI PROJECT NO. 101-2403

ELECTRICAL POWER PART PLAN - GROUND FLOOR - AREA B

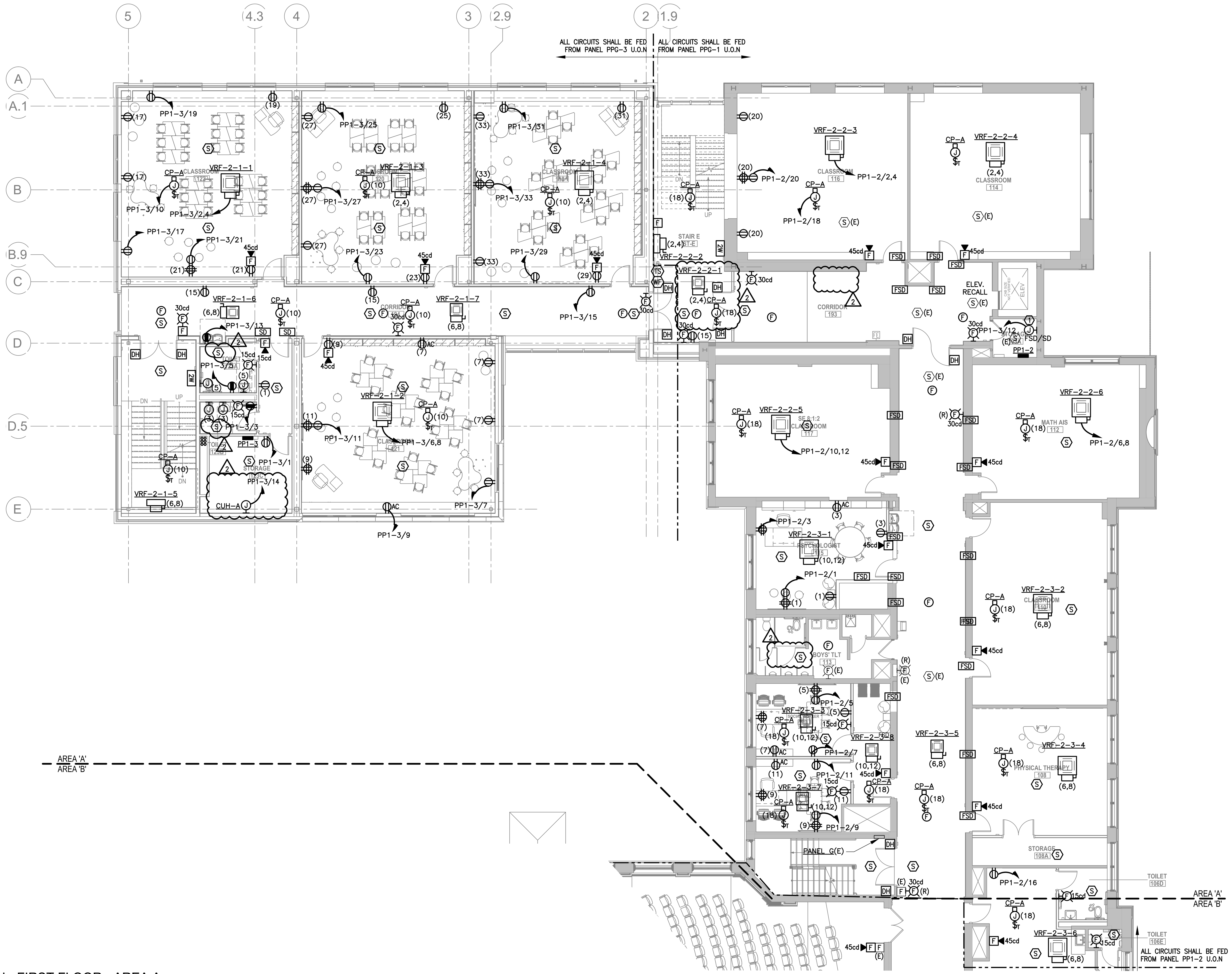
E102-B

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. RECEPTACLE FOR SMARTBOARDS SHALL BE MOUNTED IN A RECESSED BOX. COORDINATE ALL REQUIREMENTS WITH VENDOR AND IT CONSULTANT DRAWINGS.

POWER PLAN KEY NOTES:

(1) PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.



ELECTRICAL POWER PART PLAN - FIRST FLOOR - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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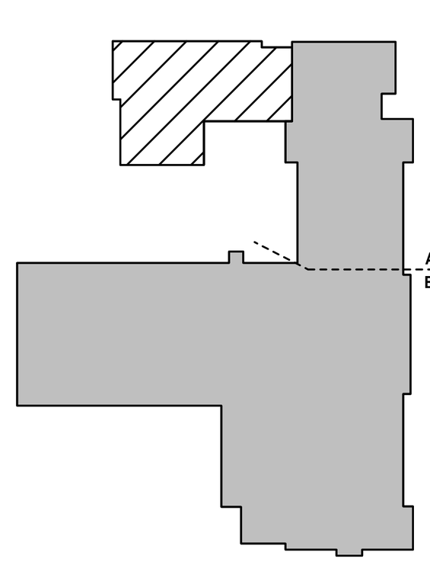

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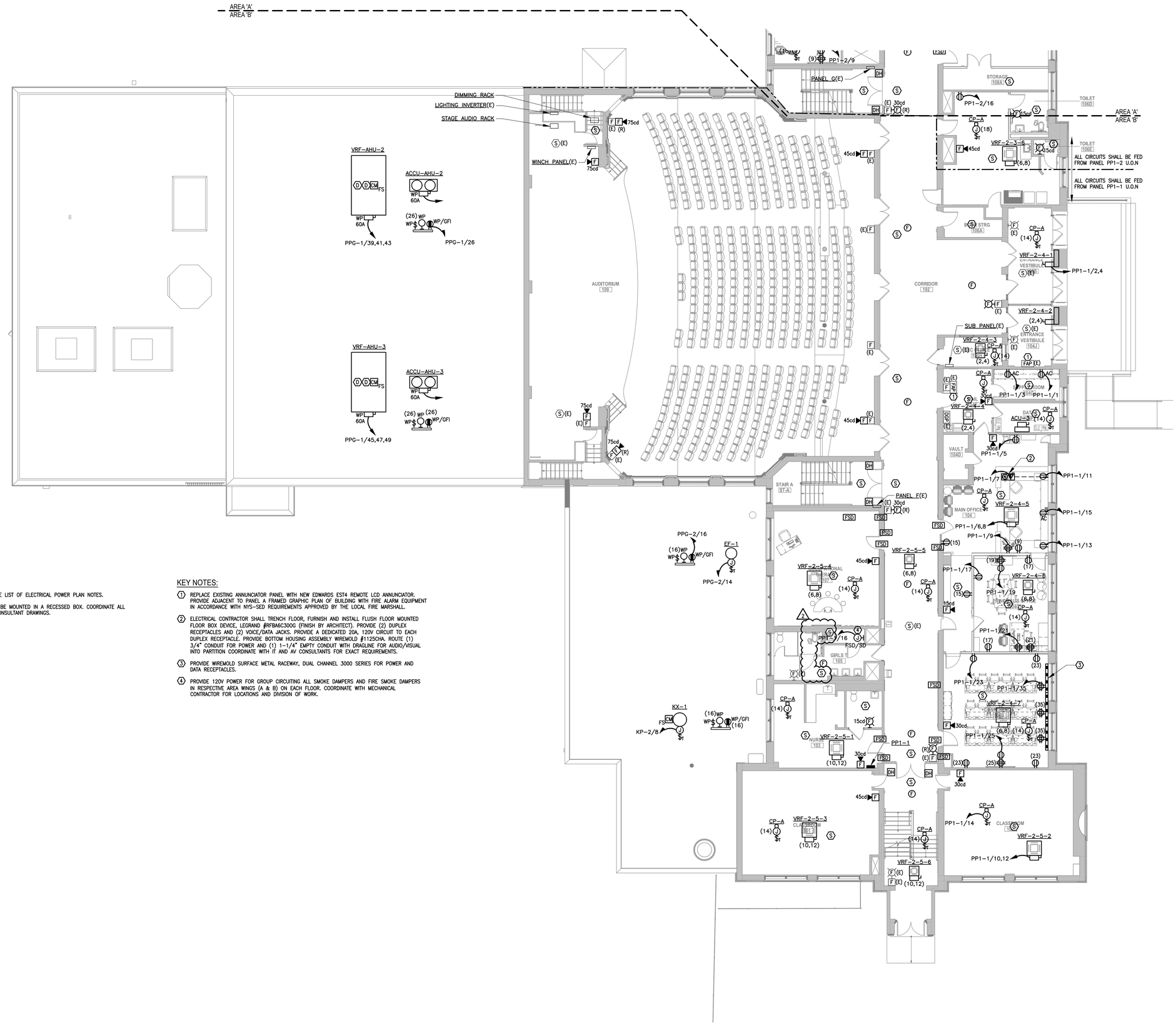
ELECTRICAL POWER PART PLAN - FIRST FLOOR - AREA A

E103-A



POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. RECEPTACLE FOR SMARTBOARDS SHALL BE MOUNTED IN A RECESSED BOX. COORDINATE ALL REQUIREMENTS WITH VENDOR AND IT CONSULTANT DRAWINGS.

KEY NOTES:

1. REPLACE EXISTING ANNUNCIATOR PANEL WITH NEW EDWARDS EST4 REMOTE LCD ANNUNCIATOR. PROVIDE ADJACENT TO PANEL A FRAMED GRAPHIC PLAN OF BUILDING WITH FIRE ALARM EQUIPMENT IN ACCORDANCE WITH NYS-SED REQUIREMENTS APPROVED BY THE LOCAL FIRE MARSHALL.
2. ELECTRICAL CONTRACTOR SHALL TRENCH FLOOR, FURNISH AND INSTALL FLUSH FLOOR MOUNTED FLOOR BOX DEVICE, LEGRAND #RFBA6C300G (FINISH BY ARCHITECT). PROVIDE (2) DUPLEX RECEPTACLES AND (2) VOICE/DATA JACKS. PROVIDE A DEDICATED 20A, 120V CIRCUIT TO EACH DUPLEX RECEPTACLE. PROVIDE BOTTOM HOUSING ASSEMBLY WIREMOLD #1125CHA. ROUTE (1) 3/4" CONDUIT FOR POWER AND (1) 1-1/4" EMPTY CONDUIT WITH DRAGLINE FOR AUDIO/VISUAL INTO PARTITION COORDINATE WITH IT AND AV CONSULTANTS FOR EXACT REQUIREMENTS.
3. PROVIDE WIREMOLD SURFACE METAL RACEWAY, DUAL CHANNEL 3000 SERIES FOR POWER AND DATA RECEPTACLES.
4. PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.

ELECTRICAL POWER PART PLAN - FIRST FLOOR - AREA B
1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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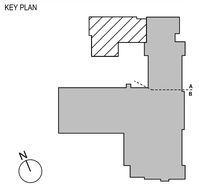
ISSUE	DATE
⚠ ADDENDUM #4	04/17/2026
ISSUED FOR BID	03/27/2026

KEY PLAN



SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

ELECTRICAL POWER PART PLAN - FIRST FLOOR - AREA B

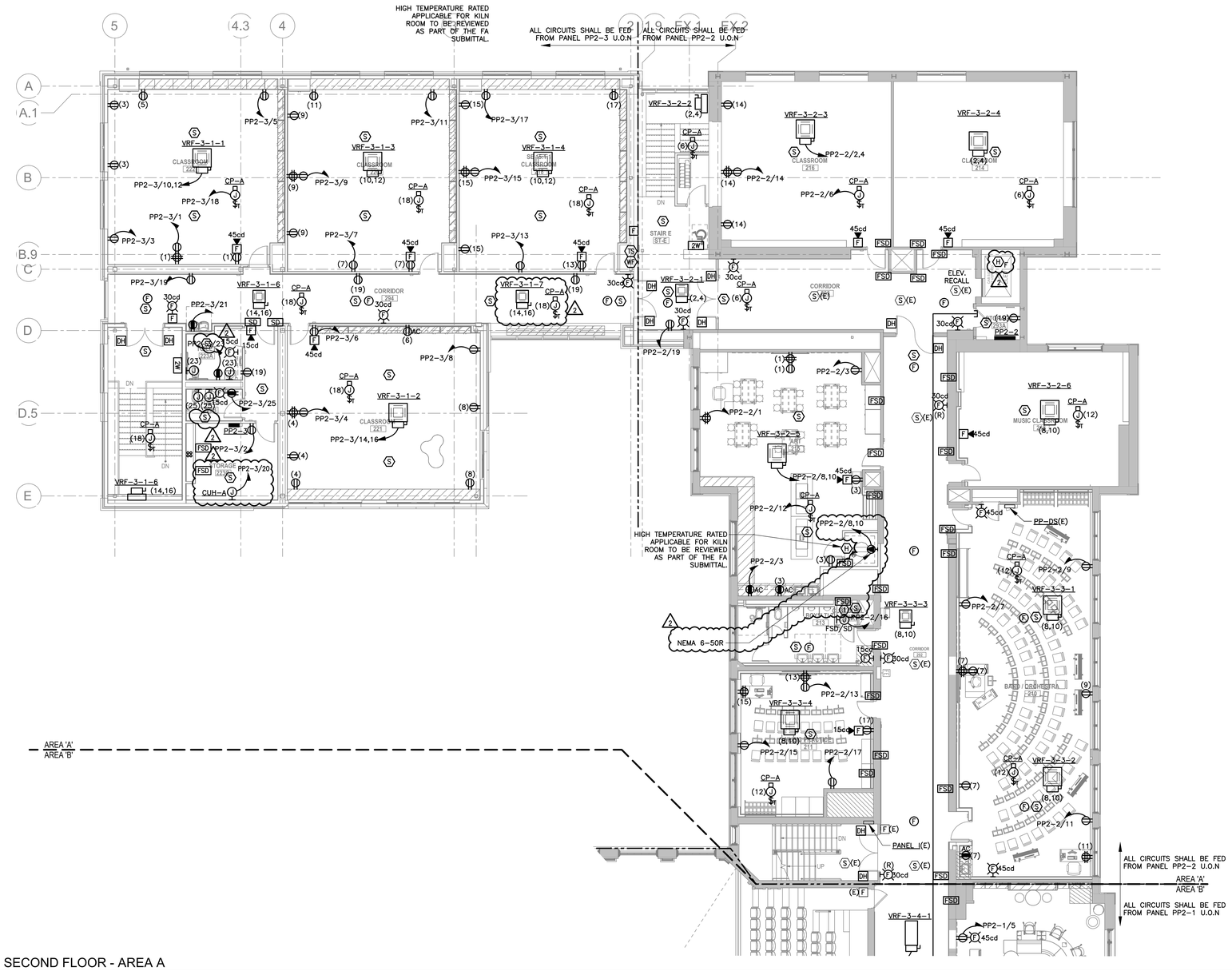
E103-B

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. RECEPTACLE FOR SMARTBOARDS SHALL BE MOUNTED IN A RECESSED BOX. COORDINATE ALL REQUIREMENTS WITH VENDOR AND IT CONSULTANT DRAWINGS.

POWER PLAN KEY NOTES:

1. PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.
2. ELECTRICAL CONTRACTOR SHALL TRENCH FLOOR, FURNISH AND INSTALL FLUSH FLOOR MOUNTED FLOOR BOX DEVICE, LEGRAND #RFBA6C30OG (FINISH BY ARCHITECT). PROVIDE (2) DUPLEX RECEPTACLES AND (2) VOICE/DATA JACKS. PROVIDE A DEDICATED 20A, 120V CIRCUIT TO EACH DUPLEX RECEPTACLE. PROVIDE BOTTOM HOUSING ASSEMBLY WIREMOLD #1125CHA. ROUTE (1) 3/4" CONDUIT FOR POWER AND (1) 1-1/4" EMPTY CONDUIT WITH DRAGLINE FOR AUDIO/VISUAL INTO PARTITION COORDINATE WITH IT AND AV CONSULTANTS FOR EXACT REQUIREMENTS.



ELECTRICAL POWER PART PLAN - SECOND FLOOR - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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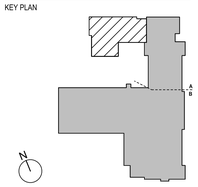
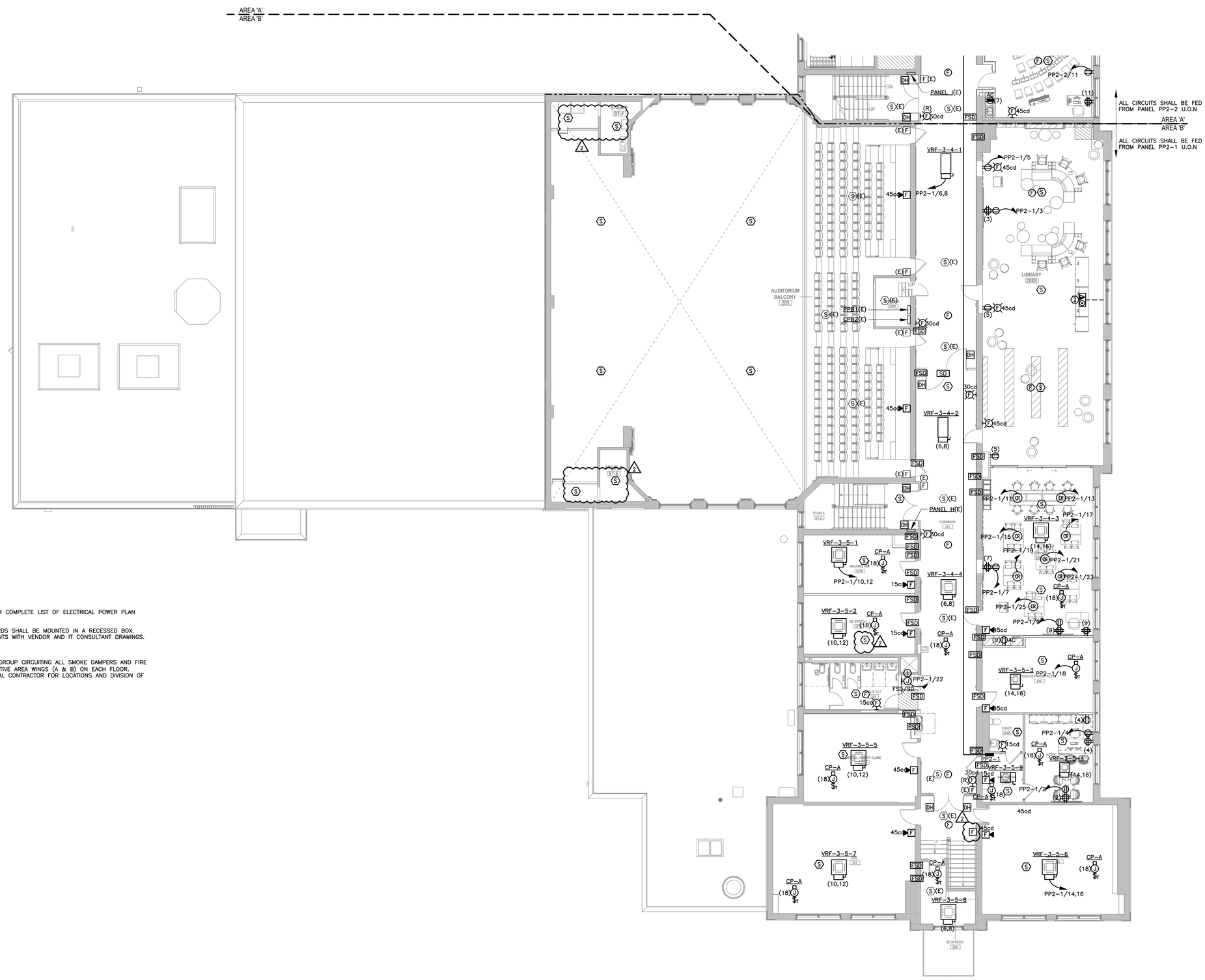


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

ELECTRICAL POWER PART PLAN - SECOND FLOOR - AREA A

E104-A



POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. RECEPTACLE FOR SMARTBOARDS SHALL BE MOUNTED IN A RECESSED BOX. COORDINATE ALL REQUIREMENTS WITH VENDOR AND IT CONSULTANT DRAWINGS.

POWER PLAN KEY NOTES:

(1) PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.

ELECTRICAL POWER PART PLAN - SECOND FLOOR - AREA B

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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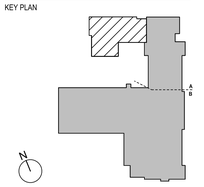
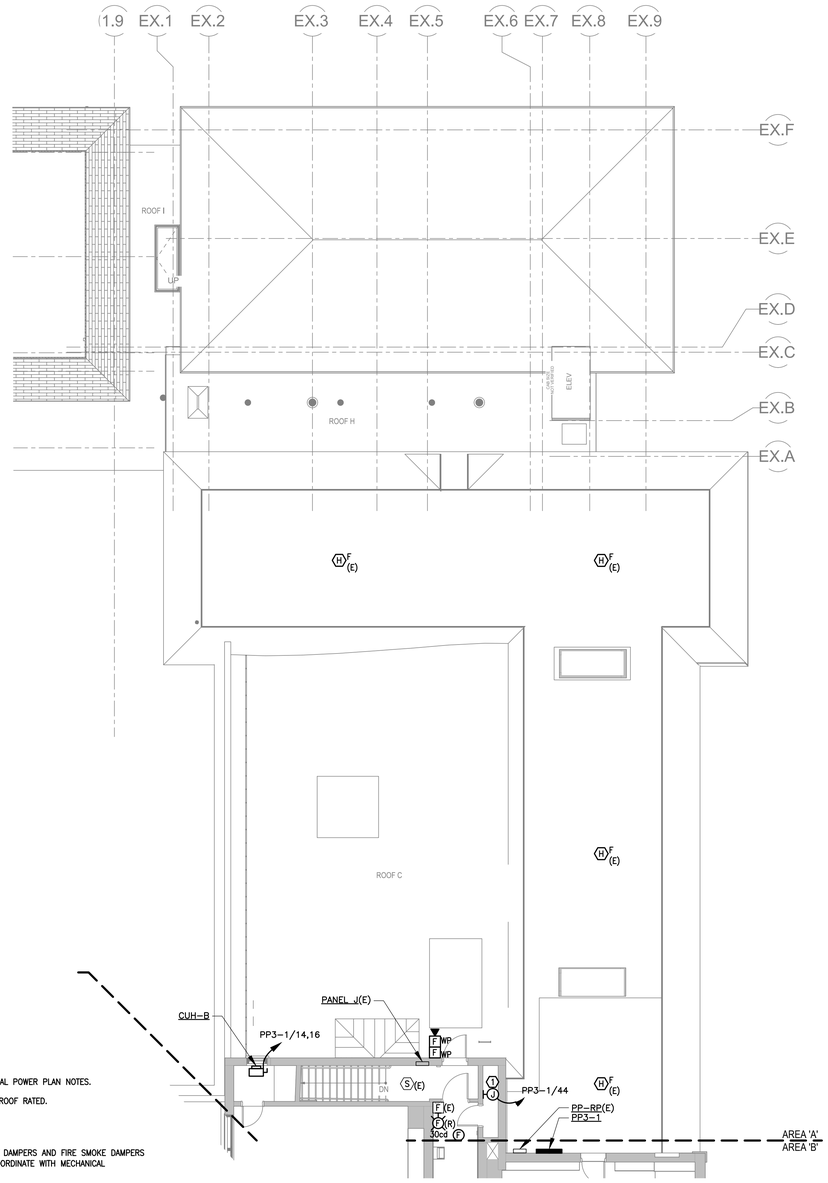


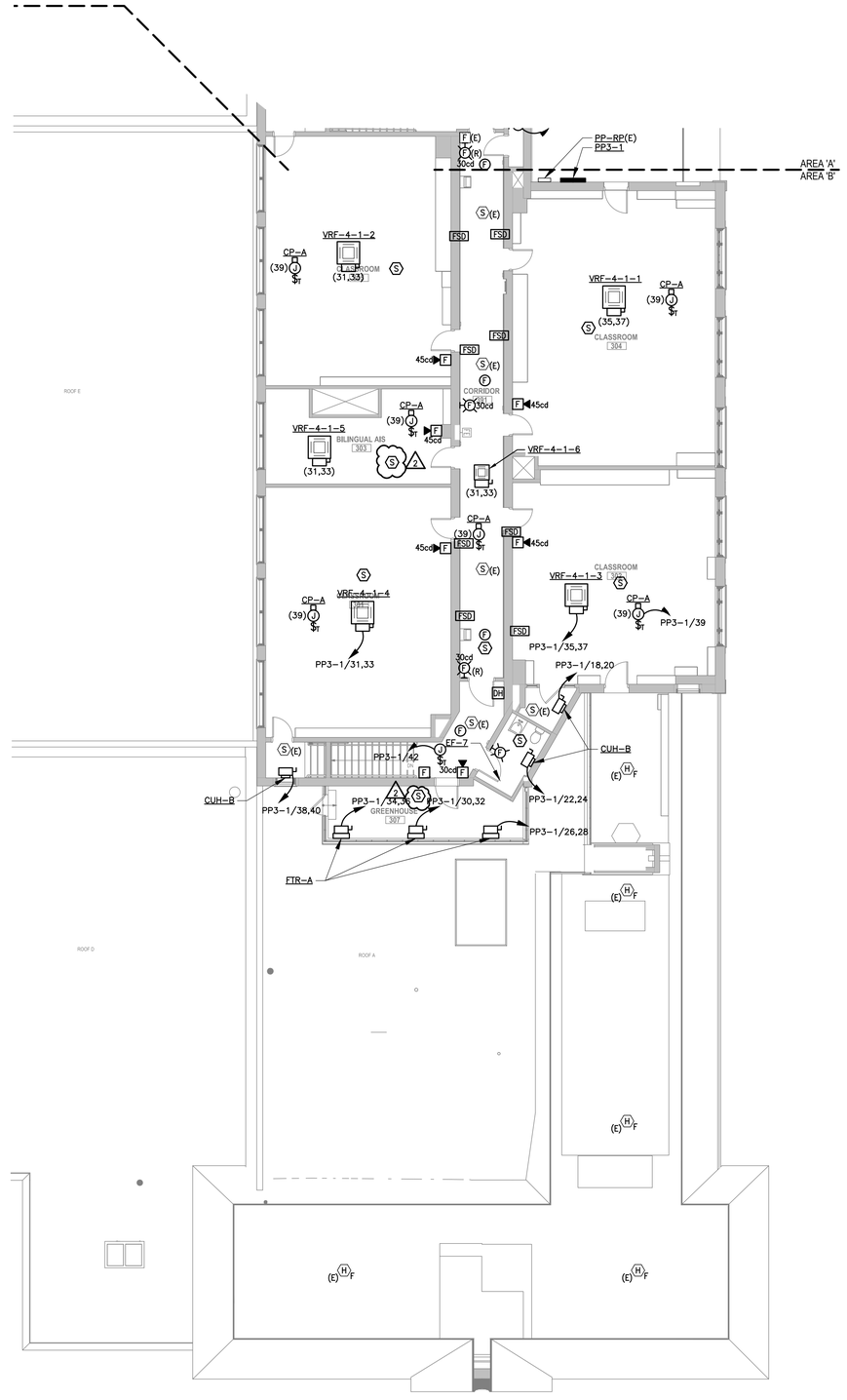
SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

ELECTRICAL POWER PART PLAN - SECOND FLOOR - AREA B

E104-B



AREA A



AREA B

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. ALL EXTERIOR DISCONNECT SWITCHES SHALL BE WEATHERPROOF RATED.

POWER PLAN KEY NOTES:

(1) PROVIDE 120V POWER FOR GROUP CIRCUITING ALL SMOKE DAMPERS AND FIRE SMOKE DAMPERS IN RESPECTIVE AREA WINGS (A & B) ON EACH FLOOR. COORDINATE WITH MECHANICAL CONTRACTOR FOR LOCATIONS AND DIVISION OF WORK.

ELECTRICAL POWER PLAN - THIRD FLOOR
1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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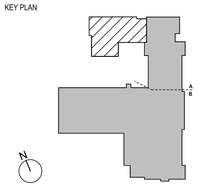


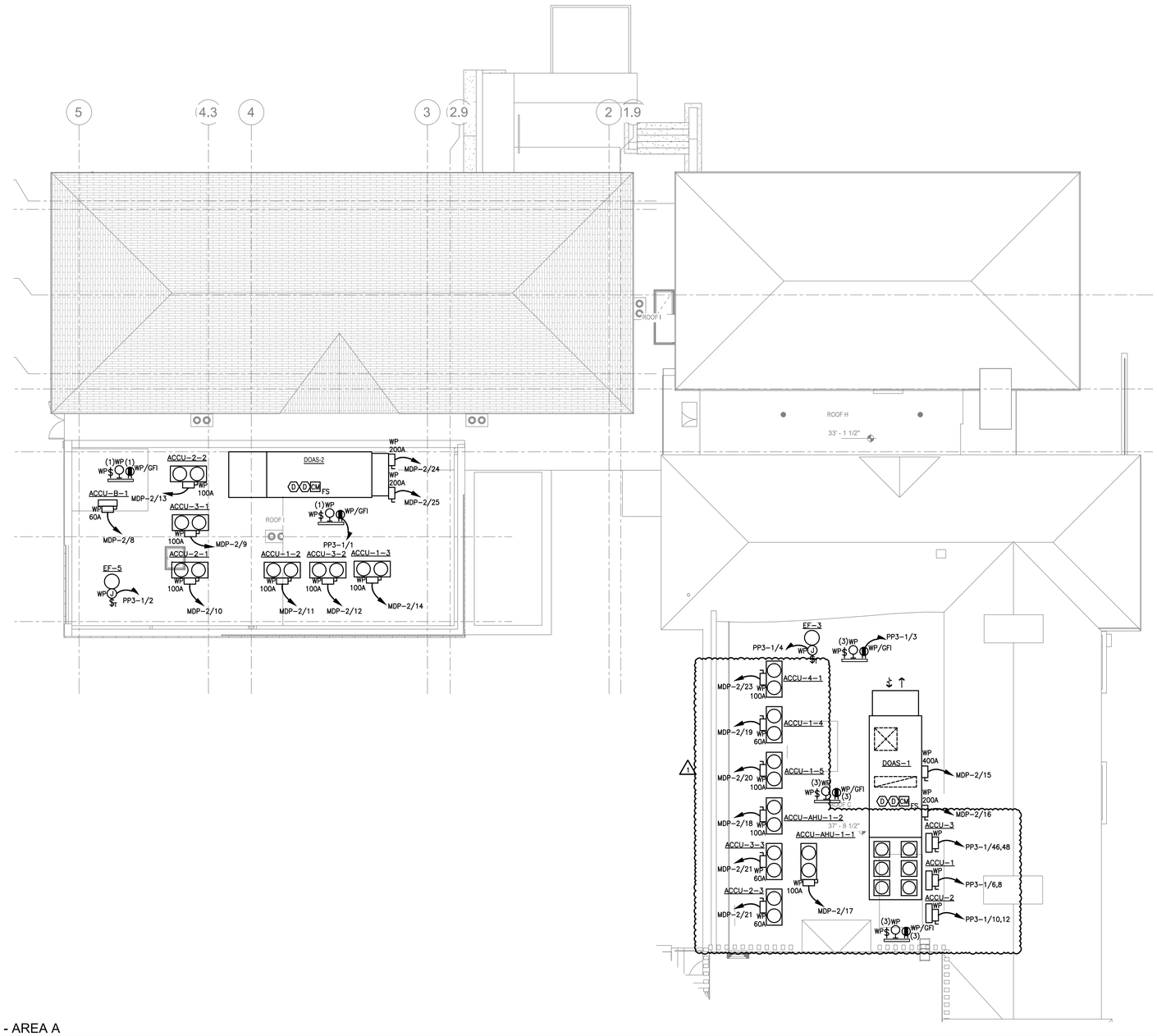
SED PROJECT NO. 28-05-01-06-1-009-XXX
MEMASI PROJECT NO. 101-2403

ELECTRICAL POWER PLAN - THIRD FLOOR

E105

POWER PLAN NOTES:

1. REFER TO SHEET E-002 FOR COMPLETE LIST OF ELECTRICAL POWER PLAN NOTES.
2. ALL EXTERIOR DISCONNECT SWITCHES SHALL BE WEATHERPROOF RATED.



ELECTRICAL POWER PART PLAN - ROOF - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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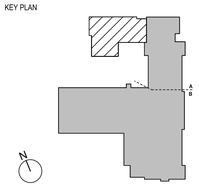

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MEMASI PROJECT NO. 101-2403

ELECTRICAL POWER PART PLAN - ROOF - AREA A

E106-A

DISTRIBUTION PANELBOARD DESIGNATION : **SWBD-1**

VOLTAGE	208Y/120 V	NEUTRAL	100%	BUS RATING	4000 A
PHASE	3 Ø	MIN. K.A.I.C. SYM	200 K.A.I.C.	MAIN FUSED SWITCH	4000 A
WIRE	4 W + G	REMARKS			

CIRCUIT BREAKER NO.	FRAME	TRIP	ø	LOAD DESCRIPTION	LOAD	QUANTITY OF FEEDERS (SETS)	FEEDER (EACH) PHASE LEGS NO.	PHASE LEGS SIZE	NEUTRAL NO.	NEUTRAL SIZE	GROUND NO.	GROUND SIZE	INSULATION TYPE	CONDUIT SIZE	REMARKS
1	400A	400A	3	PPSB-1	120.4 KVA	REFER TO RISER DIAGRAM									
2	225A	225A	3	PPB-1	28.0 KVA	REFER TO RISER DIAGRAM									
3	100A	100A	3	PPG-3	18.6 KVA	REFER TO RISER DIAGRAM									
4	100A	100A	3	PP1-3	16.5 KVA	REFER TO RISER DIAGRAM									
5	100A	100A	3	PP2-3	15.0 KVA	REFER TO RISER DIAGRAM									
6	800A	800A	3	PPG-1	232.2 KVA	REFER TO RISER DIAGRAM									
7	400A	400A	3	PP3-1	106.3 KVA	REFER TO RISER DIAGRAM									
8	60A	35A	3	ACCU-B1	7.9 KVA	1	3A	8	_	_	1	10	_	3/4"C	
9	100A	80A	3	ACCU-3-1	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
10	100A	80A	3	ACCU-2-1	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
11	100A	80A	3	ACCU-1-2	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
12	100A	80A	3	ACCU-3-2	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
13	100A	80A	3	ACCU-2-2	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
14	100A	80A	3	ACCU-1-3	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
15	400A	350A	3	DOAS-1 CONNECTION #1	96.4 KVA	1	3A	400	_	_	1	3	_	4"C	
16	225A	175A	3	DOAS-1 CONNECTION #2	47.8 KVA	1	3A	400	_	_	1	3	_	4"C	
17	100A	80A	3	ACCU-AHU-1-1	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
18	100A	80A	3	ACCU-AHU-1-2	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
19	100A	60A	3	ACCU-1-4	19.8 KVA	1	3A	6	_	_	1	10	_	1-1/4"C	
20	100A	80A	3	ACCU-1-5	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
21	100A	60A	3	ACCU-3-3	21.2 KVA	1	3A	6	_	_	1	10	_	1-1/4"C	
22	100A	60A	3	ACCU-2-3	21.2 KVA	1	3A	6	_	_	1	10	_	1-1/4"C	
23	100A	80A	3	ACCU-4-1	21.2 KVA	1	3A	4	_	_	1	8	_	1-1/4"C	
24	225A	200A	3	DOAS-2 CONNECTION #1	53.5 KVA	1	3A	400	_	_	1	3	_	4"C	
25	225A	110A	3	DOAS-2 CONNECTION #2	29.9 KVA										
26	225A	200A	3	**SPARE**											
27	800A	800A	3	MDP-1 (BACK FEED FOR EXISTING SWITCHGEAR)	160.0 KVA	REFER TO RISER DIAGRAM									
28	100A	100A	3	**SPARE**											

PANEL DESIGNATION : **PPB-1**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN LUGS ONLY	N/A
WIRE	4 W + G			MAIN BUS	225 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE	X	GROUND BUS	X
FEED THROUGH LUGS					

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	ELEC ROOM REC	4280			DWH-2 3#8 + 1#10GRD IN 3/4"C	45A	2
3	20A	WATER SERVICE ROOM REC		4280				4
5	20A	BASEMENT REC			5000			6
7	20A	TOILET B09A REC	1688			ELEC RM UH-A	20A	8
9	20A	TOILET B09B REC		1688				10
11	20A	FOUNTAIN ANTEROOM B09 REC			1688	WATER SERVICE RM UH-A	20A	12
13	20A	STAIR D LIGHTING	1788					14
15	20A	BASEMENT LIGHTING		780		BASEMENT VRF	15A	16
17	20A	EXIT SIGNS			420			18
19	20A	OAF-2	400			CP-A	20A	20
21	20A	**SPARE**		1500		TOILET B09A CUH-B	20A	22
23	20A	**SPARE**			1500			24
25	20A	**SPARE**	1500			TOILET B09B CUH-B	20A	26
27	20A	**SPARE**		1500				28
29	20A	**SPARE**			0	**SPARE**	20A	30
31	20A	**SPARE**	0			**SPARE**	20A	32
33	20A	**SPARE**		0		**SPARE**	20A	34
35	20A	**SPARE**			0	**SPARE**	20A	36
37			0					38
39				0				40
41					0			42
TOTAL CONNECTED LOAD PER PHASE (kVA)			9.68	9.75	8.61			
TOTAL CONNECTED LOAD			28.01 KVA			77.8 A		
TOTAL DEMAND LOAD			28.01 KVA			77.8 A		

PANEL DESIGNATION : **PPG-3**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	100 A
WIRE	4 W + G			MAIN BUS	100 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE	X	GROUND BUS	X
FEED THROUGH LUGS		NEMA 3R ENCLOSURE			

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	CLASSROOM 22 SMARTBOARD REC	920			CLASSROOM VRF	15A	2
3	20A	CLASSROOM 22 REC		920				4
5	20A	CLASSROOM 22 REC			920	CORRIDOR AND CLASSROOM VRF	15A	6
7	20A	CORRIDOR C004	920					8
9	20A	CORRIDOR C004 FOUNTAIN		1020		CP-A	20A	10
11	20A	TOILET 23A			480	FSD	20A	12
13	20A	REC	1680			CUH-A	20A	14
15	20A	**SPARE**		0		**SPARE**	20A	16
17	20A	STORAGE 23C			280	**SPARE**	20A	18
19	20A	CLASSROOM 21 SMARTBOARD REC	720			**SPARE**	20A	20
21	20A	CLASSROOM 21 REC		720		**SPARE**	20A	22
23	20A	CLASSROOM 21 REC			720	**SPARE**	20A	24
25	20A	CLASSROOM 20 SMARTBOARD REC	720			**SPARE**	20A	26
27	20A	CLASSROOM 20 REC		720		**SPARE**	20A	28
29	20A	CLASSROOM 20 REC			720	**SPARE**	20A	30
31	20A	CLASSROOM 19 SMARTBOARD REC	720			**SPARE**	20A	32
33	20A	CLASSROOM 19 REC		720		**SPARE**	20A	34
35	20A	CLASSROOM 19 REC			720	**SPARE**	20A	36
37	20A	CLASSROOM LIGHTING	500			**SPARE**	20A	38
39	20A	CLASSROOM LIGHTING		500		**SPARE**	20A	40
41	20A	CORRIDOR C004 LIGHTING			500	**SPARE**	20A	42
43	20A	**SPARE**	0			**SPARE**	20A	44
45	20A	EMERGENCY SIGNS		0		**SPARE**	20A	46
47	20A	**SPARE**				**SPARE**	20A	48

PANEL DESIGNATION : **PP1-3**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	100 A
WIRE	4 W + G			MAIN BUS	100 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE		GROUND BUS	X
FEED THROUGH LUGS		NEMA 3R ENCLOSURE			

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	STORAGE 123C REC	1020			CLASSROOM VRF	15A	2
3	20A	TOILET 123B REC		1020				4
5	20A	TOILET 123B REC			1020	CORRIDOR, STAIR & CLASSROOM VRF	15A	6
7	20A	CLASSROOM 121 REC	1020					8
9	20A	CLASSROOM 121 REC		1120		CP-A	20A	10
11	20A	CLASSROOM 121 SMARTBOARD REC			920	FSD	20A	12
13	20A	CORRIDOR C103 FOUNTAIN	2220			CUH-A	20A	14
15	20A	CORRIDOR C103		720		**SPARE**	20A	16
17	20A	CLASSROOM 122 REC			720	**SPARE**	20A	18
19	20A	CLASSROOM 122 REC	720			**SPARE**	20A	20
21	20A	CLASSROOM 122 SMARTBOARD REC		720		**SPARE**	20A	22
23	20A	CLASSROOM 120 REC			720	**SPARE**	20A	24
25	20A	CLASSROOM 120 REC	720			**SPARE**	20A	26
27	20A	CLASSROOM 120 SMARTBOARD REC		720		**SPARE**	20A	28
29	20A	CLASSROOM 118 REC			720	**SPARE**	20A	30
31	20A	CLASSROOM 118 REC	720			**SPARE**	20A	32
33	20A	CLASSROOM 118 SMARTBOARD REC		720		**SPARE**	20A	34
35	20A	CLASSROOM 121 LIGHTING			300	**SPARE**	20A	36
37	20A	CORRIDOR C103	140			**SPARE**	20A	38
39	20A	CLASSROOM LIGHTING		540		**SPARE**	20A	40
41	20A	EXIT SIGNS			20	**SPARE**	20A	42
TOTAL CONNECTED LOAD PER PHASE (kVA)			6.56	5.56	4.42			
TOTAL CONNECTED LOAD			16.54 KVA			45.9 A		
TOTAL DEMAND LOAD			15.42 KVA			42.8 A		

PANEL DESIGNATION : **PP2-3**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	100 A
WIRE	4 W + G			MAIN BUS	100 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE		GROUND BUS	X
FEED THROUGH LUGS		NEMA 3R ENCLOSURE			

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	CLASSROOM 222 REC	900			ELEC CLSOET REC	20A	2
3	20A	CLASSROOM 222 REC		1440		CLASSROOM 221 REC	20A	4
5	20A	CLASSROOM 222 REC			1440	CLASSROOM 221 REC	20A	6
7	20A	CLASSROOM 220 REC	1440			CLASSROOM 221 REC	20A	8
9	20A	CLASSROOM 220 REC		920		CLASSROOM VRF	15A	10
11	20A	CLASSROOM 220 REC			920			12
13	20A	CLASSROOM 218 REC	920			CLASSROOM, CORRIDOR, & STAIRS VRF	15A	14
15	20A	CLASSROOM 218 REC		920				16
17	20A	CLASSROOM 218 REC			1120	CP-A	20A	18
19	20A	CORRIDOR REC	2040			CUH-A	20A	20
21	20A	CORRIDOR FOUNTAIN REC		1200		**SPARE**	20A	22
23	20A	TOILET 223A REC			360	**SPARE**	20A	24
25	20A	TOILET 223B REC	360			**SPARE**	20A	26
27	20A	CLASSROOM 221 LIGHTING		300		**SPARE**	20A	28
29	20A	CORRIDOR C204 LIGHTING			140	**SPARE**	20A	30
31	20A	CLASSROOM LIGHTING	540			**SPARE**	20A	32
33	20A	EXIT SIGNS		40		**SPARE**	20A	34
35	20A	**SPARE**			0	**SPARE**	20A	36
37	20A	**SPARE**	0			**SPARE**	20A	38
39	20A	**SPARE**		0		**SPARE**	20A	40
41	20A	**SPARE**			0	**SPARE**	20A	42
TOTAL CONNECTED LOAD PER PHASE (kVA)			6.20	4.82	3.98			
TOTAL CONNECTED LOAD			15.00 KVA			41.6 A		
TOTAL DEMAND LOAD			14.36 KVA			39.9 A		

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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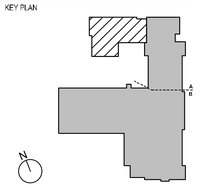
⚠1 ADDENDUM #4	04/17/2026
ISSUED FOR BID	03/27/2026
ISSUE	DATE

KEY PLAN



SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

ELECTRICAL PANEL SCHEDULE SHEET1

E601

PANEL DESIGNATION : **PP2-1**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	100 A
WIRE	4 W + G			MAIN BUS	100 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE	X	GROUND BUS	X
FEED THROUGH LUGS		NEMA 3R ENCLOSURE			

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	DEDICATED LIBRARY DESK REC	1920			ASSOC PRINCIPAL OFFICE REC	20A	2
3	20A	LIBRARY SMARTBOARD REC		1440		ASSOC PRINCIPAL OFFICE REC	20A	4
5	20A	LIBRARY REC			560	CORRIDOR VRF	15A	6
7	20A	STEM LAB REC	920					8
9	20A	STEM LAB REC		920		CLASSROOM VRF	15A	10
11	20A	STEM LAB CORD REEL			1200			12
13	20A	STEM LAB CORD REEL	1200			CLASSROOM VRF	15A	14
15	20A	STEM LAB CORD REEL		1200				16
17	20A	STEM LAB CORD REEL			1400	CP-A	20A	18
19	20A	STEM LAB CORD REEL	2100			FIRE SMOKE DAMPERS, SMOKE DAMPERS	20A	20
21	20A	STEM LAB CORD REEL		1100		EXTERIOR LIGHTING	20A	22
23	20A	STEM LAB CORD REEL			1000	**SPARE**	20A	24
25	20A	STEM LAB CORD REEL	1000			**SPARE**	20A	26
27	20A	STEM LAB LIGHTING		540		**SPARE**	20A	28
29	20A	CORRIDOR C201 LIGHTING			280	**SPARE**	20A	30
31	20A	STEM LAB LIGHTING	780			**SPARE**	20A	32
33	20A	EXIT SIGNS		100		**SPARE**	20A	34
35	20A	**SPARE**			0	**SPARE**	20A	36
37	20A	**SPARE**	0			**SPARE**	20A	38
39	20A	**SPARE**		0		**SPARE**	20A	40
41	20A	**SPARE**			0	**SPARE**	20A	42

TOTAL CONNECTED LOAD PER PHASE (kVA)	7.92	5.30	4.44
TOTAL CONNECTED LOAD	17.66 KVA		49.0 A
TOTAL DEMAND LOAD	16.08 KVA		44.6 A

PANEL DESIGNATION : **PP2-2**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	42
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	100 A
WIRE	4 W + G			MAIN BUS	100 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE	X	GROUND BUS	X
FEED THROUGH LUGS					

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	ART CLASSROOM 215 REC	920			NORTH CLASSROOMS, CORRIDOR & STAIRS VRF	15A	2
3	20A	ART CLASSROOM 215 REC		920				4
5	20A	ART CLASSROOM 215 REC			920	NORTH CP-A	20A	6
7	20A	ORCHESTRA ROOM 210	920			CLASSROOMS & CORRIDOR VRF	15A	8
9	20A	ORCHESTRA ROOM 210		920				10
11	20A	ORCHESTRA ROOM 210			920	CP-A	20A	12
13	20A	MUSIC PRACTICE RM 211B	1620			CLASSROOM 216 REC	20A	14
15	20A	MUSIC PRACTICE RM 211B		1720		FIRE SMOKE DAMPERS, SMOKE DAMPERS	20A	16
17	20A	MUSIC PRACTICE RM 211B			720	**SPARE**	20A	18
19	20A	CORRIDOR REC	360			**SPARE**	20A	20
21	20A	MUSIC/ART LIGHTING		700		**SPARE**	20A	22
23	20A	CORRIDOR C202/203 LIGHTING			300	**SPARE**	20A	24
25	20A	BAND ROOM LIGHTING	860			**SPARE**	20A	26
27	20A	EXIT SIGNS		40		**SPARE**	20A	28
29	20A	**SPARE**			0			30
31	50A	KILN POWER 2#8 + 1#10GND IN 3/4"C	4000					32
33				4000				34
35					0			36
37			0					38
39				0				40
41					0			42

TOTAL CONNECTED LOAD PER PHASE (kVA)	8.68	8.30	2.86
TOTAL CONNECTED LOAD	19.84 KVA		55.1 A
TOTAL DEMAND LOAD	19.84 KVA		55.1 A

PANEL DESIGNATION : **PP3-1**

VOLTAGE	208Y/120 V	NEUTRAL	100%	QUANTITY OF POLES	84
PHASE	3 Ø	SCC RATING (SYM)	42 K.A.I.C.	MAIN CIRCUIT BREAKER	400 A
WIRE	4 W + G			MAIN BUS	400 A
SURFACE MOUNTED	X	NEMA 1 ENCLOSURE	X	GROUND BUS	X
FEED THROUGH LUGS					

REMARKS :

CKT #	TRIP	LOAD DESCRIPTION	ØA (VA)	ØB (VA)	ØC (VA)	LOAD DESCRIPTION	TRIP	CKT #
1	20A	ROOF I REC	700			EF-5	20A	2
3	20A	ROOF C REC		700		EF-3	20A	4
5	20A	ROOF REC			1872	ACCU-1	20A	6
7			6640					8
9	60A	ACCU-2-4 3#6 + 1#10GRD IN 1"C		6640		ACCU-2	20A	10
11					6640			12
13			8572			CUH-B STAIRS	20A	14
15	70A	ACCU-2-5 3#4 + 1#8GRD IN 1-1/4"C		8572				16
17					8572	CUH-B STORAGE CLOSET	20A	18
19			6768					20
21	60A	ACCU-3-4 3#6 + 1#10GRD IN 1"C		6768		CUH-B SINGLE OCCUPANT RESTROOM	20A	22
23					6768			24
25			8322			FTR-A GREENHOUSE	20A	26
27	80A	ACCU-3-5 3#4 + 1#8GRD IN 1-1/4"C		8322				28
29					8322	FTR-A GREENHOUSE	20A	30
31	15A	WEST CLASSROOM & CORRIDOR VRF	1450					32
33				1450		FTR-A GREENHOUSE	20A	34
35	15A	EAST CLASSROOM VRF			1450			36
37			1700			CUH-B STORAGE CLOSET	20A	38
39	20A	CP-A		1700				40
41	20A	3RD FL CORRIDOR LIGHTING			500	EX-7	20A	42
43	20A	3RD FL CLASSROOM LIGHTING WEST	500			FIRE SMOKE DAMPERS	20A	44
45	20A	3RD FL CLASSROOM LIGHTING EAST		1672		ACCU-3	20A	46
47	20A	ATTIC LIGHTING			1672			48
49	20A	**SPARE**	0			**SPARE**	20A	50
51	20A	**SPARE**		0		**SPARE**	20A	52
53	20A	**SPARE**			0	**SPARE**	20A	54
55	20A	**SPARE**	0			**SPARE**	20A	56
57	20A	**SPARE**		0		**SPARE**	20A	58
59	20A	**SPARE**			0	**SPARE**	20A	60
61			0					62
63				0				64
65					0			66
67			0					68
69				0				70
71					0			72
73			0					74
75				0				76
77					0			78
79			0					80
81				0				82
83					0			84

TOTAL CONNECTED LOAD PER PHASE (kVA)	34.65	35.82	35.80
TOTAL CONNECTED LOAD	106.27 KVA		295.0 A
TOTAL DEMAND LOAD	106.27 KVA		295.0 A

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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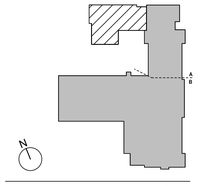
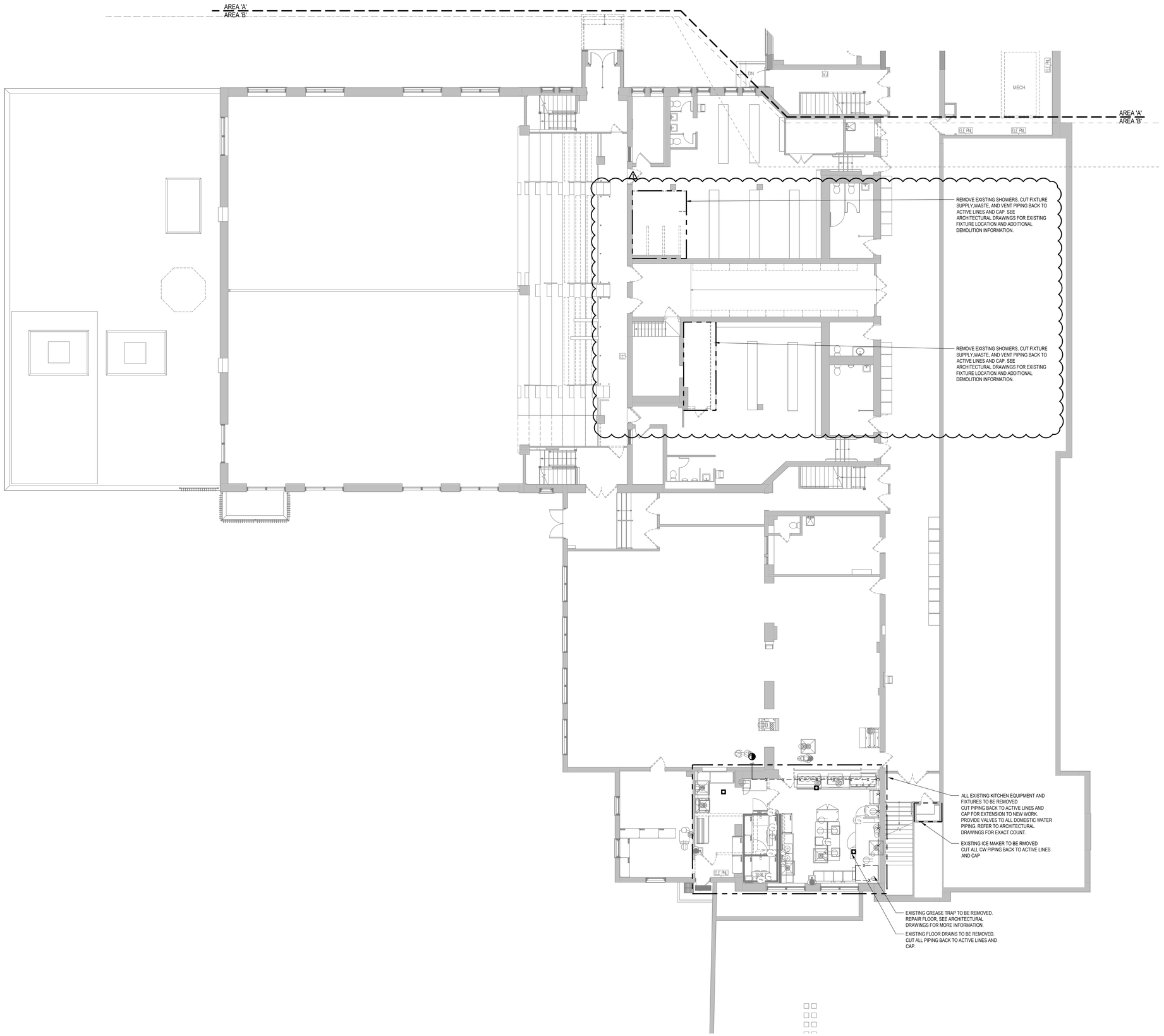


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

ELECTRICAL PANEL SCHEDULES SHEET3

E603



PLUMBING DEMOLITION PART PLAN - GROUND FLOOR - AREA B

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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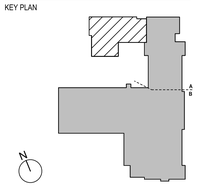
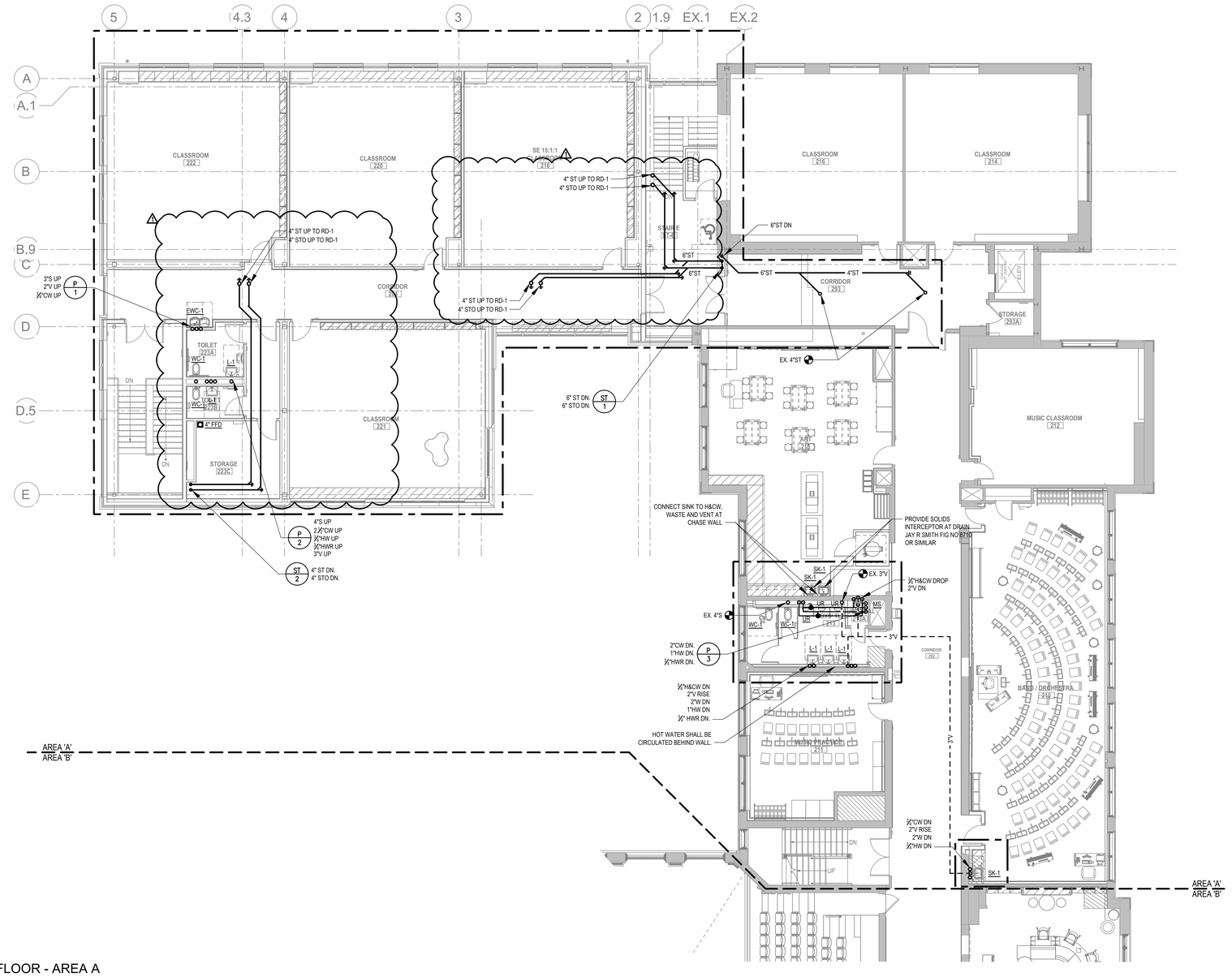


SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

PLUMBING DEMOLITION PART PLAN - GROUND FLOOR - AREA B

PD102-B



PLUMBING PART PLAN - SECOND FLOOR - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

WASHINGTON IRVING INTERMEDIATE SCHOOL

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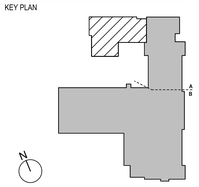

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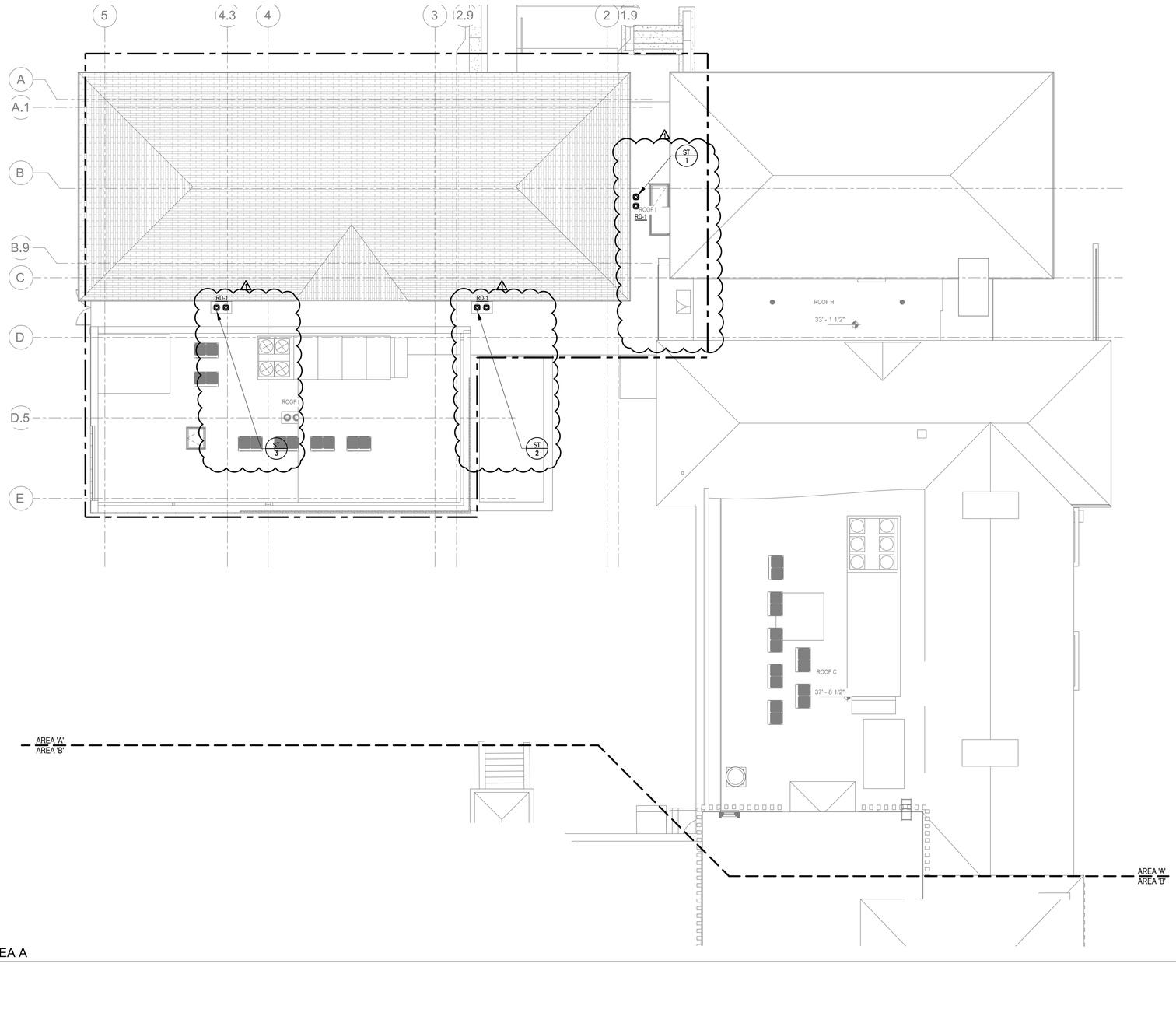
MEMASI PROJECT NO. 101-2403

PLUMBING PART PLAN - SECOND FLOOR - AREA A

P104-A



PLUMBING PART PLAN - ROOF - AREA A

1/8" = 1'-0"

PUBLIC SCHOOLS OF THE TARRYTOWNS

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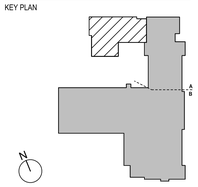
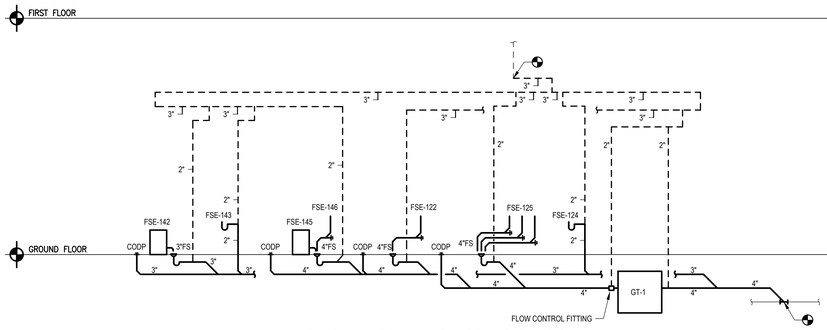


SED PROJECT NO.	28-05-01-06-1-009-XXX
MEMASI PROJECT NO.	101-2403

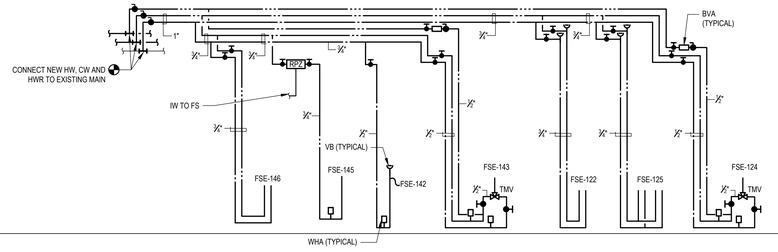
PLUMBING PART PLAN - ROOF - AREA A

P106-A



KITCHEN SANITARY RISER DIAGRAM

SCALE: NTS



KITCHEN DOMESTIC WATER RISER DIAGRAM

SCALE: NTS

NOTES

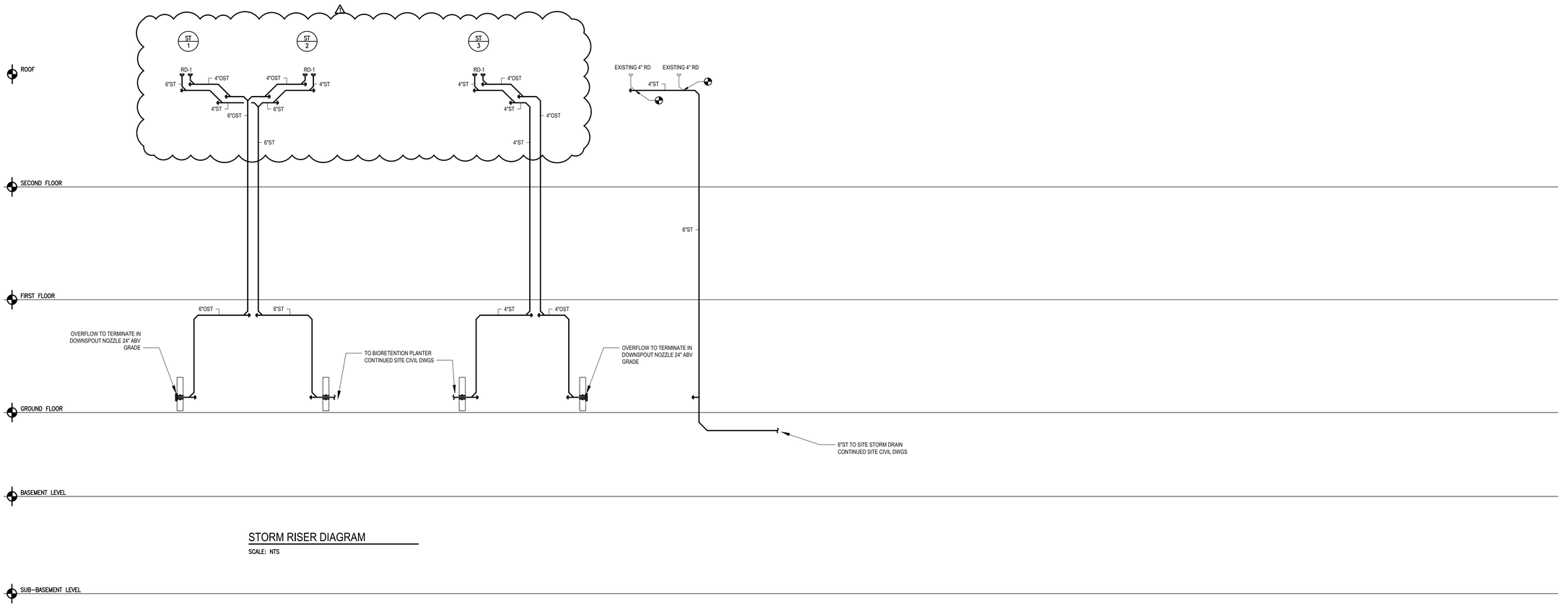
1. LEAK DETECTION DEVICE TO BE PROVIDED FOR ALL HWHS.
2. ACCESS PANELS TO BE PROVIDED FOR ALL VALVES. LOCATION TO BE COORDINATED WITH ARCHITECT.
3. SHOCK ABSORBERS TO BE PROVIDED AS SPECIFIED IN THESE DRAWINGS. ABSORBERS SHALL BE LOCATED IN ACCESSIBLE LOCATIONS (PREFERABLY ABOVE CEILING) AND NOT BEHIND DETAILED FINISHED CONSTRUCTION.
4. CONNECTION POINTS TO BE FIELD VERIFIED BY CONTRACTOR.
5. PROVIDE VACUUM BREAKER FOR CW CONNECTIONS TO REF. AND HW CONNECTION TO DW. PROVIDE DOUBLE CHECK VALVE FOR ALL CM. VB LOCATIONS TO BE COORDINATED BEHIND EQUIPMENT SERVED AND SHALL BE INSTALLED 6" ABOVE TOP OF EQUIPMENT.

FOOD SERVICE EQUIPMENT SCHEDULE

NO.	QTY.	CATEGORY	REMARKS
FSE-122	1	PREP TABLE (SINK)	NEW
FSE-124	1	HAND SINK	NEW
FSE-125	1	THREE COMPARTMENT SINK	NEW
FSE-142	1	ICE MACHINE	NEW
FSE-143	1	HAND SINK	NEW
FSE-145	1	DISHWASHER	NEW
FSE-146	1	SOILED DISHTABLE (SINK)	NEW

NOTES:

1. ALL EQUIPMENT MOUNTINGS, LOCATIONS AND ELEVATIONS ARE TO BE AS SHOWN ON COMMERCIAL FOOD SERVICE/KITCHEN CONSULTANT/ARCHITECTURAL DRAWINGS.
2. ALL EQUIPMENT TO BE FURNISHED BY COMMERCIAL FOOD SERVICE/KITCHEN CONSULTANT VENDOR. SEE COMMERCIAL FOOD SERVICE/KITCHEN CONSULTANT DRAWINGS AND SCHEDULES FOR ALL FIXTURE, APPLIANCE AND EQUIPMENT INFORMATION.



STORM RISER DIAGRAM

SCALE: NTS

PUBLIC SCHOOLS OF THE TARRYTOWNS

2024 CAPITAL BOND PROJECT - PHASE 2

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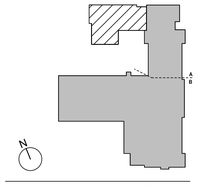
SITE - CIVIL CONSULTANT
THE LA GROUP
40 LONG ALLEY
SARATOGA SPRING, NY 12866

SECURITY CONSULTANT
BUILDING TECHNOLOGY CONSULTING LLC
992 BEDFORD STREET
BRIDGEWATER, MA. 02324

ISSUE	DATE
⚠1 ADDENDUM #4	04/17/2026
ISSUED FOR BID	03/27/2026

KEY PLAN



SED PROJECT NO. 28-05-01-06-1-009-XXX

MEMASI PROJECT NO. 101-2403

PLUMBING RISER DIAGRAMS

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